PROJECT INFORMATION:

DR. PAULA KISLAK 545 HODGES LANE

3627 CAMPANIL DRIVE

RS-1A (SBMC TITLE 30)

YES - COASTAL INLAND

GROUP R-3 (RESIDENCE)

YES

REQUIRED BY CITY OF SANTA BARBARA WILL BOTH BE ACCOMPLISHED BY

INSTALLATION OF CISTERN STORAGE SYSTEM. THIS SYSTEM ASSURES NO INCREASE

IN RUNOFF FROM THE PROPOSED DEVELOPMENT FOR STORM RUNOFF UP TO THE 25 YEAR STORM. OVERFLOW FOR STORMS GREATER THAN THIS WILL HONOR THE EXISTING HISTORICAL RUNOFF PATTERNS OF USING THE EXISTING NATURAL

IN THE CITY OF SANTA BARBARA PURSUANT TO SBMC 30.185.040. ADDITIONALLY,

STORM WATER SUMMARY:

WATER QUALITY PER TIER 2 (STORAGE NOT REQUIRED)

PROPOSED REPLACED/REDEVELOPED IMPERVIOUS AREA: 0 SF

SEE CIVIL PLANS ON SHEET C2.1 FOR CALCULATIONS & DETAILS

PROPOSED NEW IMPERVIOUS AREA: 1539 SF

TOTAL NEW IMPERVIOUS AREA: 1906 SF

SELF TREATING WALKS: 367 SF

SCOPE OF WORK

PROPOSED REMOVED IMPERVIOUS AREA: 264 SF EXISTING IMPERVIOUS AREA TO REMAIN: 7031 SF

PK99@COX.NET

047-101-003

61,561 SF 1.32 ACRES

SANTA BARBARA, CALIFORNIA 93108

SANTA BARBARA, CALIFORNIA 93109

EXISTING: 1 SINGLE FAMILY RESIDENCE PROPOSED: SINGLE FAMILY w/ ADU

LOW DENSITY RESIDENTIAL (MAX. 1 DU/AC.)

21% APPROX. PER CITY RECORDS

16'-0" FOR ACCESSORY DWELLING UNIT

FRONT: 35'-0" INTERIOR: 15'-0" (HOA 25'-0") ADU: 4'-0'

EXISTING PARKING - PRIMARY RES.: COVERED 3 / 1 UNCOVERED PROPOSED PARKING - PRIMARY RES.: COVERED 3 / 0 UNCOVERED

PROPOSED PARKING - ADU: PROPOSED 0 COVERED / 1 UNCOVERED `REQUIRED PARKING — ADU: 1 PURSUANT TO SBMC 30.185.040 SINCE\

PROJECT DOES NOT COMPLY WITH EXEMPTIONS

REQUIRED PARKING - PRIMARY RES.: 2 COVERED STALLS

30'-0" FROM LOWEST NATURAL OR FINISHED GRADE

OWNERS:

PROJECT ADDRESS:

PROPERTY USE:

LOT SIZE:

ZONING:

SETBACKS:

GENERAL PLAN:

SPECIAL DESIGN DISTRICT:

AVG. PARCEL SLOPE:

FEMA FLOOD ZONE:

FIRE SPRINKLERS:

TYPE OF CONSTRUCTION:

PARKING REQUIREMENTS:

HEIGHT LIMIT:

HIGH FIRE:

OCCUPANCY:

F.A.R. Cald	culator
nstructions: Enter the information in the white boxes below. The spreadsheet nax FAR (per the Zoning Ordinance for "Required FAR"), and the 85% max FAR will determine whether a FAR Modification is required. "Guideline FAR" calculatingle Family Residence Design Guidelines, page 23-C.	will calculate the proposed FAR (floor area ratio), the 10 R (per the Zoning Ordinance for "Required FAR"). Additi
he Net Lot Area does not include any Public Road Easements or Public Road leasements or Public Road lease shall include the net floor area of all stories of all building, but may or may rease the definitions please refer to SBMC §28.15.083 & 30.300. This form has respectively solutions of the SBMC §30.05.010 for comparison.	not include basement/cellar floor area. For further clarifi
ENTER Project Address:	3627 CAMPANIL DRIVE
Is there a basement or cellar existing or	No
proposed? ENTER Proposed TOTAL Net FAR Floor Area (in sq. ft.):	4,022
ENTER Zone ONLY from drop-down list:	A-1 or RS-1A
ENTER Net Lot Area (in sq. ft.):	57,499
Is the height of existing or proposed buildings 17 feet or greater?	No
Are existing or proposed buildings two stories or greater?	No
The FAR Requirements are:	GUIDELINE**
ENTER Average Slope of Lot:	19.00%
Does the height of existing or proposed buildings exceed 25 feet?	No
Is the site in the Hillside Design District?	Yes
Does the project include 500 or more cu. yds. of grading outside the main building footprint?	No
An FAR MOD is not required per SB	MC §28.15 or §30.20.030
FLOOR AREA RATIO (FAR):	0.070
Lot Size Range:	>= 20,000 sq. ft.
MAX FAR Calculation (in sq. ft.):	4,430 + (0.013 x lot size in sq. ft.)
100% MAX FAR:	0.090
100% MAX FAR (in sq. ft.):	5,177
85% of MAX FAR (in sq. ft.):	4,401
80% of MAX FAR (in sq. ft.):	4,142
The 4022 square foot proposed total	is 78% of the MAX FAR.*
NOTE: Percentage total is rounded up. NOTE: If your project is located on a site with multiple or overlay zones, printiations are "Required" or "Guideline".	please contact Planning Staff to confirm whether the
Acreage Conversion Ca	alculator
	1.32
Acreage Conversion Ca	

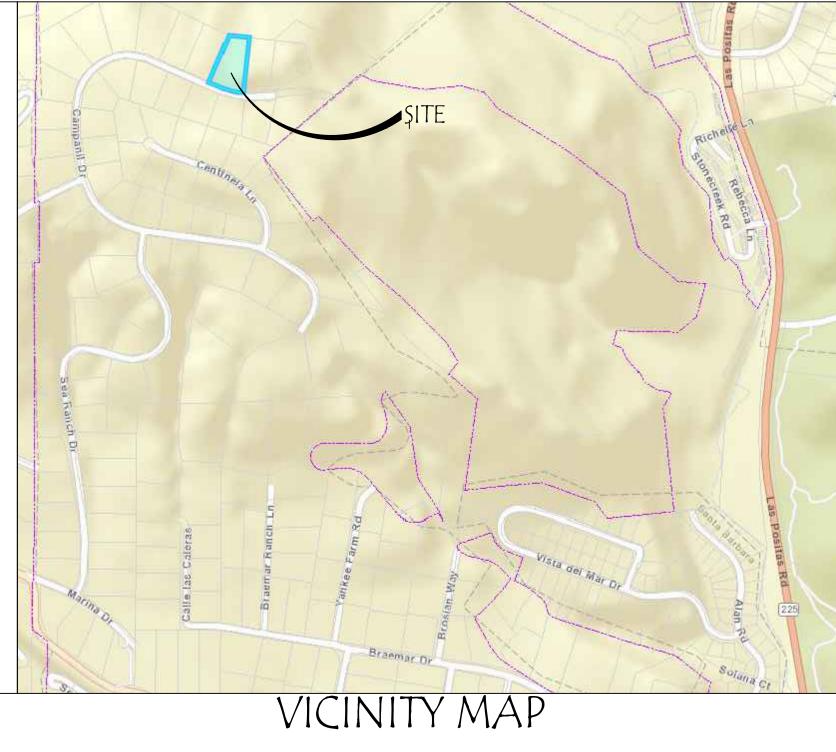
Net Lot Area (in sq. ft.):

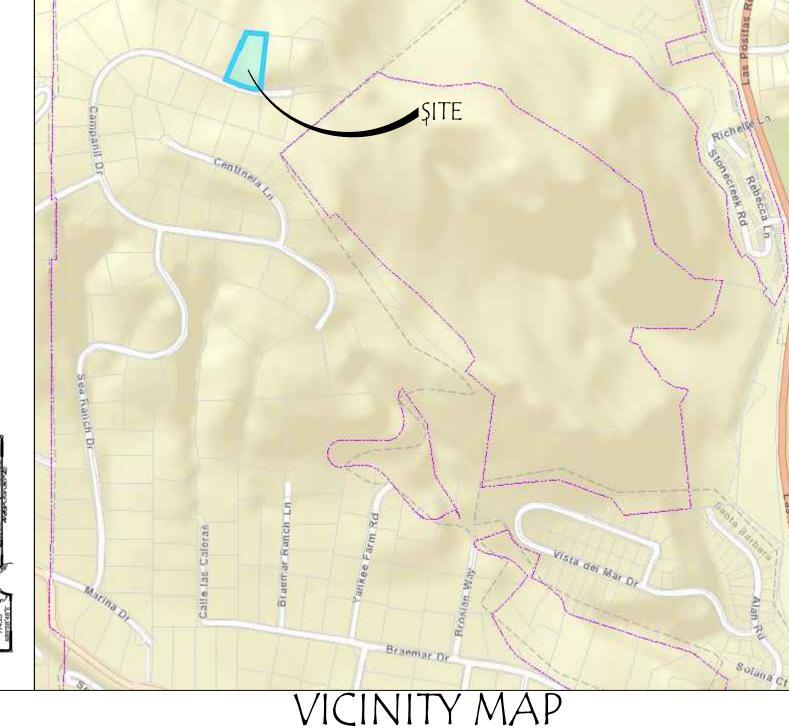
57499.2

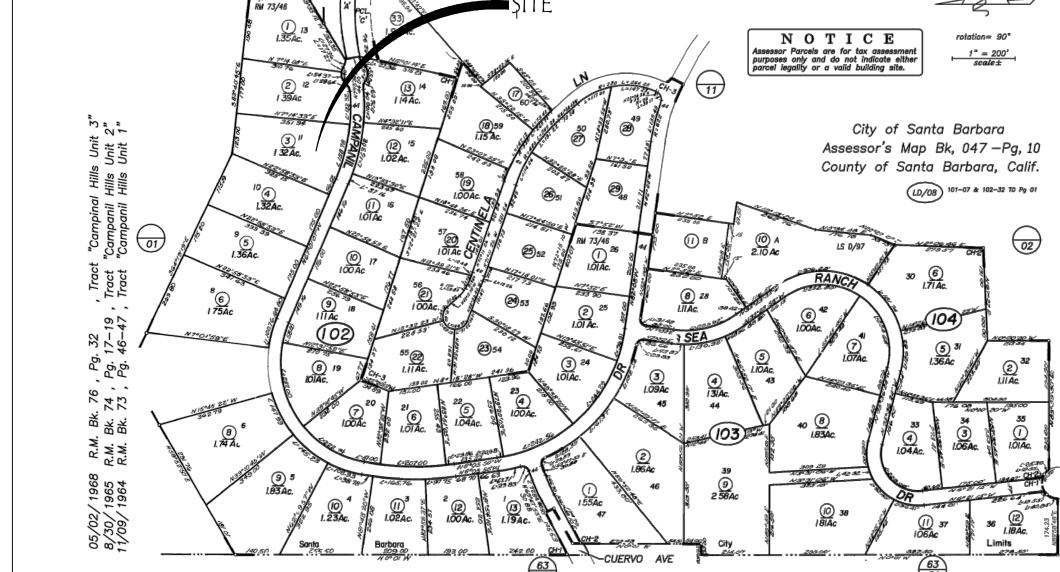
RICHARD E JOHNSON, A.I.A ALL CO	DMMON LAW COPYRIGHT AND OTHER PROPERTY RIGHTS RESERVED THIS DOCUMENTS IS AN MENT SHALL CONSTITUTE CONCLUSIVE EVIDENCE OF ACCEPTANCE OF THESE RESTRICTIONS (ORIGINAL AND UNPUBLI: CONTRACTORS SHALL VE	SHED WORK F ERIFY AND BE	
CONSULTANTS	:			
ADCUITECT:	MISSION GROUP ARCHITECTS	A-0	PROJE	
ARCHITECT:	RICHARD E. JOHNSON ARCHITECT, AIA	A-0.1	EXIST	
	1230—I COAST VILLAGE CIRCLE SANTA BARBARA, CA. 93108	A-0.2	CALIF	
	(805) 969–5910	A-0.3	CALIF	
	rejmgasb@gmail.com	A-0.4	PHOT	
STRUCTURAL ENGINEER:	KEVIN L. VANDERVORT STRUCTURAL ENGINEERING	A-0.4a	SOILS	
	250 STORKE ROAD SUITE 12 GOLETA, CA. 93117	A-0.5	SITE	
	(805) 562-8462	A-0.6	SITE	
	kevin@klvse.com	C1.0	CIVIL	
		C1.1	OVER,	
CIVIL ENGINEER:	MIKE GONES C.E. RCE #38168	C2.0	PARTI	
STORM WATER MANAGEMENT:	1219 1/2 LAGUNA STREET SANTA BARBARA, CA. 93101	C2.1	STOR	
	(805) 966-2259	C3.0	EROSI	
	mikegonesce@outlook.com	C4.0	DETAI	
		C5.0	DETAI	
ELECTRICAL ENGINEERING:	JMPE ELECTRICAL ENGINEERING	A1.0	PROP	
	JOHN MALONEY CA. REG. E13083	A1.1	PROP	
	627 OLIVE STREET SANTA BARBARA, CA. 93105	A2.0	PROP	
	(805) 569-9216	A3.0	PROP	
	MALONEY@JMPE.NET	A4.0	PROP	
		A5.0	PROP	
MECHANICAL ENGINEEDING.	MECHANICAL ENGINEERING CONSULTANTS, INC.	A6.0	ARCH	
MECHANICAL ENGINEERING: ENERGY/TITLE-24:	PAUL TZIOUVARAS	A6.1	ARCH	
,	315 E. CANON PERDIDO STREET SUITE "B" SANTA BARBARA, CA. 93101	A6.2	ARCH	
	(805) 957–4632	A6.3	ARCH	
	PAUL@MECENG.COM	A6.4	ARCH	
		A6.5	ARCH	
SOILS INVESTIGATIONS:	PACIFIC MATERIALS LABORATORY	E-1	ELEC1	
	RON PIKE 35-A SOUTH LA PATERA LANE	E-1.1	LIGHT	
	GOLETA, CA. 93116	E-2	POWE	
	(805) 964-6901 PML@PML.SBCOXMAIL.COM	E-3	LIGHT	
	SEE SHEET A-0.4 FOR SOILS RECOMENDATIONS	M1.1	MECH	
	LAB #: 115040-2	M1.2	MECH	
	FILE #: 16-11153-2	M1.3	T-24	
		M2.1	MECH	
		P1.1	PLUM	
		P2.1	PLUM	

047-10

	sheet index	DATE	REVISION
A-0	PROJECT INFORMATION	REV. 9-14-24	AEXT. COLO
A-0.1	EXISTING SITE PLAN		
A-0.2	CALIFORNIA GREEN BUILDING STANDARDS SHEET 1		
A-0.3	CALIFORNIA GREEN BUILDING STANDARDS SHEET 2		
A-0.4	PHOTOS & COLOR BOARD	REV. 9-14-24	AEXT. COLO
A-0.4a	SOILS RECOMMENDATIONS		
A-0.5	SITE PHOTOS		
A-0.6	SITE PHOTOS		
C1.0	CIVIL GENERAL NOTES		
C1.1	OVERALL SITE PLAN		
C2.0	PARTIAL SITE PLAN		
C2.1	STORM WATER CONTROL PLAN		
C3.0	EROSION CONTROL PLAN / SITE SECTIONS		
C4.0	DETAILS		
C5.0	DETAILS		
A1.0	PROPOSED OVERALL SITE PLAN		
A1.1	PROPOSED PARTIAL SITE PLAN		
A2.0	PROPOSED FLOOR PLAN & SCHEDULES	REV. 7-8-24	AWTR.HTR.
A3.0	PROPOSED ROOF PLAN & REFLECTED CEILING PLAN		
A4.0	PROPOSED EXTERIOR ELEVATIONS	REV. 9-14-24	AEXT. COLO
A5.0	PROPOSED SECTIONS		
A6.0	ARCHITECTURAL DETAILS		
A6.1	ARCHITECTURAL DETAILS		
A6.2	ARCHITECTURAL DETAILS		
A6.3	ARCHITECTURAL DETAILS		
A6.4	ARCHITECTURAL DETAILS		
A6.5	ARCHITECTURAL DETAILS		
E-1	ELECTRICAL GENERAL NOTES		
E-1.1	LIGHTING COMPLIANCE FORMS		
E-2	POWER PLAN		
E-3	LIGHTING PLAN & ELECTRICAL ROOF PLAN		
M1.1	MECHANICAL GENERAL NOTES	REV. 4-19-24	⚠ WTR.HTR.
M1.2	MECHANICAL SCHEDULES	REV. 4-19-24	MWTR.HTR.
M1.3	T-24 COMPLIANCE DOCUMENTS	REV. 4-19-24	MTR.HTR.
M2.1	MECHANICAL PLAN & DETAILS	REV. 4-19-24	MTR.HTR.
P1.1	PLUMBING SCHEDULES & SPECIFICATIONS	REV. 4-19-24	MTR.HTR.
P2.1	PLUMBING PLAN & DETAILS	REV. 4-19-24	MTR.HTR.
S1.1	GENERAL STRUCTURAL SPECIFICATIONS / NOTES		
S1.2	STANDARD STRUCTURAL DETAILS		
S1.3	SPECIFICATIONS		
S2	FOUNDATION PLAN		
S3	ROOF FRAMING PLAN		
S4	STRUCTURAL DETAILS		







POR. RANCHO LAS POSITAS Y CALERA

CODE COMPLIANCE

ASSESSOR PARCEL MAP

1. ALL WORK SHALL COMPLY WITH ALL OF THE FOLLOWING: 2019 CALIFORNIA RESIDENTIAL CODE

2019 CALIFORNIA MECHANICAL CODE 2019 CALIFORNIA ELECTRICAL CODE

2019 CALIFORNIA FIRE CODE 2019 CALIFORNIA ENERGY CODE (T-24) 2019 CALIFORNIA PLUMBING CODE

2019 CALIFORNIA GREEN BUILDING STANDARDS CODE 2019 CALIFORNIA BUILDING CODE

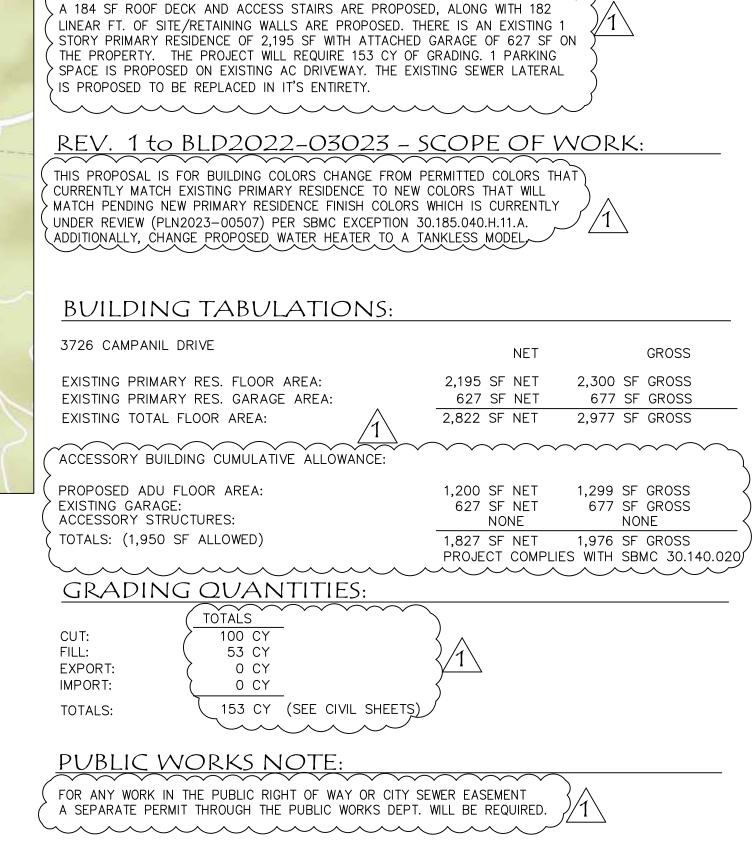
CITY OF SANTA BARBARA MUNICIPAL CODE AND ADOPTED ORDINANCES #5919 CRC SECTION R327 MATERIALS & CONSTRUCTION METHOD FOR EXT. WILDFIRE EXPOSURE.

SOLAR PANELS AT ROOF DECK WILL BE UNDER A SEPARATE PERMIT.

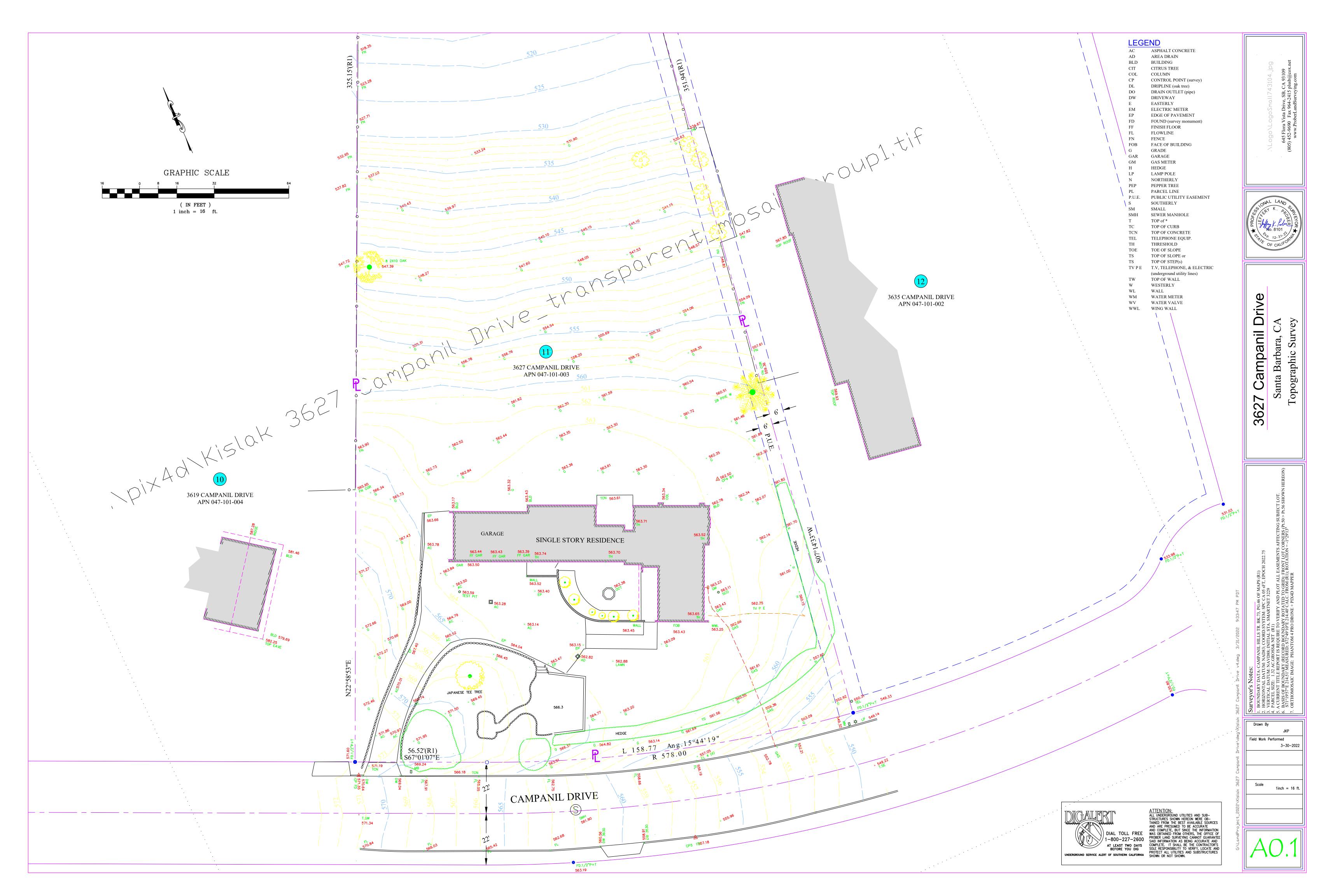
GENERAL NOTES:

PRIOR TO START OF CONSTRUCTION, THE CONTRACTOR SHALL SCHEDULE A PRECONSTRUCTION CONFERENCE WITH ALL PARTIES INVOLVED AT THE PROJECT SITE TO REVIEW THE SPECIAL INSPECTION REQUIREMENTS, PROCEDURES, AND INDIVIDUAL SPECIAL INSPECTORS THAT WILL BE ASSIGNED TO THE PROJECT, AS WELL AS REQUIREMENTS FOR STRUCTURAL OBSERVATION. CONTRACTOR SHALL CONTACT THE CITY OF SANTA BARBARA BUILDING DIVISION TO CONFIRM AN ACCEPTABLE MEETING DATE AND TIME. NO NATURAL GAS INFRASTRUCTURE TO SERVE ASSESSORY DWELLING UNIT PER

SECTION 22.110 OF THE CITY OF SANTA BARBARA MUNICIPAL CODE. THE NFRC THERMAL PERFORMANCE LABELS SHALL REMAIN ON THE WINDOWS AND/OR DOORS UNTIL FINAL INSPECTION.



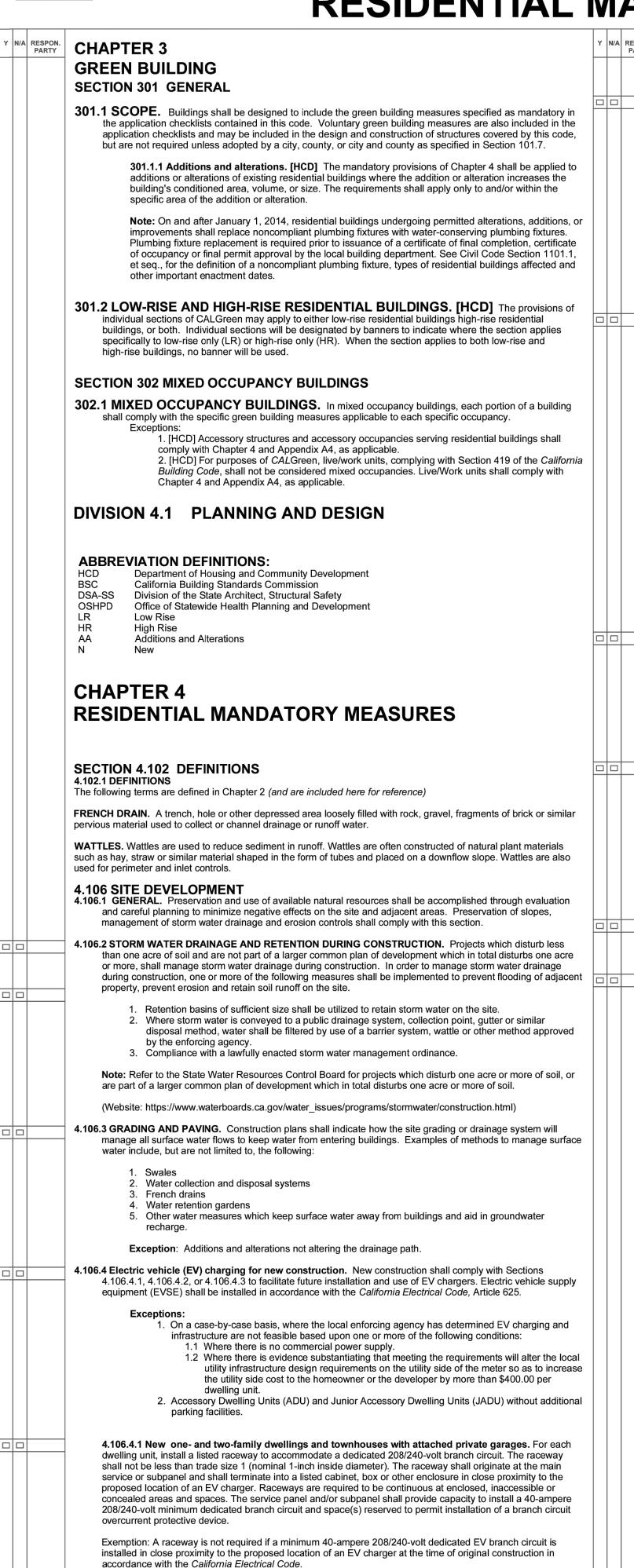
PROPOSED ACCESSORY DWELLING UNIT FOR: KISLAK RESIDENCE @ 3627 CAMPANIL DRIVE SANTA BARBARA CALIFORNIA 93109



California 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE

RESIDENTIAL MANDATORY MEASURES, SHEET 1 (July 2021, Includes July 2021 Supplement)

RESPONSIBLE PARTY (ie: ARCHITECT, ENGINEER



4.106.4.1.1 Identification. The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging as "EV CAPABLE". The raceway termination

location shall be permanently and visibly marked as "EV CAPABLE".

4.106.4.2 New multifamily dwellings. If residential parking is available, ten (10) percent of the total number of parking spaces on a building site, provided for all types of parking facilities, shall be electric vehicle charging spaces (EV spaces) capable of supporting future EVSE. Calculations for the required number of EV spaces shall be rounded up to the nearest whole number. 1. Construction documents are intended to demonstrate the project's capability and capacity for 2. There is no requirement for EV spaces to be constructed or available until EV chargers are installed 3. A parking space served by electric vehicle supply equipment or designated as a future EV charging space shall count as at least one standard automobile parking space for the purpose of complying with any applicable minimum parking space requirements established by a local jurisdiction. See Vehicle Code Section 22511.2 for further details. **4.106.4.2.1 Electric vehicle charging space (EV space) locations.** Construction documents shall indicate the location of proposed EV spaces. Where common use parking is provided at least one EV space shall be located in the common use parking area and shall be available for use by all residents. **4.106.4.2.1.1 Electric Vehicle Charging Stations (EVCS)** When EV chargers are installed, EV spaces required by Section 4.106.2.2, Item 3, shall comply with at least one of the following options: 1. The EV space shall be located adjacent to an accessible parking space meeting the requirements of the California Building Code, Chapter 11A, to allow use of the EV charger from the accessible parking space. 2. The EV space shall be located on an accessible route, as defined in the California Building Code, Chapter 2, to the building. **Exception:** Electric vehicle charging stations designed and constructed in compliance with the California Building Code, Chapter 11B, are not required to comply with Section 4.106.4.2.1.1 and Section 4.106.4.2.2, Item 3. Note: Electric Vehicle charging stations serving public housing are required to comply with the California 4.106.4.2.2 Electric vehicle charging space (EV space) dimensions. The EV space shall be designed to comply with the following: 1. The minimum length of each EV space shall be 18 feet (5486 mm). 2. The minimum width of each EV space shall be 9 feet (2743 mm). 3. One in every 25 EV spaces, but not less than one EV space, shall have an 8-foot (2438 mm) wide minimum aisle. A 5-foot (1524 mm) wide minimum aisle shall be permitted provided the minimum width of the EV space is 12 feet (3658 mm). a. Surface slope for this EV space and the aisle shall not exceed 1 unit vertical in 48 units horizontal (2.083 percent slope) in any direction. 4.106.4.2.3 Single EV space required. Install a listed raceway capable of accommodating a 208/240volt dedicated branch circuit. The raceway shall not be less than trade size 1 (nominal 1-inch inside diameter). The raceway shall originate at the main service or subpanel and shall terminate into a listed cabinet, box or enclosure in close proximity to the proposed location of the EV space. Construction documents shall identify the raceway termination point. The service panel and/or subpanel shall provide capacity to install a 40-ampere minimum dedicated branch circuit and space(s) reserved to permit installation of a branch circuit overcurrent protective device. Exemption: A raceway is not required if a minimum 40-ampere 208/240-volt dedicated EV branch circuit is installed in close proximity to the proposed location of an EV charger, at the time of original construction in accordance with the California Electrical Code. 4.106.4.2.4 Multiple EV spaces required. Construction documents shall indicate the raceway termination point and proposed location of future EV spaces and EV chargers. Construction documents shall also provide information on amperage of future EVSE, raceway method(s), wiring schematics and electrical load calculations to verify that the electrical panel service capacity and electrical system, including any on-site distribution transformer(s), have sufficient capacity to simultaneously charge all EVs at all required EV spaces at the full rated amperage of the EVSE. Plan design shall be based upon a 40-ampere minimum branch circuit. Required raceways and related components that are planned to be installed underground, enclosed, inaccessible or in concealed areas and spaces shall be installed at the time of original construction. Exemption: A raceway is not required if a minimum 40-ampere 208/240-volt dedicated EV branch circuit is installed in close proximity to the proposed location of an EV charger, at the time of original construction in accordance with the California Electrical Code. **4.106.4.2.5 Identification.** The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging purposes as "EV CAPABLE" in accordance with the California Electrical Code. **4.106.4.3 New hotels and motels.** All newly constructed hotels and motels shall provide EV spaces capable of supporting future installation of EVSE. The construction documents shall identify the location of the EV spaces. 1. Construction documents are intended to demonstrate the project's capability and capacity or facilitating future EV charging. 2. There is no requirement for EV spaces to be constructed or available until EV chargers 3. A parking space served by electrical vehicle supple equipment or designed as a future EV charging space shall count as at least one standard automobile parking space for the purpose of complying with any applicable minimum parking space requirements established by a local jurisdiction. See Vehicle Code Section 22511.2 for further details. **4.106.4.3.1 Number of required EV spaces.** The number of required EV spaces shall be based on the total number of parking spaces provided for all types of parking facilities in accordance with Table 4.106.4.3.1. Calculations for the required number of EV spaces shall be rounded up to the

TABLE 4.106.4.3.1	
TOTAL NUMBER OF PARKING SPACES	NUMBER OF REQUIRED EV SPACES
0-9	0
10-25	1
26-50	2
51-75	4
76-100	5
101-150	7
151-200	10
201 and over	6 percent of total

4.106.4.3.2 Electric vehicle charging space (EV space) dimensions. The EV spaces shall be designed to

1. The minimum length of each EV space shall be 18 feet (5486mm). 2. The minimum width of each EV space shall be 9 feet (2743mm)

4.106.4.3.3 Single EV space required. When a single EV space is required, the EV space shall be designed

4.106.4.3.4 Multiple EV spaces required. When multiple EV spaces are required, the EV spaces shall be designed in accordance with Section 4.106.4.2.4.

4.106.4.3.5 Identification. The service panels or sub-panels shall be identified in accordance with Section

4.106.4.3.6 Accessible EV spaces. In addition to the requirements in Section 4.106.4.3, EV spaces for hotels/motels and all EVSE, when installed, shall comply with the accessibility provisions for the EV charging stations in the California Building Code, Chapter 11B.

DIVISION 4.2 ENERGY EFFICIENCY

4.201.1 SCOPE. For the purposes of mandatory energy efficiency standards in this code, the California Energy Commission will continue to adopt mandatory standards.

DIVISION 4.3 WATER EFFICIENCY AND CONSERVATION

4.303 INDOOR WATER USE 4.303.1 WATER CONSERVING PLUMBING FIXTURES AND FITTINGS. Plumbing fixtures (water closets and urinals) and fittings (faucets and showerheads) shall comply with the sections 4.303.1.1, 4.303.1.2, 4.303.1.3,

Note: All noncompliant plumbing fixtures in any residential real property shall be replaced with water-conserving plumbing fixtures. Plumbing fixture replacement is required prior to issuance of a certificate of final completion, certificate of occupancy, or final permit approval by the local building department. See Civil Code Section 1101.1, et seq., for the definition of a noncompliant plumbing fixture, types of residential buildings affected and other important enactment dates.

4.303.1.1 Water Closets. The effective flush volume of all water closets shall not exceed 1.28 gallons per flush. Tank-type water closets shall be certified to the performance criteria of the U.S. EPA WaterSense Specification for Tank-type Toilets

Note: The effective flush volume of dual flush toilets is defined as the composite, average flush volume of two reduced flushes and one full flush.

4.303.1.2 Urinals. The effective flush volume of wall mounted urinals shall not exceed 0.125 gallons per flush. The effective flush volume of all other urinals shall not exceed 0.5 gallons per flush.

4.303.1.3 Showerheads.

4.303.1.3.1 Single Showerhead. Showerheads shall have a maximum flow rate of not more than 1.8 gallons per minute at 80 psi. Showerheads shall be certified to the performance criteria of the U.S. EPA WaterSense Specification for Showerheads.

4.303.1.3.2 Multiple showerheads serving one shower. When a shower is served by more than one showerhead, the combined flow rate of all the showerheads and/or other shower outlets controlled by a single valve shall not exceed 1.8 gallons per minute at 80 psi, or the shower shall be designed to only allow one shower outlet to be in operation at a time.

Note: A hand-held shower shall be considered a showerhead.

4.303.1.4 Faucets

4.303.1.4.1 Residential Lavatory Faucets. The maximum flow rate of residential lavatory faucets shall not exceed 1.2 gallons per minute at 60 psi. The minimum flow rate of residential lavatory faucets shall not be less than 0.8 gallons per minute at 20 psi.

4.303.1.4.2 Lavatory Faucets in Common and Public Use Areas. The maximum flow rate of lavatory faucets installed in common and public use areas (outside of dwellings or sleeping units) in residential buildings shall not exceed 0.5 gallons per minute at 60 psi.

4.303.1.4.3 Metering Faucets. Metering faucets when installed in residential buildings shall not deliver more than 0.2 gallons per cycle.

4.303.1.4.4 Kitchen Faucets. The maximum flow rate of kitchen faucets shall not exceed 1.8 gallons per minute at 60 psi. Kitchen faucets may temporarily increase the flow above the maximum rate, but not to exceed 2.2 gallons per minute at 60 psi, and must default to a maximum flow rate of 1.8 gallons per minute at 60 psi.

Note: Where complying faucets are unavailable, aerators or other means may be used to achieve

4.303.1.4.5 Pre-rinse spray valves. When installed, shall meet the requirements in the California Code of Regulations. Title 20 (Appliance Efficiency Regulations), Sections 1605.1 (h)(4) Table H-2, Section 1605.3 (h)(4)(A), and Section 1607 (d)(/) and shall be equipped with an integral automatic shutof

FOR REFERENCE ONLY: The following table and code section have been reprinted from the California Code of Regulations, Title 20 (Appliance Efficiency Regulations), Section 1605.1 (h)(4) and Section

TABLE H-2

STANDARDS FOR COMMERCIAL PRE-RINSE SPRAY VALUES MANUFACTURED ON OR AFTER JANUARY 28, 2019

PRODUCT CLASS [spray force in ounce force (ozf)]	MAXIMUM FLOW RATE (gpm)
Product Class 1 (≤ 5.0 ozf)	1.00
Product Class 2 (> 5.0 ozf and ≤ 8.0 ozf)	1.20
Product Class 3 (> 8.0 ozf)	1.28

Title 20 Section 1605.3 (h)(4)(A): Commercial prerinse spray values manufactured on or after January 1, 2006, shall have a minimum spray force of not less than 4.0 ounces-force (ozf)[113 grams-force(gf)]

4.303.2 Submeters for multifamily buildings and dwelling units in mixed-used residential/commercial

Submeters shall be installed to measure water usage of individual rental dwelling units in accordance with the California Plumbing Code.

4.303.3 Standards for plumbing fixtures and fittings. Plumbing fixtures and fittings shall be installed in accordance with the California Plumbing Code, and shall meet the applicable standards referenced in Table 1701.1 of the California Plumbing Code.

> THIS TABLE COMPILES THE DATA IN SECTION 4.303.1, AND IS INCLUDED AS A CONVENIENCE FOR THE USER.

TABLE - MAXIMUM FIXTUR	RE WATER USE
FIXTURE TYPE	FLOW RATE
SHOWER HEADS (RESIDENTIAL)	1.8 GMP @ 80 PSI
LAVATORY FAUCETS (RESIDENTIAL)	MAX. 1.2 GPM @ 60 PSI MIN. 0.8 GPM @ 20 PSI
LAVATORY FAUCETS IN COMMON & PUBLIC USE AREAS	0.5 GPM @ 60 PSI
KITCHEN FAUCETS	1.8 GPM @ 60 PSI
METERING FAUCETS	0.2 GAL/CYCLE
WATER CLOSET	1.28 GAL/FLUSH
URINALS	0.125 GAL/FLUSH

4.304 OUTDOOR WATER USE

4.304.1 OUTDOOR POTABLE WATER USE IN LANDSCAPE AREAS. Residential developments shall comply with a local water efficient landscape ordinance or the current California Department of Water Resources' Model Water Efficient Landscape Ordinance (MWELO), whichever is more stringent.

1. The Model Water Efficient Landscape Ordinance (MWELO) is located in the California Code Regulations, Title 23, Chapter 2.7, Division 2. MWELO and supporting documents, including water budget calculator, are available at: https://www.water.ca.gov/

DIVISION 4.4	MATERIAL CONSERVATION AND RESOURCE
EFFICIENCY	

4.406 ENHANCED DURABILITY AND REDUCED MAINTENANCE 4.406.1 RODENT PROOFING. Annular spaces around pipes, electric cables, conduits or other openings in sole/bottom plates at exterior walls shall be protected against the passage of rodents by closing such openings with cement mortar, concrete masonry or a similar method acceptable to the enforcing

4.408 CONSTRUCTION WASTE REDUCTION, DISPOSAL AND RECYCLING 4.408.1 CONSTRUCTION WASTE MANAGEMENT. Recycle and/or salvage for reuse a minimum of 65 percent of the non-hazardous construction and demolition waste in accordance with either Section

4.408.2, 4.408.3 or 4.408.4, or meet a more stringent local construction and demolition waste

management ordinance.

Excavated soil and land-clearing debris.

Alternate waste reduction methods developed by working with local agencies if diversion or recycle facilities capable of compliance with this item do not exist or are not located reasonably

3. The enforcing agency may make exceptions to the requirements of this section when isolated jobsites are located in areas beyond the haul boundaries of the diversion facility.

4.408.2 CONSTRUCTION WASTE MANAGEMENT PLAN. Submit a construction waste management plan in conformance with Items 1 through 5. The construction waste management plan shall be updated as necessary and shall be available during construction for examination by the enforcing agency.

1. Identify the construction and demolition waste materials to be diverted from disposal by recycling,

reuse on the project or salvage for future use or sale. Specify if construction and demolition waste materials will be sorted on-site (source separated) or

3. Identify diversion facilities where the construction and demolition waste material collected will be

4. Identify construction methods employed to reduce the amount of construction and demolition waste

Specify that the amount of construction and demolition waste materials diverted shall be calculated by weight or volume, but not by both.

4.408.3 WASTE MANAGEMENT COMPANY. Utilize a waste management company, approved by the enforcing agency, which can provide verifiable documentation that the percentage of construction and demolition waste material diverted from the landfill complies with Section 4.408.1

Note: The owner or contractor may make the determination if the construction and demolition waste materials will be diverted by a waste management company.

4.408.4 WASTE STREAM REDUCTION ALTERNATIVE [LR]. Projects that generate a total combined weight of construction and demolition waste disposed of in landfills, which do not exceed 3.4 lbs./sq.ft. of the building area shall meet the minimum 65% construction waste reduction requirement in

4.408.4.1 WASTE STREAM REDUCTION ALTERNATIVE. Projects that generate a total combined weight of construction and demolition waste disposed of in landfills, which do not exceed 2 pounds per square foot of the building area, shall meet the minimum 65% construction waste reduction requirement in Section 4.408.1

4.408.5 DOCUMENTATION. Documentation shall be provided to the enforcing agency which demonstrates compliance with Section 4.408.2, items 1 through 5, Section 4.408.3 or Section 4.408.4...

1. Sample forms found in "A Guide to the California Green Building Standards Code (Residential)" located at www.hcd.ca.gov/CALGreen.html may be used to assist in

documenting compliance with this section. 2. Mixed construction and demolition debris (C & D) processors can be located at the California

Department of Resources Recycling and Recovery (CalRecycle).

4.410 BUILDING MAINTENANCE AND OPERATION

4.410.1 OPERATION AND MAINTENANCE MANUAL. At the time of final inspection, a manual, compact disc, web-based reference or other media acceptable to the enforcing agency which includes all of the following shall be placed in the building:

1. Directions to the owner or occupant that the manual shall remain with the building throughout the life cycle of the structure.

Operation and maintenance instructions for the following: a. Equipment and appliances, including water-saving devices and systems, HVAC systems,

photovoltaic systems, electric vehicle chargers, water-heating systems and other major appliances and equipment

b. Roof and yard drainage, including gutters and downspouts. c. Space conditioning systems, including condensers and air filters.

d. Landscape irrigation systems.

e. Water reuse systems. Information from local utility, water and waste recovery providers on methods to further reduce resource consumption, including recycle programs and locations. Public transportation and/or carpool options available in the area.

5. Educational material on the positive impacts of an interior relative humidity between 30-60 percent and what methods an occupant may use to maintain the relative humidity level in that range. 6. Information about water-conserving landscape and irrigation design and controllers which conserve

. Instructions for maintaining gutters and downspouts and the importance of diverting water at least 5

feet away from the foundation 8. Information on required routine maintenance measures, including, but not limited to, caulking,

painting, grading around the building. etc. Information about state solar energy and incentive programs available.

10. A copy of all special inspections verifications required by the enforcing agency or this code. 11. Information from CAL Fire on maintenance of defensible space around residential structures.

4.410.2 RECYCLING BY OCCUPANTS. Where 5 or more multifamily dwelling units are constructed on a

building site, provide readily accessible area(s) that serves all buildings on the site and are identified for the depositing, storage and collection of non-hazardous materials for recycling, including (at a minimum) paper,

corrugated cardboard, glass, plastics, organic waster, and metals, or meet a lawfully enacted local recycling ordinance, if more restrictive. **Exception:** Rural jurisdictions that meet and apply for the exemption in Public Resources Code Section

42649.82 (a)(2)(A) et seq. are note required to comply with the organic waste portion of

DIVISION 4.5 ENVIRONMENTAL QUALITY

SECTION 4.501 GENERAL

The provisions of this chapter shall outline means of reducing the quality of air contaminants that are odorous, irritating and/or harmful to the comfort and well being of a building's installers, occupants and neighbors.

SECTION 4.502 DEFINITIONS 5.102.1 DEFINITIONS

The following terms are defined in Chapter 2 (and are included here for reference)

AGRIFIBER PRODUCTS. Agrifiber products include wheatboard, strawboard, panel substrates and door cores, not including furniture, fixtures and equipment (FF&E) not considered base building elements.

COMPOSITE WOOD PRODUCTS. Composite wood products include hardwood plywood, particleboard and

medium density fiberboard. "Composite wood products" does not include hardboard, structural plywood, structural panels, structural composite lumber, oriented strand board, glued laminated timber, prefabricated wood I-joists or finger-jointed lumber, all as specified in California Code of regulations (CCR), title 17, Section

DIRECT-VENT APPLIANCE. A fuel-burning appliance with a sealed combustion system that draws all air for combustion from the outside atmosphere and discharges all flue gases to the outside atmosphere.

SH IDEN

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California 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE MANDATORY MEASURES, SHEET 1 (July 2021, Includes July 2021 Supplement)

CHAPTER 7

			RESIDENTIAL	M
Υ	N/A	RESPON. PARTY		Y N//
			MAXIMUM INCREMENTAL REACTIVITY (MIR). The maximum change in weight of ozone formed by adding a compound to the "Base Reactive Organic Gas (ROG) Mixture" per weight of compound added, expressed to hundredths of a gram (g O³/g ROC). Note: MIR values for individual compounds and hydrocarbon solvents are specified in CCR, Title 17, Sections 94700 and 94701. MOISTURE CONTENT. The weight of the water in wood expressed in percentage of the weight of the oven-dry wood. PRODUCT-WEIGHTED MIR (PWMIR). The sum of all weighted-MIR for all ingredients in a product subject to this article. The PWMIR is the total product reactivity expressed to hundredths of a gram of ozone formed per gram of	
			product (excluding container and packaging). Note: PWMIR is calculated according to equations found in CCR, Title 17, Section 94521 (a). REACTIVE ORGANIC COMPOUND (ROC). Any compound that has the potential, once emitted, to contribute to ozone formation in the troposphere.	
			VOC. A volatile organic compound (VOC) broadly defined as a chemical compound based on carbon chains or rings with vapor pressures greater than 0.1 millimeters of mercury at room temperature. These compounds typically contain hydrogen and may contain oxygen, nitrogen and other elements. See CCR Title 17, Section 94508(a).	
			4.503 FIREPLACES 4.503.1 GENERAL . Any installed gas fireplace shall be a direct-vent sealed-combustion type. Any installed woodstove or pellet stove shall comply with U.S. EPA New Source Performance Standards (NSPS) emission limits as applicable, and shall have a permanent label indicating they are certified to meet the emission limits. Woodstoves, pellet stoves and fireplaces shall also comply with applicable local ordinances.	
			4.504 POLLUTANT CONTROL 4.504.1 COVERING OF DUCT OPENINGS & PROTECTION OF MECHANICAL EQUIPMENT DURING CONSTRUCTION. At the time of rough installation, during storage on the construction site and until final startup of the heating, cooling and ventilating equipment, all duct and other related air distribution component openings shall be covered with tape, plastic, sheet metal or other methods acceptable to the enforcing agency to reduce the amount of water, dust or debris which may enter the system.	
巨			4.504.2 FINISH MATERIAL POLLUTANT CONTROL. Finish materials shall comply with this section.	
			4.504.2.1 Adhesives, Sealants and Caulks. Adhesives, sealant and caulks used on the project shall meet the requirements of the following standards unless more stringent local or regional air pollution or air quality management district rules apply:	
			1. Adhesives, adhesive bonding primers, adhesive primers, sealants, sealant primers and caulks shall comply with local or regional air pollution control or air quality management district rules where applicable or SCAQMD Rule 1168 VOC limits, as shown in Table 4.504.1 or 4.504.2, as applicable. Such products also shall comply with the Rule 1168 prohibition on the use of certain toxic compounds (chloroform, ethylene dichloride, methylene chloride, perchloroethylene and tricloroethylene), except for aerosol products, as specified in Subsection 2 below.	
			 Aerosol adhesives, and smaller unit sizes of adhesives, and sealant or caulking compounds (in units of product, less packaging, which do not weigh more than 1 pound and do not consist of more than 16 fluid ounces) shall comply with statewide VOC standards and other requirements, including prohibitions on use of certain toxic compounds, of <i>California Code of Regulations</i>, Title 17, commencing with section 94507. 	
			4.504.2.2 Paints and Coatings . Architectural paints and coatings shall comply with VOC limits in Table 1 of the ARB Architectural Suggested Control Measure, as shown in Table 4.504.3, unless more stringent local limits apply. The VOC content limit for coatings that do not meet the definitions for the specialty coatings categories listed in Table 4.504.3 shall be determined by classifying the coating as a Flat, Nonflat or Nonflat-High Gloss coating, based on its gloss, as defined in subsections 4.21, 4.36, and 4.37 of the 2007 California Air Resources Board, Suggested Control Measure, and the corresponding Flat, Nonflat or Nonflat-High Gloss VOC limit in Table 4.504.3 shall apply.	
			4.504.2.3 Aerosol Paints and Coatings. Aerosol paints and coatings shall meet the Product-weighted MIR Limits for ROC in Section 94522(a)(2) and other requirements, including prohibitions on use of certain toxic compounds and ozone depleting substances, in Sections 94522(e)(1) and (f)(1) of <i>California Code of Regulations</i> , Title 17, commencing with Section 94520; and in areas under the jurisdiction of the Bay Area Air Quality Management District additionally comply with the percent VOC by weight of product limits of Regulation 8, Rule 49.	

4.504.2.4 Verification. Verification of compliance with this section shall be provided at the request of the enforcing agency. Documentation may include, but is not limited to, the following:

1. Manufacturer's product specification. 2. Field verification of on-site product containers.

TABLE 4.504.1 - ADHESIVE VOC LIM	$IT_{1,2}$
(Less Water and Less Exempt Compounds in Gram	s per Liter)
ARCHITECTURAL APPLICATIONS	VOC LIMIT
INDOOR CARPET ADHESIVES	50
CARPET PAD ADHESIVES	50
OUTDOOR CARPET ADHESIVES	150
WOOD FLOORING ADHESIVES	100
RUBBER FLOOR ADHESIVES	60
SUBFLOOR ADHESIVES	50
CERAMIC TILE ADHESIVES	65
VCT & ASPHALT TILE ADHESIVES	50
DRYWALL & PANEL ADHESIVES	50
COVE BASE ADHESIVES	50
MULTIPURPOSE CONSTRUCTION ADHESIVE	70
STRUCTURAL GLAZING ADHESIVES	100
SINGLE-PLY ROOF MEMBRANE ADHESIVES	250
OTHER ADHESIVES NOT LISTED	50
SPECIALTY APPLICATIONS	
PVC WELDING	510
CPVC WELDING	490
ABS WELDING	325
PLASTIC CEMENT WELDING	250
ADHESIVE PRIMER FOR PLASTIC	550
CONTACT ADHESIVE	80
SPECIAL PURPOSE CONTACT ADHESIVE	250
STRUCTURAL WOOD MEMBER ADHESIVE	140
TOP & TRIM ADHESIVE	250
SUBSTRATE SPECIFIC APPLICATIONS	
METAL TO METAL	30
PLASTIC FOAMS	50
POROUS MATERIAL (EXCEPT WOOD)	50
WOOD	30
FIBERGLASS	80

THE ADHESIVE WITH THE HIGHEST VOC CONTENT SHALL BE ALLOWED.

2. FOR ADDITIONAL INFORMATION REGARDING METHODS TO MEASURE

THE VOC CONTENT SPECIFIED IN THIS TABLE, SEE SOUTH COAST AIR

QUALITY MANAGEMENT DISTRICT RULE 1168.

SEALANTS	VOC LIMIT
ARCHITECTURAL	250
MARINE DECK	760
NONMEMBRANE ROOF	300
ROADWAY	250
SINGLE-PLY ROOF MEMBRANE	450
OTHER	420
SEALANT PRIMERS	
ARCHITECTURAL	
NON-POROUS	250
POROUS	775
MODIFIED BITUMINOUS	500
MARINE DECK	760
OTHER	750

TABLE 4.504.3 - VOC CONTENT LIMITS FOR

COMPOUNDS	WATER & LESS EXEMPT
COATING CATEGORY	VOC LIMIT
FLAT COATINGS	50
NON-FLAT COATINGS	100
NONFLAT-HIGH GLOSS COATINGS	150
SPECIALTY COATINGS	
ALUMINUM ROOF COATINGS	400
BASEMENT SPECIALTY COATINGS	400
BITUMINOUS ROOF COATINGS	50
BITUMINOUS ROOF PRIMERS	350
BOND BREAKERS	350
CONCRETE CURING COMPOUNDS	350
CONCRETE/MASONRY SEALERS	100
DRIVEWAY SEALERS	50
DRY FOG COATINGS	150
FAUX FINISHING COATINGS	350
FIRE RESISTIVE COATINGS	350
FLOOR COATINGS	100
FORM-RELEASE COMPOUNDS	250
GRAPHIC ARTS COATINGS (SIGN PAINTS)	500
HIGH TEMPERATURE COATINGS	420
INDUSTRIAL MAINTENANCE COATINGS	250
LOW SOLIDS COATINGS ₁	120
MAGNESITE CEMENT COATINGS	450
MASTIC TEXTURE COATINGS	100
METALLIC PIGMENTED COATINGS	500
MULTICOLOR COATINGS	250
PRETREATMENT WASH PRIMERS	420
PRIMERS, SEALERS, & UNDERCOATERS	100
REACTIVE PENETRATING SEALERS	350
RECYCLED COATINGS	250
ROOF COATINGS	50
RUST PREVENTATIVE COATINGS	250
SHELLACS	
CLEAR	730
OPAQUE	550
SPECIALTY PRIMERS, SEALERS & UNDERCOATERS	100
STAINS	250
STONE CONSOLIDANTS	450
SWIMMING POOL COATINGS	340
TRAFFIC MARKING COATINGS	100
TUB & TILE REFINISH COATINGS	420
WATERPROOFING MEMBRANES	250
WOOD COATINGS	275
WOOD PRESERVATIVES	350

1. GRAMS OF VOC PER LITER OF COATING, INCLUDING WATER & EXEMPT COMPOUNDS 2. THE SPECIFIED LIMITS REMAIN IN EFFECT UNLESS REVISED LIMITS

ARE LISTED IN SUBSEQUENT COLUMNS IN THE TABLE.

3. VALUES IN THIS TABLE ARE DERIVED FROM THOSE SPECIFIED BY THE CALIFORNIA AIR RESOURCES BOARD, ARCHITECTURAL COATINGS SUGGESTED CONTROL MEASURE, FEB. 1, 2008. MORE INFORMATION IS AVAILABLE FROM THE AIR RESOURCES BOARD.

TABLE 4.504.5 - FORMALDEHYDE LIMITS	
MAXIMUM FORMALDEHYDE EMISSIONS IN PA	RTS PER MILLION
PRODUCT	CURRENT LIMIT
HARDWOOD PLYWOOD VENEER CORE	0.05
HARDWOOD PLYWOOD COMPOSITE CORE	0.05
PARTICLE BOARD	0.09
MEDIUM DENSITY FIBERBOARD	0.11
THIN MEDIUM DENSITY FIBERBOARD2	0.13

1. VALUES IN THIS TABLE ARE DERIVED FROM THOSE SPECIFIED BY THE CALIF. AIR RESOURCES BOARD, AIR TOXICS CONTROL MEASURE FOR COMPOSITE WOOD AS TESTED IN ACCORDANCE WITH ASTM E 1333. FOR ADDITIONAL INFORMATION, SEE CALIF. CODE OF REGULATIONS, TITLE 17, SECTIONS 93120 THROUGH

2. THIN MEDIUM DENSITY FIBERBOARD HAS A MAXIMUM THICKNESS OF 5/16" (8 MM).

DIVISION 4.5 ENVIRONMENTAL QUALITY (continued) 4.504.3 CARPET SYSTEMS. All carpet installed in the building interior shall meet the requirements of the California Department of Public Health, "Standard Method for the Testing and Evaluation of Volatile Organic Chemical Emissions from Indoor Sources Using Environmental Chambers," Version 1.2, January 2017 (Emission testing method for

See California Department of Public Health's website for certification programs and testing labs.

https://www.cdph.ca.gov/Programs/CCDPHP/DEODC/EHLB/IAQ/Pages/VOC.aspx.

https://www.cdph.ca.gov/Programs/CCDPHP/DEODC/EHLB/IAQ/Pages/VOC.aspx.

4.504.3.1 Carpet cushion. All carpet cushion installed in the building interior shall meet the requirements of the California Department of Public Health, "Standard Method for the Testing and Evaluation of Volatile Organic Chemical Emissions from Indoor Sources Using Environmental Chambers," Version 1.2, January 2017 (Emission testing method for California Specification 01350)

See California Department of Public Health's website for certification programs and testing labs.

4.504.3.2 Carpet adhesive. All carpet adhesive shall meet the requirements of Table 4.504.1.

4.504.4 RESILIENT FLOORING SYSTEMS. Where resilient flooring is installed, at least 80% of floor area receiving resilient flooring shall meet the requirements of the California Department of Public Health, "Standard Method for the Testing and Evaluation of Volatile Organic Chemical Emissions from Indoor Sources Using Environmental Chambers,' Version 1.2, January 2017 (Emission testing method for California Specification 01350)

See California Department of Public Health's website for certification programs and testing labs.

hhtps://www.cdph.ca.gov/Programs/CCDPHP/DEODC/EHLB/IAQ/Pages/VOC.aspx

4.504.5 COMPOSITE WOOD PRODUCTS. Hardwood plywood, particleboard and medium density fiberboard composite wood products used on the interior or exterior of the buildings shall meet the requirements for formaldehyde as specified in ARB's Air Toxics Control Measure for Composite Wood (17 CCR 93120 et seq.), by or before the dates specified in those sections, as shown in Table 4.504.5

4.504.5.1 Documentation. Verification of compliance with this section shall be provided as requested by the enforcing agency. Documentation shall include at least one of the following:

- 1. Product certifications and specifications.
- Chain of custody certifications. 3. Product labeled and invoiced as meeting the Composite Wood Products regulation (see CCR, Title 17, Section 93120, et seq.).
- 4. Exterior grade products marked as meeting the PS-1 or PS-2 standards of the Engineered Wood Association, the Australian AS/NZS 2269, European 636 3S standards, and Canadian CSA 0121, CSA 0151, CSA 0153 and CSA 0325 standards.
- Other methods acceptable to the enforcing agency.

4.505 INTERIOR MOISTURE CONTROL **4.505.1 General.** Buildings shall meet or exceed the provisions of the *California Building Standards Code*.

4.505.2 CONCRETE SLAB FOUNDATIONS. Concrete slab foundations required to have a vapor retarder by California Building Code, Chapter 19, or concrete slab-on-ground floors required to have a vapor retarder by the California Residential Code, Chapter 5, shall also comply with this section.

4.505.2.1 Capillary break. A capillary break shall be installed in compliance with at least one of the

- 1. A 4-inch (101.6 mm) thick base of 1/2 inch (12.7mm) or larger clean aggregate shall be provided with a vapor barrier in direct contact with concrete and a concrete mix design, which will address bleeding, shrinkage, and curling, shall be used. For additional information, see American Concrete Institute, ACI 302.2R-06.
- 2. Other equivalent methods approved by the enforcing agency. 3. A slab design specified by a licensed design professional.

4.505.3 MOISTURE CONTENT OF BUILDING MATERIALS. Building materials with visible signs of water damage shall not be installed. Wall and floor framing shall not be enclosed when the framing members exceed 19 percent moisture content. Moisture content shall be verified in compliance with the following:

- 1. Moisture content shall be determined with either a probe-type or contact-type moisture meter. Equivalent moisture verification methods may be approved by the enforcing agency and shall satisfy requirements
- found in Section 101.8 of this code.
- 2. Moisture readings shall be taken at a point 2 feet (610 mm) to 4 feet (1219 mm) from the grade stamped end 3. At least three random moisture readings shall be performed on wall and floor framing with documentation
- acceptable to the enforcing agency provided at the time of approval to enclose the wall and floor framing.

Insulation products which are visibly wet or have a high moisture content shall be replaced or allowed to dry prior to enclosure in wall or floor cavities. Wet-applied insulation products shall follow the manufacturers' drying recommendations prior to enclosure.

4.506 INDOOR AIR QUALITY AND EXHAUST **4.506.1 Bathroom exhaust fans.** Each bathroom shall be mechanically ventilated and shall comply with the

- 1. Fans shall be ENERGY STAR compliant and be ducted to terminate outside the building. 2. Unless functioning as a component of a whole house ventilation system, fans must be controlled by a
- a. Humidity controls shall be capable of adjustment between a relative humidity range less than or equal to 50% to a maximum of 80%. A humidity control may utilize manual or automatic means of
- b. A humidity control may be a separate component to the exhaust fan and is not required to be integral (i.e., built-in)

following:

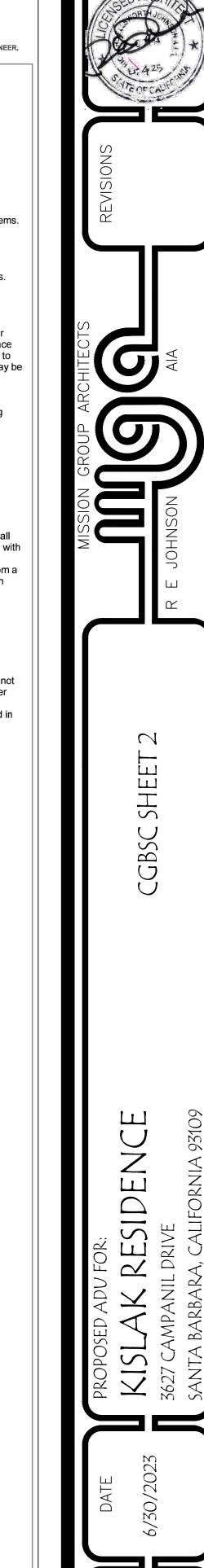
- 1. For the purposes of this section, a bathroom is a room which contains a bathtub, shower or
- 2. Lighting integral to bathroom exhaust fans shall comply with the California Energy Code.

4.507 ENVIRONMENTAL COMFORT 4.507.2 HEATING AND AIR-CONDITIONING SYSTEM DESIGN. Heating and air conditioning systems shall be

- sized, designed and have their equipment selected using the following methods: 1. The heat loss and heat gain is established according to ANSI/ACCA 2 Manual J - 2011 (Residential
- Load Calculation), ASHRAE handbooks or other equivalent design software or methods. 2. Duct systems are sized according to ANSI/ACCA 1 Manual D - 2014 (Residential Duct Systems). ASHRAE handbooks or other equivalent design software or methods. 3. Select heating and cooling equipment according to ANSI/ACCA 3 Manual S - 2014 (Residential
- **Exception:** Use of alternate design temperatures necessary to ensure the system functions are acceptable.

Equipment Selection), or other equivalent design software or methods.

RESPONSIBLE PARTY (ie: ARCHITECT, ENGINEER, OWNER, CONTRACTOR, INSPECTOR ETC.) **INSTALLER & SPECIAL INSPECTOR QUALIFICATIONS 702 QUALIFICATIONS 702.1 INSTALLER TRAINING.** HVAC system installers shall be trained and certified in the proper installation of HVAC systems including ducts and equipment by a nationally or regionally recognized training or certification program. Uncertified persons may perform HVAC installations when under the direct supervision and responsibility of a person trained and certified to install HVAC systems or contractor licensed to install HVAC systems. Examples of acceptable HVAC training and certification programs include but are not limited to the following: 1. State certified apprenticeship programs. 2. Public utility training programs. 3. Training programs sponsored by trade, labor or statewide energy consulting or verification organizations. 4. Programs sponsored by manufacturing organizations. 5. Other programs acceptable to the enforcing agency. **702.2 SPECIAL INSPECTION [HCD].** When required by the enforcing agency, the owner or the responsible entity acting as the owner's agent shall employ one or more special inspectors to provide inspection or other duties necessary to substantiate compliance with this code. Special inspectors shall demonstrate competence to the satisfaction of the enforcing agency for the particular type of inspection or task to be performed. In addition to other certifications or qualifications acceptable to the enforcing agency, the following certifications or education may be considered by the enforcing agency when evaluating the qualifications of a special inspector: Certification by a national or regional green building program or standard publisher. 2. Certification by a statewide energy consulting or verification organization, such as HERS raters, building performance contractors, and home energy auditors. Successful completion of a third party apprentice training program in the appropriate trade. 4. Other programs acceptable to the enforcing agency. 1. Special inspectors shall be independent entities with no financial interest in the materials or the project they are inspecting for compliance with this code. 2. HERS raters are special inspectors certified by the California Energy Commission (CEC) to rate homes in California according to the Home Energy Rating System (HERS). [BSC] When required by the enforcing agency, the owner or the responsible entity acting as the owner's agent shall employ one or more special inspectors to provide inspection or other duties necessary to substantiate compliance with this code. Special inspectors shall demonstrate competence to the satisfaction of the enforcing agency for the particular type of inspection or task to be performed. In addition, the special inspector shall have a certification from a recognized state, national or international association, as determined by the local agency. The area of certification shall be closely related to the primary job function, as determined by the local agency. Note: Special inspectors shall be independent entities with no financial interest in the materials or the project they are inspecting for compliance with this code. 703 VERIFICATIONS 703.1 DOCUMENTATION. Documentation used to show compliance with this code shall include but is not limited to, construction documents, plans, specifications, builder or installer certification, inspection reports, or other methods acceptable to the enforcing agency which demonstrate substantial conformance. When specific documentation or special inspection is necessary to verify compliance, that method of compliance will be specified in the appropriate section or identified applicable checklist.



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SOUTH SIDE EXISTING RESIDENCE TO BE DEMOLISHED



NORTH SIDE EXISTING RESIDENCE TO BE DEMOLISHED



CONCEPTUAL RENDERING OF PENDING PROPOSED RESIDENCE (PLN2023-00507)

MATERIALS LEGEND:

NOTE: VERIFY ALL COLORS AND MATERIALS WITH OWNER PRIOR TO ORDERING

(1) ROOFING:

TWO PIECE MISSION TILE BY REDLANDS CLAY TILE UL CLASS "A" FIRE RATING ICC: ESR-4395
COLOR - 3 TILE CUSTOM BLEND
40% LIGHT TILE, 30% & 30% DARK TILES

2 EXTERIOR PLASTER:

7/8" SMOOTH TROWEL EXTERIOR PLASTER OVER MTL. LATH. & TYVEK HOMEWRAP TYPICAL COLOR: CUSTOM COLOR AS SHOWN (SOFT PINK)

BEIGE TRAVERTINE VENEER OVER SCRATCH COAT/GAL. LATH OVER TYVEK WATERPROOF BARRIER OVER PLYWD. 3 STONE VENEER: FLOORING TO BE CHEROKEE SANDSTONE

4 CARVED STONE TRIM: BEIGE TRAVERTINE VENEER (SEE ABOVE)

STEEL WINDOWS & DOORS BY MIRAMAR MFG. INC ALL GLASS TO MEET CRITERIA SET FORTH IN ENERGY CALC. DUAL GLAZED w/ MAX. U-FACTOR OF .35 & MAX. SHGC OF OF .35 COLOR: BLACK 5 DOORS & WDWS.:

20 GA. MIN. COPPER GUTTERS AND FLASHING NEW GUTTERS TO HAVE DEBRIS GUARD 6 GUTTERS & FLASHING:

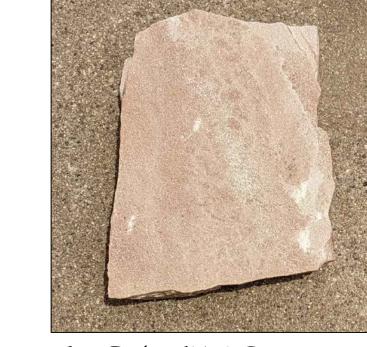
7 CUSTOM WROUGHT IRON: DESIGN TO BE DETERMINED
DESIGN SHALL COMPLY WITH [CRC SECTION R312]
SEE DETAIL 2/A6.2 FOR GENERAL NOTES

AB EUROPA COLLECTION

5 DOORS & WINDOWS



2 EXT. PLASTER



NT OF RICHARD E JOHNSON A.I.A

3 PAVING



ROOF TILE



3 VENEER & TRIM



OVERALL PROPOSED MATERIALS BOARD

ADJACENT LOADS

surcharge loads.

APPURTENANCES

December 22, 2016

Lab No: 115040-2

Lab No: 115040-2

File No: 16-11153-2

This report presents the results of a preliminary geotechnical investigation performed at

3627 Campanil Drive, in the County of Santa Barbara, California. Existing at the site is a

single-family residence. It is proposed to build a swimming pool in the backyard. It is also proposed to build additions around the perimeter of the existing footprint at selected locations.

The site is level as the result of a previous grading operation. The level pad transitions to

It is the purpose of this investigation to classify the soil disclosed by the exploratory

borings by observation and tests on selected samples. In addition, this study includes

laboratory tests to evaluate soil strength, the effect of moisture variation on the soil-bearing

capacity, compressibility, liquefaction, and expansiveness. Based upon this information, we

the soil, previous site construction, or analysis of geologic structures and their associated features, such as faults, fractures, bedding planes, strike and dip angles, ancient landslides,

potential for earth movement in undisturbed or natural soil formations sloped or level, or other

sources of potential instability which relate to the geologic conditions, as these items should be

This exploration was conducted in accordance with presently accepted geotechnical

appropriate geotechnical design characteristics of the foundations soils and of the proposed fill soils in order to properly evaluate the proposed structure with respect to differential settlement

This Laboratory's basic assumption is that the soil borings presented herein are

implied. If, during the course of construction, soil conditions are encountered which vary from

It is your responsibility to contact our office, providing at least 48 hours of notice for

grading or footing excavation observations and testing. The observation of excavations during

the construction phase represents an opportunity by our firm to either confirm soil conditions

estimated by the exploratory borings or to discover soil conditions which have not been

December 22, 2016 Pacific Materials Laborato by of Santa Barbara, Inc.

addressed. When such undisclosed conditions are encountered, opinions and

This report is considered preliminary and no person should consider the

grading and foundation works accomplished and installed are appropriate for the soil

recommendations or soil conditions described herein as conclusive. The recommendations

observed during the construction phase, after which a final report will be issued stating that the

The subsurface soil conditions were explored by four truck-mounted auger borings,

which were drilled to depths of up to 50 feet. The locations of the borings were selected as

appropriate and representative. Representative relatively "undisturbed" tube soil samples

were obtained during the drilling operation by the Modified California sampler method.

Laboratory tests and analysis of representative soil samples, obtained during the drilling

operation, were performed to estimate the engineering properties and determine the soil

approximate and have not been located by surveyed measurements. The boring log data is

No groundwater was encountered in the exploratory borings that extended to

depths of up to 50 feet. It should be recognized that water table elevations, even

seasonal perched water tables, might fluctuate with time, being dependent upon

investigation may vary from those encountered during the construction phase of

the project. The evaluation of such factors is beyond the scope of this report.

The black clay topsoil of the Rincon Formation was encountered at a depth of

40 feet in Boring No. 1, indicating approximately 40 feet of fill was placed to

The results of the Consolidation Tests indicate the soil is subject to swell with

create the level pad. The Rincon Shale was encountered at a depth of 50 feet.

The soil profile at this site is judged to be stiff soil corresponding to a Site Class

estimate is based on the borings, which encountered the geologic formation

known as the Rincon Formation , which is widely regarded as a Type D soil

It is the opinion of this Laboratory the proposed construction is feasible from a soil-

The clay soil supporting the existing structure is expansive. As the moisture content

It is the understanding of this Laboratory the proposed additions will be one-story wood

frame structures. The foundation recommendations for the additions, which follow, conform to

the California Building Code (CBC) requirements for the construction of foundations placed on

engineering perspective provided the recommendations contained in this soil engineering report are incorporated into the design and implemented during construction, and provided the

changes from season to season, the soils supporting the foundation are expected to shrink

and expand. The existing single-story house appears to be experiencing these phenomena,

D as defined by Table 1613.5.2 of the 2013 California Building Code (CBC). This

profile since the Standard Penetration Resistance typically results in blow counts

seasonal precipitation, irrigation, land use, and climatic conditions, as well as

other factors. Therefore, water level observations at the time of the field

The soil profile consists of the dark brown clay of the Rincon Formation,

4. The supporting soil was found to have a very high potential for expansion.

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presented in Appendix A, "Field Investigation", while the results of the laboratory tests are

classification. The locations of the borings are shown on Plate 1; these locations are

and conclusions of this report are considered preliminary until all excavations have been

recommendations addressing these conditions will be rendered at that time.

representative of the entire footprint of the proposed development, however, no warranty is

those presented herein, please contact this Laboratory immediately so appropriate field

engineering procedures currently applied in the local community in order to provide the

based upon the anticipated soil characteristics at the time of construction.

will provide preliminary foundation recommendations for the proposed additions and swimming

The scope of this investigation does not include the analysis of the corrosive potential of

descending slopes on the north, south, and east sides.

addressed by a qualified Engineering Geologist.

modifications may be expeditiously proposed.

December 22, 2016

SCOPE OF WORK

LIMITATIONS

conditions encountered.

FIELD INVESTIGATION

SOIL CONDITIONS

December 22, 2016

expansive soil.

provided in Appendix B, "Laboratory Tests".

underlain by the Rincon Shale.

increased moisture content.

having a range of between 15 to 50.

PRELIMINARY CONCLUSIONS AND RECOMMENDATIONS

based on my observation of cracks in the surfaces of the structure.

7. The potential for liquefaction is considered to be very low.

owner and future owners understand certain foundation performance limitations.

NT OF RICHARD E JOHNSON A.I.A

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provided in this report for the proposed additions to the house because it is assumed the owners will not want to go to the expense of using a pile foundation throughout the old house and the new addition, but rather tolerate the cracks in the surfaces of walls,

December 22, 2016

floors, and ceilings both in the existing and in the proposed structure. If this is not correct and pile recommendations for the house foundation are wanted, please notify

Based upon this understanding, we present the following preliminary recommendations: **FOUNDATIONS**

- The supporting soils were found to be expansive based on the current CBC. Such soil conditions define the soil as being expansive and require the foundation be designed in compliance with Section 1808.6.2. Based on this understanding, we recommend the following:
- A mat foundation shall be designed to conform to Section 1808.6.2, using the Wire Reinforcement Institute (WRI) Method. The Project Structural Engineer shall perform the calculation to design the foundation for the addition using the WRL Manual. If the dimensions of the slab, footings, and tie beams are less than the minimum dimensions specified in this section, the larger dimensions shall govern. The same is true for the steel reinforcement.
- The foundation recommendations which follow are recommended minimums. The actual foundation may exceed these minimums depending on the results of
- the design using the WRI Method. All continuous exterior footings of the structure shall extend a minimum of 36 inches and all continuous interior footings shall extend a minimum distance of
- 24 inches below the compacted pad grade. 4. Due to the expansion potential of the soil, the Project Structural Engineer shall utilize Section 1808.6.2 of the CBC for the design of the concrete slab-on-grade floor, foundation, and the interior tie beams using the WRI Method. The effective Plasticity Index (P.I. x C_S x C₀) shall be assumed to be 46. As a minimum, a network of tie beams shall be placed in two directions, perpendicular to each other and at a spacing of approximately 15 feet on center each way. The tie beams shall be a minimum of 20 inches deep below the top of the concrete slabon-grade floor and shall be a minimum of 12 inches wide, with horizontal rebar placed near the top and near the bottom of the tie beam. The tie beam shall be doweled into the concrete slab-on-grade floor with a minimum of No. 3 rebar at
- All footings shall contain a minimum of two No. 4 horizontal rebar placed one in the base and one in the stem of the footing. The Project Civil or Structural Engineer shall specify the foundation steel reinforcement based on either the WRI method and/or as required to resist the imposed loads.

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18 inches on center

Interior isolated spread footings may not be utilized unless connected on 4 sides, where possible, to tie beams. These footings shall extend a minimum of 24 inches below pad grade on the interior and 36 inches below the lowest

Lab No: 115040-2

- adjacent ground surface on the exterior. As a minimum, concrete slabs on grade shall be a full 6 inches thick and shall contain No. 3 rebar spaced 18 inches on center each way. The steel reinforcement shall be placed near the center of the slab. The slab shall be underlain with a minimum 4-inch coarse washed concrete sand layer. A 10-mil or heavier impervious membrane shall cover a sand layer. These concrete slabon-grade requirements shall be modified as needed by the designers for surcharge loads, wheel loads, concentrated loads, or for moisture control. The floor covering supplier or manufacturer should be contacted for their specifications for design features which will result in a successful bond between the concrete slab and floor covering. Floor flatness and shrinkage crack control must be addressed by a competent contractor experienced in the skill of concrete placement. The owners or their agents shall inform those designing, building, and installing the concrete slab on grade and flooring of the performance and
- Concrete slabs on grade shall be doweled into all adjacent footings using No. 3
- ebar spaced 18 inches on center. Based upon compliance with the above recommendations, an allowable soil bearing value for compacted soil 2,000 psf for 24-inch-deep footings and 2,500 psf for 36-inch-deep footings with a one-third increase when considering
- wind or seismic forces may be assumed. 10. Floor elevations located lower than the surrounding exterior grades are recommended to be protected from moisture intrusion. Please consult the
- building designer for details, such as waterproofing and French drains. 11. The Geotechnical Engineer shall be requested to observe the footing

excavations after the compaction tests are completed and prior to placement of the rebar reinforcing. While these recommendations are intended to provide satisfactory foundation performance for a new structure, it should be noted the concrete foundation will be in contact with an expansive soil. The new foundation will be connected to the existing foundation, which is different and inadequate to prevent damage from expansive soil. Consequently, you may expect cracks to appear as a result of the movement. Also, changes over time may alter the foundation's ability to remain level (i.e. broken water lines, droughts, non-symmetrical irrigation practices and altered drainage grades). In addition, associated features, such as porches. patios, driveways, sidewalks, and curbs, will be subject to differential movement. Porches and patios of concrete slab-on-grade construction are recommended to be attached to the

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perimeter foundation of the proposed structures such that an equivalent elevation is

Lab No: 115040steel and doweled cold joints. The joint between the old and the new structures may also experience differential movement. Where possible, construction joints are recommended for surface crack control. Over the life of the structure, maintenance or replacement of sections of

maintained at the joints. All concrete features shall be reinforced with continuous horizontal

the associated features and appurtenances damaged by the expansive soils may be required. RESISTANCE TO LATERAL LOADS

Lateral loads may be resisted by frictional resistance along the foundation base and passive earth pressures along the foundation sides. An allowable friction coefficient of 0.35 may be used. The passive pressures of 350 pcf of footing may be used. A triangular distribution should be used. The frictional resistance and the passive pressure may be

combined without reduction. The resistance may be increased by one-third for wind or seismic loading.

SWIMMING POOL The location selected for the proposed swimming pool was found to be on a previously placed fill, approximately 40-feet thick. The fill is underlain by a 10-foot layer of black Rincon clay. This is the original topsoil layer. The Rincon shale was encountered below the black clay at a depth of 50 feet. These soil conditions make the placement of a swimming pool at that location a potential risk for damage to the pool. The potential for unstable slopes and expansive soil to damage the pool is high and there is no economical mitigation. Supporting the pool on piles doesn't make sense because the piles would need to be at least 60 feet deep and that is too long to prevent lateral creep movement. All slopes in Santa Barbara are landslide prone when heavy rains occur. The height and soil type associated with the north slope is such that an incident of instability is possible. It is possible to construct a swimming pool at the proposed location provided it is understood the pool may be damaged by potential

- slope instability. The recommendations below provide a pool foundation that mitigates the effects of expansive soil only.
 - concrete grade beams supported by reinforced concrete cast-in-place piles. The reinforced concrete piles shall be a minimum of 18 inches in diameter and

The foundation system for the proposed replacement pool shall be reinforced

- shall extend a minimum distance of 15 feet below the floor of the pool. The vertical load capacity of the piles may be based on a skin friction value of
- 500 psf for the total length of the pile embedded below the floor of the pool. 4. The piles, grade beams, walls, and floor of the pool shall be designed by a Civil
- 5. A 4-inch-tall collapsible cardboard box (CCB) forming material, such as SureVoid[™] or equivalent, shall be placed below the grade beams and below concrete structural slabs to prevent the uplift swell pressures of the expansive soil from acting on the bottom of the structure.

This Laboratory shall be requested to inspect all excavations prior to steel and concrete placement.

support from the adjacent soil.

movement that must be accepted by the owner.

soil. Drainage can serve to reduce the rate of movement.

and grade beam foundation system.

the vertical differential is measured

CONSTRUCTION OBSERVATION

December 22, 2016

PLAN REVIEW

CLOSURE

performed by this Laboratory.

Addressee (3)

regarding this matter, please do not hesitate to call.

SETTLEMENT

35-A South La Patera Lane P.O. Box 96, Goleta, CA 93116 (805) 964-6901 FAX No.: (805) 964-6239 E-mail: pml@pml.sbcoxmail.com

April 6, 2023 Lab No: 140385-2 File No: 23-11153-2

Paula Kislak 545 Hodges Lane

Materials

Santa Barbara, CA 93108 SUBJECT: Grading and Foundation Plan Review

of Santa Barbara, Inc.

Proposed Accessory Dwelling Unit (ADU) 3627 Campanil Drive

Santa Barbara, California REFERENCE: This Laboratory's Preliminary Geotechnical Investigation Report

Lab No. 115040-2, Dated December 22, 2017 This Laboratory's Preliminary Geotechnical Investigation Update Report

Lab No. 137805-2, Dated May 3, 2022

Dear Ms. Kislak: In accordance with your request, I have reviewed the grading and foundation plan for the proposed ADU. Based upon my review, it is my opinion the grading and foundation plan is in substantial conformance

with the recommendations of our geotechnical report referenced above. This review is not comprehensive and does not include a review of the architect's and/or civil and

structural engineers' design and calculations. The observation of excavations during the construction phase represents an opportunity by our firm either to confirm soil conditions estimated by the exploratory borings, or to discover soil conditions that have not been addressed. When such undisclosed conditions are encountered, opinions and recommendations

addressing these conditions will be rendered at that time. The reference report is considered preliminary and no person should consider the recommendations or soil conditions described therein as conclusive. The recommendations and conclusions contained in the reference report are considered preliminary until all excavations have been observed during the construction phase, after which a final report will be issued at your request stating that the grading and foundation works

accomplished and installed are appropriate for the soil conditions encountered. If you have any questions concerning this report, please do not hesitate to call. Thank you for the

Respectfully submitted,

PACIFIC MATERIALS LABORATORY, INC.

opportunity of providing this service.

Geotechnical Engineer, G. E. 2291

The bottom of the pile excavation shall be cleaned of debris, using a pancake auger for the last pass, in order to provide end-bearing contact.

Concrete pool decks will move differentially with respect to the pool structure.

best and hides the differential movement. An example of a flexible deck is

recommended that a flexible surrounding deck be incorporated into the design

The walls of the swimming pool shall be designed to resist a lateral earth load of

10. The walls of the pool shall be designed as freestanding walls deriving no lateral

11. The owner or his agent shall request the Project Geotechnical Engineer to also

12. We request the foundation plan be submitted to our office for a general review to

Where footings are placed at varying elevations, the effect of adjacent loads may be

The expansive soil encountered at the site is identified as the most challenging aspect

of the foundation design. The Rincon Formation is prone to instabilities, such as surficial slope

failures, mudflows, creep, expansion, and shrinkage. Perfect performance of appurtenant

improvements, as well as the old and new footings of the house, is an unacceptable level of

expectation on the part of the property owner or future owners. Associated features, such as

patios, walkways, trellis columns, pool equipment enclosure, and driveways will be subject to

movement due to the expansion and shrinkage of the clay surface soils. The foundation

design is intended to support the structure in a safe manner and with an acceptable risk of

An economical way of reducing the anticipated movement of the appurtenant

by carefully choosing the type of building materials used to construct these associated

improvements, such as walkways and patios, is by moisture control of the supporting soils and

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features. Moisture control can be approached by installing surface storm drain collection

systems, controlling surface water, and the proper placement of planting areas around the

rock, patios, or paths. Controlling surface water and directing it away from slopes is critical.

to winter and, therefore, the soils will shrink and swell, moving any item supported over the

decks are more flexible than concrete patios and do not reflect as much damage from soil

is to maintain exterior flatwork disconnected from the exterior foundation of the house, since

disconnected, the exterior flatwork is allowed to float. The problem, however, is that the

foundation and the edge of the concrete slab or creating a trip edge at porch entries.

the exterior flatwork may experience more movement than the foundation. By leaving the two

flatwork may tend to float away from the house, creating an uneven gap distance between the

to prevent the differential movement and to prevent the gap and trip edge from occurring. This

insive soil. Prevention of the downward movement is only accomplished with a drilled pile

It is the intent of the recommendations contained in this report to achieve angular

anticipated in the undisturbed, native soil and approximately 1% to 1.5% of the fill height is the

anticipated total settlement at areas where compacted fill soil is placed in accordance with the

soil has already been discussed in this report under the heading APPURTENANCES. The soil

grading recommendations provided in this soil engineering report. Movement from expansive

Angular distortion is the ratio of the vertical differential settlement divided by the horizontal distance over which

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excavations prior to placement of compacted soil, gravel backfill, or rebar and concrete.

review to verify substantial compliance to the recommendations contained in this report.

The owner or his agent shall request the Project Geotechnical Engineer to observe all

We request the grading and foundation plans be submitted to our office for a general

The recommendations contained herein are for the sole use of our client and are based

upon this Laboratory's understanding of the project which has been described herein. If the

project scope, location, or conceptual design is subsequently altered, this Laboratory shall be

requested to modify, as necessary, the recommendations contained herein as is appropriate

for the new development concept. If the recommendations of this report are not implemented

The recommendations contained herein are based upon the assumption that Pacific

Materials Laboratory shall be requested to perform the testing and observation services which

will be required during the grading and foundation operations in order to verify that the actual

recommendations contained herein. If this service is performed by others, only the technical

correctness of the actual analytical soil tests described here is attested to by this Laboratory.

Clause Construction, Attn: Dan Clause, Email: dan@clauseconstruction.com

Thomas V. Smith Architect, Email: thomas.v.smith@netscape.com

JT Engineering & Associates, Inc., Attn: Jason Tani, Email: jtengr@gmail.com

Pacific Materials Laboratory of Santa Barbara, Inc.

Thank you for the opportunity of providing this service. If you have any questions

Respectfully submitted,

Ronald J. Pike

PACIFIC MATERIALS LABORATORY, INC.

Geotechnical Engineer, G. E. 2291

soil conditions encountered and the construction procedures are consistent with the

within one year, we recommend an update and review of the contents of this report be

bearing values and estimated settlements contained in this report are preliminary and may

need to be modified after the foundation and grading plans are substantially complete.

distortions¹ of approximately 1/480. A total settlement of approximately 1 inch or less is

method, however, holds one end of the flatwork fixed while the other is free to move, thus

cracking the concrete. For either method, the owner can expect movement and, therefore,

cracks. Individual stone pavers with grass between the joints provide an alternative style of

"flatwork" with less cracks and the differential movement is not as noticeable. Where the

improvements discussed above are located adjacent to slopes, a lateral and downward

creeping occurs which undermines the feature constructed over and supported by the

The other alternative is to dowel the flatwork into the exterior foundation with steel rebar

foundation system. Water entering the subsurface soils can be reduced by the placement of a

false bottom below planters or an impervious membrane, such as visqueen beneath decorative

However, even with correct drainage, the moisture content of the soil will change from summer

With respect to construction materials for the appurtenant improvements, redwood

movement. If concrete flatwork is required, there are at least two alternative approaches. One

calculated using the widely published Formulas for Stresses in Semi-infinite Elastic

Foundations or the Boussinesq figures and equations for both vertical and horizontal

verify substantial compliance to the recommendations contained in this report.

observe all excavations prior to placement of compacted soil, or rebar and

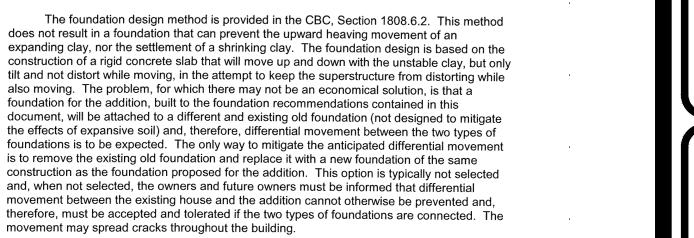
individual stone pavers with grass growing between the joints. It is

This may be due to the difference in support elevations. A flexible deck performs

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or Structural Engineer

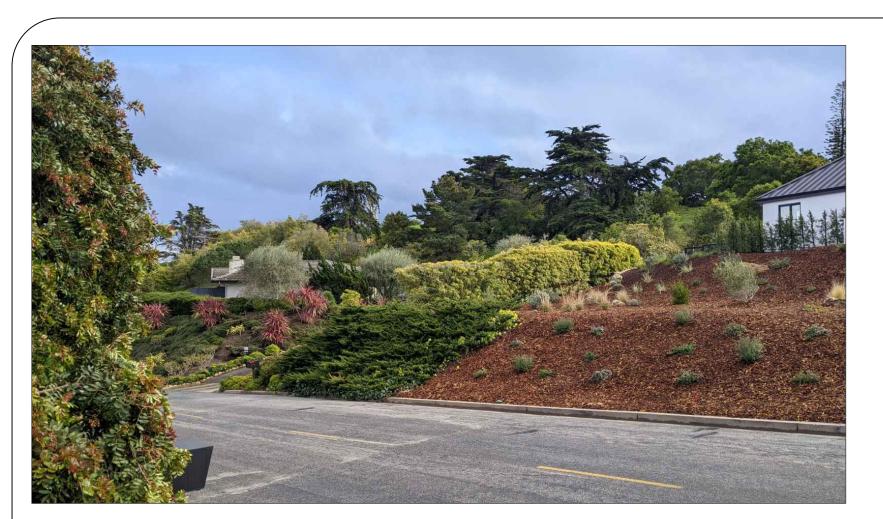
Pacific Materials Laboratory of Santa Barbara, Inc.



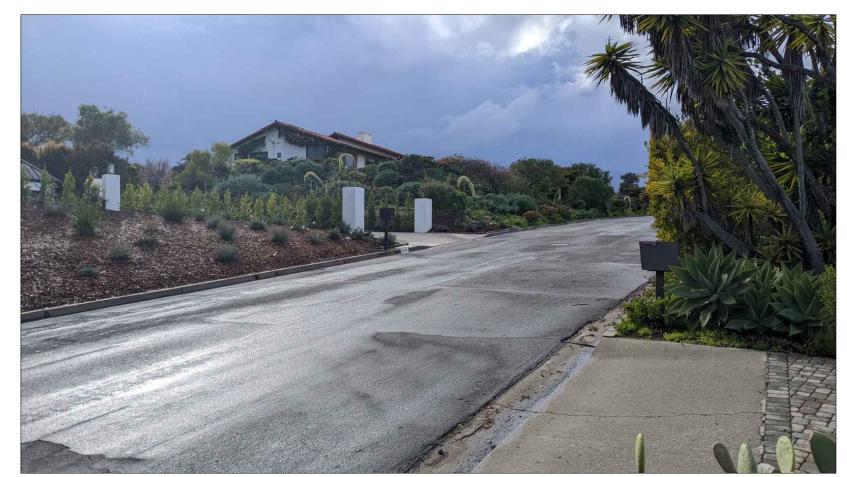
Lab No: 115040-2

File No: 16-11153-2

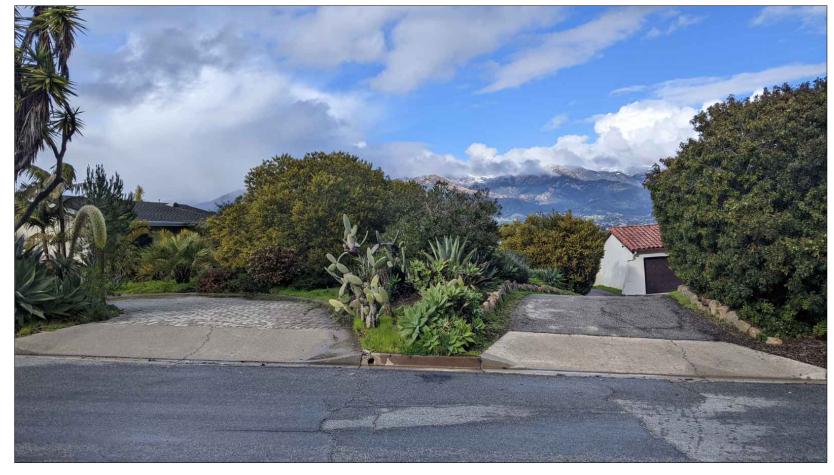
therefore, must be accepted and tolerated if the two types of foundations are connected. The movement may spread cracks throughout the building. The complete removal of the existing foundation and the construction of a new foundation using piles, grade beams, and a structural slab would mitigate the potential foundation problems derived from expansive soil. No pile recommendations are







1) STREET VIEW TO SOUTHDIRECTLY ACROSS STREET FROM PROPERTY







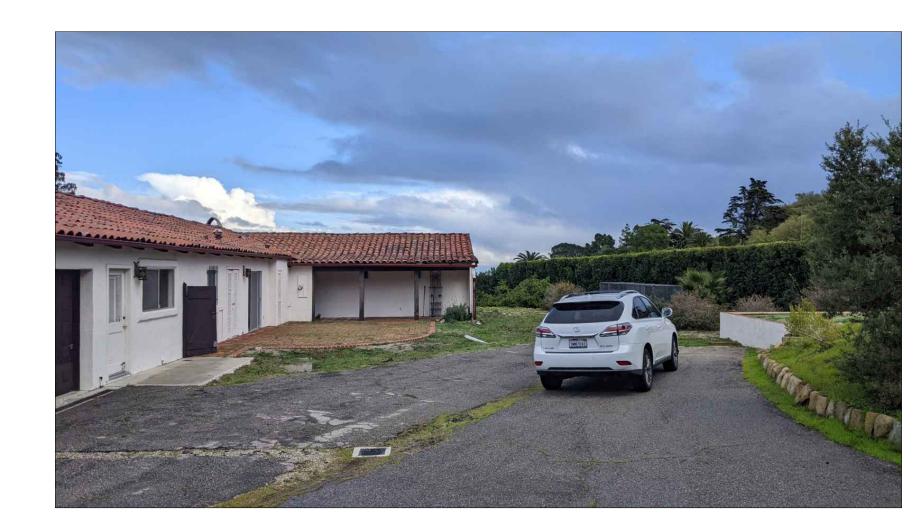
2) STREET VIEW TO NORTH TOWARDS PROPERTY





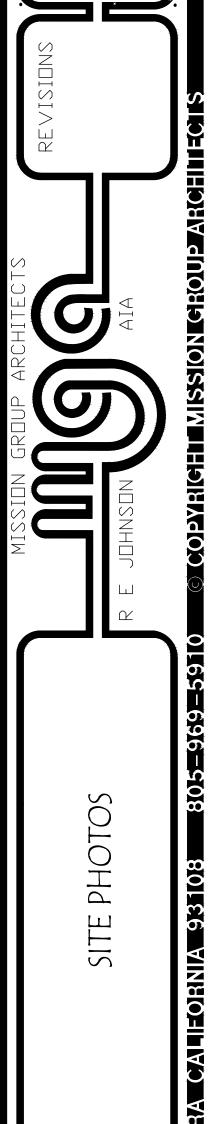
4) STREET VIEW TO NORTH TOWARDS PROPERTY







5) VIEW OF SOUTH SIDE OF EXISTING RESIDENCE LOOKING EAST



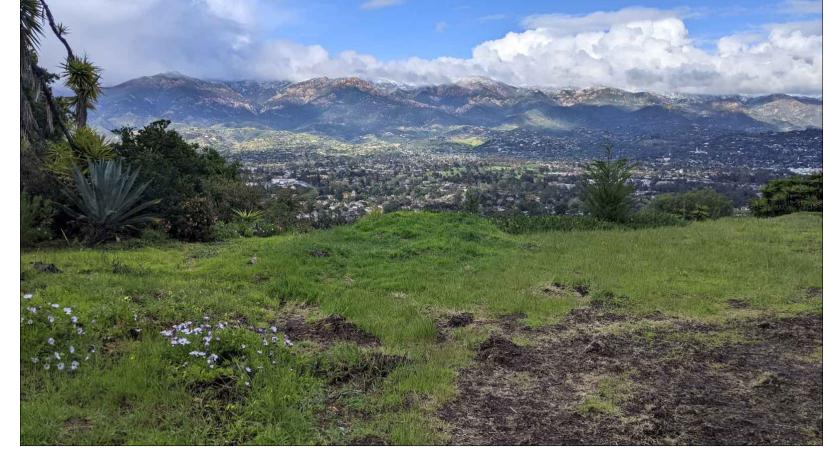


6) NORTH SIDE OF EXISTING RESIDENCE LOOKING EAST





8) EAST SIDE OF EXISTING RESIDENCE LOOKING SOUTH



10) WEST SIDE OF EXISTING PROPERTY LOOKING NORTH (ADU SITE)



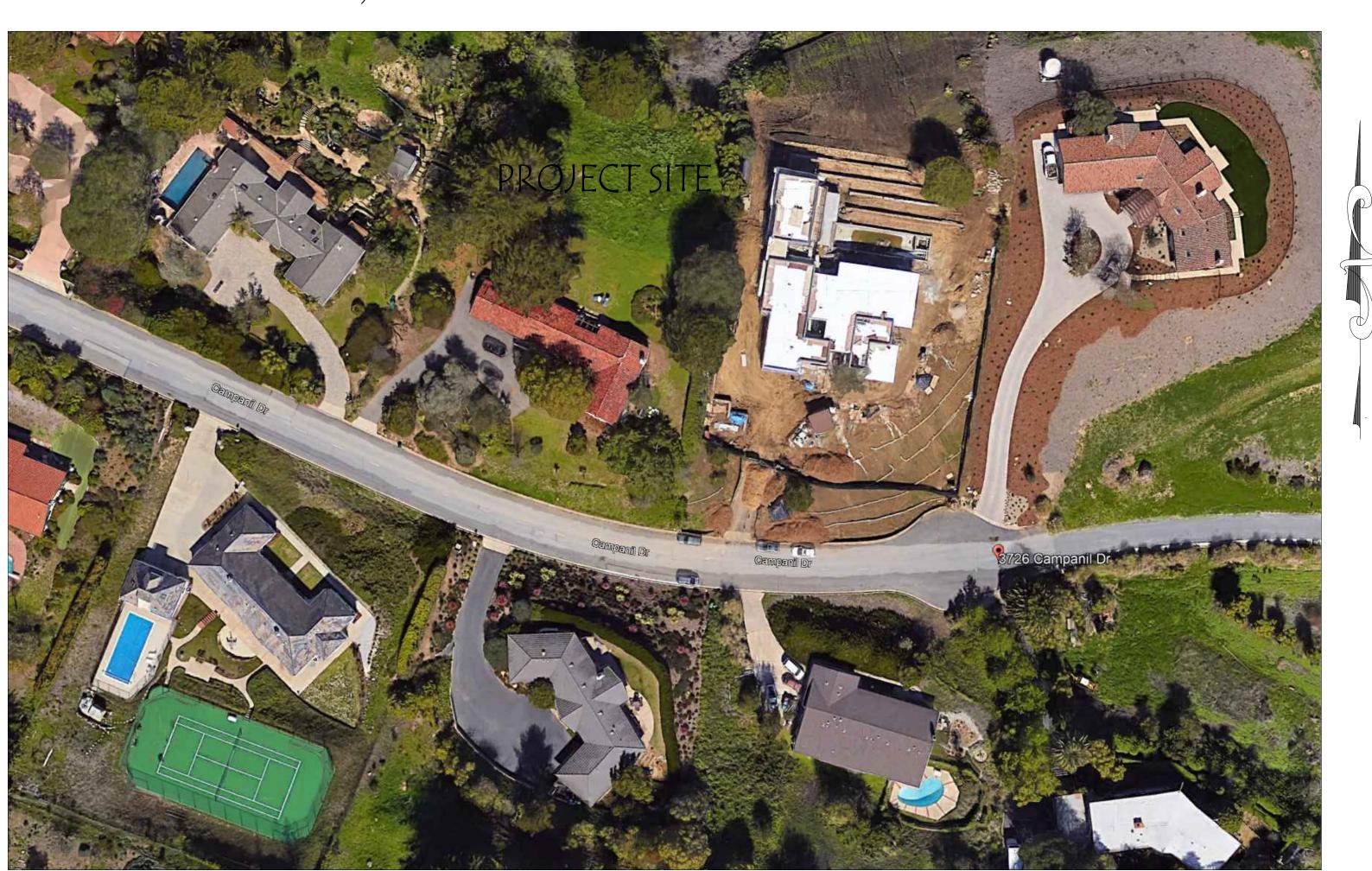
11) NORTH SIDE OF EXISTING RESIDENCE LOOKING WEST (ADU SITE)



7) WEST SIDE OF EXISTING RESIDENCE LOOKING SOUTH



9) EAST SIDE OF EXISTING RESIDENCE LOOKING NORTH



12) AERIAL VIEW OF SITE AND VICINITY

SITEMORK SPECIFICATIONS

2010 GENERAL REQUIREMENTS

- 2011 SCOPE: The proposed work consists of the following: Earthwork, storm drainage, utilities, paving, retaining walls, and related
- 2012 DETAIL: The drawings are intended to show or reference all details necessary to construct the proposed work. The contractor shall review these drawings and determine prior to commencement of construction if additional details or clarification of information is necessary. The engineer shall be given sufficient time to provide any additional information prior to construction.
- 2013 EXISTING CONDITIONS: The contractor shall verify all existing conditions and measurements shown on the drawings and report any differences to the engineer prior to construction.
- 2014 PROTECTION OF FACILITIES: The contractor shall be responsible for all on and off site structures, streets, utilities and landscaping.
- 2015 SURVEYING: The contractor shall provide for all surveying required to locate property lines, set flow lines of pipes and qutters to obtain new final grades and any other surveying required to construct the improvements.
- 2016 PERMITS: The contractor shall pay for all permits, licenses, and fee required by the governing agencies except, the owner shall pay for the general building and arading permits. The contractor may be required to sign the general building and grading permits. 2017 INSPECTION: The contractorshall be responsible for requesting
- building codes. Allow 24 hours advance notice. 2018 SPECIAL INSPECTION: Special testing and inspection by a certified material testina laboratory and/or licensed special inspector may be required as noted below. The contractor shall be responsible for requesting, coordinating and obtaining all inspections and testing as may be required. The owner shall pay for the initial testing and inspection. Any additional testing and inspection required by the contractors performance or scheduling shall be

coordinating and obtaining all inspections requires by the local

- I. Observe the over excavation to determine that the depth of excavation and bottom of the subarade are suitable.
- 2. Observe the exposed subgrade and keyway in areas to receive fill and in areas where excavation has resulted in the desired finished subgrade, observe proof rolling and
- delineate areas requiring additional excavation 3. Perform visual observation to evaluate the suitability of on-site and imported soils for fill placement; collect and submit soil samples for laboratory testing.
- 4. Perform field density and compaction testing to determine the percentage of compaction achieved during placement of 5. Observe and probe foundation bearing materials to confirm
- that suitable bearing materials are present at the design 6. Observe and test the backfill of retaining walls. 1. Observe and test the backfill of utility trenches.

8. Observe and test the construction of the subgrade and base

- for exterior paving 9. Observe the construction and protection of slopes. 2019 RECORD KEEPING: The contractor shall keep a set of the approved drawings, permits and contract documents in a protected on site location at all times and shall keep daily field
- reports of all special inspection and testing. 2020 AS BUILT DRAWING: the contractor shall keep accurate as-built drawings of all work as required such that final drawings by the
- engineer may be provided to the owner 2021 AUTHORIZED CHANGES: The engineer shall review and approve any changes to the drawings or specifications prior to
- 2022 CODE REQUIREMENTS: All work shall be performed in accordance with the latest edition of governing code's and local ordinances unless specifically noted otherwise in the drawing or specification.
- 2023 PLAN COORDINATION: The contractor shall refer to the project architectural drawings and specifications including those for site layout, building, mechanical, electrical and landscaping improvements and for interfacing with all improvements called for
- 2024 UNDERGROUND UTILITIES: The contractor shall make a thorough search for all underground structures and utilities and shall request all utilitu agéncies to locate all underground structures and utilities prior to excavation. Call the underground service alert (2) full days in advance of commencing construction at 811.
- equipment from the premise

2050 DEMOLITION

- 2051 REMOVAL: Specific existing improvements shall be removed as required to construct new improvements. Where required the improvements to be removed shall include disconnection and capping of utilities lines serving the improvements and any foundation structures supporting such improvements.
- 2052 UTILITIES: Location of existing utilities may or may not be showed in their entirety or exact location on the drawing. Contractor shall determine actual extent and location of utilities. Contractor shall coordinate disconnection of utilities with the utility company and owner. Where shown on the drawings, utilities may be abandoned in place, unless they conflict with new improvements. Contractor shall provide for temporary disconnect where reconnection is required.
- 2053 SALVAGING OF MATERIALS: Where saving of materials to be removed is required, the contractor shall deliver materials to an on site storage location designated by the owner.
- 2054 PAVEMENT: Where required by the drawings, certain sections of pavement shall be replaced or removed. The contractor and engineer shall meet at the site and designate the specific areas to be replaced or removed. All cuts in pavement shall be made by sawcutting to create straight and neat joints.

2200 SHORING AND BRACING

- 2201 SHORING AND BRACING: The contractor shall be responsible for all excavation including shoring and protecting of adjacent property, structures, streets, utilities. 2202 SHORING: The contractor is responsible for the design and
- construction of any shoring required to meet OSHA requirements. 2250 EARTHWORK
- 2251 CODE REQUIREMENTS: All grading shall conform with Appendix J of the California Building Code, the local grading ordinances and the soils/geotechnical report.
- 2253 SANTA BARBARA CITY GRADING NOTES: All grading shall conform with Santa Barbara Municipal Code Chapter 14 and standards and requirements pertaining thereto, these construction drawings and the recommendations of the soils engineer and engineering geologist.
 - 2. Contractor to notify the grading inspector and soils laboratory at least 48 hours before start of grading work or any pre-construction meeting. Contractor shall employ all labor, equipment and methods required to prevent his operations from producing dust in amounts damaging to adjacent property, cultivated vegetation
 - and domestic animals or causing a nuisance to persons occupying buildings in the vicinity of the job site. Contractor shall be responsible for damage caused by dust from his grading operation. 4. Before beginning work requiring exporting or importing of materials, the contractor shall obtain approval from Public
 - Works Road Division for haul routes used and methods provided to minimize the deposit of soils on country roads. Grading/road inspectors shall monitor this requirement with a contractor. 5. The Geotechnical engineer shall provide observation and testing during grading operations in the field and shall submit a final report stating that all earth work was properly completed and is
 - in substantial conformance with the requirements of the grading 6. Areas to be graded shall be cleared of all vegetation including roots and all unsuitable material for a structural fill, then scarified to a depth of 6" prior to placing of any fill. Call
 - grading inspector for initial inspection. 7. Á thoróugh search shall be ma'de for all abandoned man-made facilities such as septic tank systems, fuel or water storage tanks, and pipelines or conduits. Any such facilities encountered shall be removed and the depression properly filled and
 - compacted under observation of the geotechnical engineer. 8. Areas with existing slopes which are to receive fill material shall be keyed and benched. The design and installation of the keyway shall be per the geotechnical engineer's recommendation of per
 - County Standard Detail no. G-13. 9. Fill material shall be spread in lifts not exceeding 6" in compacted thickness, moistened or dried as necessary to near optimum maisture content and compacted by an approved method. Fill material shall be compacted to a minimum of 90% maximum density as determined by 1957 ASTM D-1557-91 modified proctor (AASHO) test or similar approved methods. Some fill areas may require compaction to a greater density if called for in the construction documents. Soil tests shall be conducted at not less than one test for each 18" of fill and/or for each 500 cubic yards of fill placed.

- 10. Cut slopes shall not exceed a grade of 2 horizontal to 1 vertical. Fill and combination fill and cut slopes shall not exceed 2 horizontal to 1 vertical. Slopes over three feet in vertical height shall be planted with approved perennial or treated with equally approved erosion control measures prior to final inspection.
- II. Surface drainage shall be provided at a minimum of 5% for 10 feet away from the foundation line or any structure. 12. All trees that are to remain on site shall be temporarily fenced and
- protected around the dripline during grading. 13. An erosion and sediment control shall be required as part of the grading plan and permit requirements.
- 14. "Best Management Practices for Construction Activities: Eroded sediments and other pollutants must be retained onsite and may not be transported from the site via sheet flow, swale, area drains, neutral drainage courses, or wind. Stockpiles of earth and other construction related materials must be protected from being transported from the site by the forces of wind or water. Fuels, oils, solvents, and other toxic materials must be stored in accordance with their listing and are not to contaminate the soil and surface waters. All approved storage containers are to be protected from the weather. Spills may not be washed into the drainage system. Excess or waste concrete may not be washed into public way or any other drainage system. Provisions must be made to retain concrete wastes on site until they can be disposed as a solid waste. Trash and construction related solid waste must be deposited into a covered waste receptacle to prevent contamination of rainwater and dispersal by wind. Sediments and other material may not be tracked from to the site by vehicle traffic. The construction entrance roadwaus must be stabilized so as to inhibit sediments from being deposited into the public way. Accidental deposition must be swept up immediately and may not be washed down by rain or other means. Any slopes with disturbed soils or denuded of régetation must be stabilizéd s'o as to minimize erosion by wind and
- 15. If grading occurs during Nov. I through Apr. 15, no grading shall occur unless approved erosion and sediment control measures are in place. Discharges of sediment from the project site may result in a "Stop"
- 16. All earthwork on hillsides, slopina or mountainous terrain shall be stabilized to protect and prevent loss of soils, as necessary, year-round.
- 2254 REPORTS: The following reports have been prepared for this project and the recommendations included therein shall be incorporated into this specification: Preliminary Geotechnical Investigation by Pacific Materials Lab. dated Dec. 22, 2016. Addendum Report dated Dec. 13, 2017. Update letter May 3, 2022
- 2255 SPECIFIC REQUIREMENTS:

GRADING

- All of the grading is anticipated to be a cut excavation with no fill soil being placed, or a very minor amount in the northeast corner of the motor court. In the areas to be prepared to receive compacted fill soil, the loose topsoil and compressible surface soils shall be removed and observed by a representative of our firm. Upon approval of excavation, the exposed ground surface shall be scarified an additional 6 to 8 inches, moistened or dried to near the optimum moisture content, and compacted to 90% of the relative compaction. The depth of the fill shall be limited to a vertical height of 18 inches.
- The removed surface soils and/or imported approved fill may then be placed in loose lifts of approximately 6 inches, thoroughly mixed, moistened or dried to near optimum moisture content, and compacted to a minimum of 90% relative compaction.
- 3. Import soils, if required for compacted fill, shall be granular, non-expansive soils which are equal to, or superior in quality to, the on-site soils as determined by this Laboratory prior to importation of the fill material to the site. This is not referring to retaining wall backfill. See the RETAINING WALLS section of this report for retaining wall backfill requirements.
- The compaction standard shall be the latest adoption of the ASTM D-1557 method of compaction.
- Positive surface drainage shall direct water away from all slopes and away from the oundation system of the proposed structure.
- 2256 EARTHWORK ESTIMATE: The following earthwork estimate is for permit purposes only.

42 Cu. Yds.

Note: Exact shrinkage, consolidation and subsidence factors and losses due to clearing operations are not included. Estimated earthwork quantities are based on the difference between existing around surface and proposed finished arades as shown on the plan. or subgrades, and should vary according to these factors. Contractor shall confirm existing topography, shall review the site and the soils reports, and shall preform an independent quantify takeoff and bid accordingly.

2257 TESTING AND INSPECTION: See soils/geotechnical report for scope of testing and inspection required during earthwork

2300 BUILDING EXCAVATION

2301 BUILDING EXCAVATION AND FILL: Refer to structural specifications for specific building foundation excavation and/or fill requirements and testing, inspection and construction requirements.

2350 UTILITY TRENCHING

- 2351 UTILITY TRENCHING AND BACKFILL: Vertical trench excavations less than 5 feet deep should be capable of standing with minimum shoring or bracing for short construction periods. Trenches 5 feet or more deep should be provided with more substantial shoring or oracing. The attention of contractors should be drawn to the State of California Construction Safety Orders for "Excavations, trenches, Earthwork".
- 2352 BEDDING: For the purposes of this section, bedding is defined as material placed in a trench up to I foot above a utility pipe and backfill is all material placed in the trench above the bedding. Unless concrete bedding is required around utility pipes, free draining sand shall be used as bedding. Sand proposed for use in bedding should be tested in laboratory to verify its suitability and to measure its compaction characteristics. Sand bedding should be compacted by mechanical means to achieve atleast 90 percent relative density based on ASTM tests D-4283 and D-4284.
- 2353 BACKFILL: Approved, on site, inorganic soil, or imported materials may be used as utility trench backfill, a sample of it should be tested and approved by the soils engineer before any is delivered to the site.
- 2354 COMPACTION: Proper compaction of trench backfill will be necessary under and adjacent to structural fill, building foundations, concrete slabs and vehicle pavements. In these areas, backfill should be conditioned with water to produce a soil-water content of about 3 to 5 percent above optimum value and placed in horizontal layers not exceeding 6 inches in thickness (before compaction). Each layer should be compacted to at least 90% relative compaction based on ASTM Test D-1557. The upper 12 inches of trench backfill under vehicle pavements should be compacted to at least 95% relative compaction. Where any trench crosses the perimeter line of any building, the trench should be completely plugged and sealed with compacted clay soil for a horizontal distance of 2 feet on either side of the foundation.
- 2355 TRENCHES: To reduce the potential for piping loss of utility line bedding and backfill, slurry or lean concrete plugs should be installed in utility trenches which slope steeper than 10%. The plugs should be a minimum of 2 feet thick and should be placed at 50 foot maximum intervals. The plugs should extend 2 feet below and 2 feet into both sidewalls of the trench. The top of the plug should extend to within 6 inches of the top of the trench. At the bottom of the trench, a pocket of free draining gravel (approximately I cubic foot per foot of trench width) should be placed immediatelu up-gradiant of each plug. The gravel pocket should be wrapped in a permeable synthetic filter fabric conformina to Caltrans Section 88-1.03 for underdrains, and should be drained by a PVC pipe sloped to daylight.

2400 POTABLE WATER SYSTEM

- 2401 CODE: The potable water system shall be installed in accordance with the latest edition of the California Plumbing Code.
- 2402 LIMITS: This section addresses underground domestic water from the meter to a point two feet outside of any new buildings or to the point of connection of existing buildings.
- 2403 PIPE AND FITTINGS: Pipe and fittings shall be type L copper with soldered joints or schedule 40 pvc with solvent weld or threaded joint and shall conform to the requirements of the CPC. All above grade pipe shall be metal.
- 2404 VALVES: Valves shall be PVC construction rated for 150 psi as manufactured by Ryan Herco Products Corporation or equivalent. PVC valves gréater than 2" diameter shall not be used unless approved by the engineer.
- 2405 INSTALLATION: Pipe assembly and installation shall be in conformance with AWWA and CPC standards and with the manufacturer's avidelines and recommendations. Pipe, fittings and appurtenances shall fit in place without strain and shall be supported and anchored as necessary
- Above ground piping systems shall include union or flange connections placed as necessary to allow removal of system components for servicing or repair; piping shall be supported or anchored at intervals reccomended by the manufacturer for the intended application, not to exceed intervals required by the current edition of the California Plumbing Code. Pipe and fittings shall be assembled usind non-toxic lubricants and compounds. Unless otherwise called for on the plans or otherwise specefied herein, adaption from PVC to iron or steel pipe and fittings

installed below ground shall be wrapped with coal tar tape

otherwise specified. The pressure regulator shall be IAMPMO

applied over its companion primer. 2407 TESTING: Hydrostatic pressure test in accordance with the CPC. 2408 PRESSURE REGULATOR: Shall be installed adjacent to the water valve near the service entrance to each living unit unless

2450 FIRE WATER SYSTEMS

2451 LIMITS: This section covers the private fire line and sprinkler supply system from the double check valve assembly to a point two feet outside of any new building. The double check valve assembly will be constructed under a seperate permit. The building sprinkler system will be installed under seperate permits.

2550 STORM SEWAGE SYSTEMS

- 2551 STORM DRAINAGE: Prior to commencement of storm drainage installation, the contractor and engineer shall meet to review the proposed storm drainage plan.
- 2552 SCHEDULING: Contractor shall schedule gravity pipeline work ahead of other underground conduit construction
- 2553 LAYOUT: Gravity storm drain work shall begin at the lowest point and proceed upstream.
- 2554 CONCRETE: Concrete for all drainage facilities shall be 2500 psi @ 28 days.
- 2556 NON METALLIC PIPE: Storm drain piping 6" diameter and less unless specifically noted otherwise shall be non perforated, heavy duty smooth wall poly polyethylene pipe conforming to ASTM F810 as manufactured by Advanced Drainage Systems, Hancor or equal Corrugated pipe may be used from the downspout to the collector drain line. Fittings shall be heavy duty polyethylene and selected to fit pipe and drain boxes. Unléss noted othérwise, all pipe shall be laid at a minimum 1/4" per foot slope.
- For storm drain pipe greater than 6" in diameter use heavy duty, non-perforated corrugated HDPE pipe with smooth lining as manufactured by Advanced Drainage System N-12 or equal.
- 2878 FOOTINGS: Concrete shall be 2500 psi @ 28 days conforming to PVC (Poluvinul Chloride) Pipe shall conform to the requirements of ASTM CI50. ANSI/ASTM D3034-78, Type PSM for gravity flow sewers and shall be SDR 35. Gaskets shall be flexible elastomeric seals meeting the requirements of ASTM D3212-77.
- 2558 TESTING: Underground storm drainage system, shall be water pressure tested prior to burying pipes.
- materials and construction methods shall be in accordance with these drawings and the requirements of the local building
- 2560 RAIN GUTTERS AND DOWNSPOUTS: The finished structure shall be fitted with rain autters and downspouts that collect and discharge all roof rain water run-off to underground drainage sustem or hard paved surface as indicated. Rain autters and downspouts shall be per the architectural drawings. Gutters tributary to chamber, rain barrels, or cisterns shall be screened with a leaf quard or ½" to ¼" min. corrosion resistant metallic hardware fabric.
- 2561 TRENCHING: For bedding and backfill material see Section 2350. 2564 GRATES: Grates located in yard areas shall be cast iron. Grates located in vehicle travelwaus shall be rated for heavy traffic loading, shall be bicycle proof, and shall be cast iron as

manufactured by Alhambra Foundry or the Neenan Foundry Co. or

- 2565 GRATES: Grates for non metallic drain boxes shall be PVC or ABS, or PE or equal and shall be matched to fit the drain box. Grates in hard surfaced patio areas shall be brass or bronze grate and frame as manufactured by the Zurn company or equal. Contractor to submit shop drawings of all grates to engineer for
- 2566 DRAINAGE DITCHES: Where rock in mortar drainage ditches adjacent to road pavement occurs, use 4-8 diameter sandstone boulders of dressed stone set in 4 grout (2000 psi @ 28 days) with 6x6, 10/10 w.w. mesh.
- 2567 FRENCH DRAIN: Pipe shall be PVC or ABS smooth wall non corrugated with holes (not slots) at 60 and 120. Holes shall be placed at 4 O'clock and 8 O'clock. The french drain should be placed on the outside of the continuous
- footing in the area to be protected. It should consist of a minimum 4"ø rigid PVC perforated pipe at the bottom of a 12" wide trench filled with the free draining gravel. The gravel shall be wrapped with a filter fabric. 2568 FILTER FABRIC: Shall be a non woven geotextile fabric with high
- flow capacity and small pore size such as Mirafi 140 N. or equal The fabric shall be installed in accordance with the manufacturers
- 2570 GRAVEL FILTER: Shall be 3/4"-1" diameter round rock.

<u>2600 SANITARY SEWAGE SYSTEMS</u>

meeting ASTM D-3034.

approval prior to installation.

- 2601 CODE REQUIREMENTS: The sanitary sewer system shall be installed in accordance with the latest edition of the California Plumbing
- 2602 LIMITS: This section covers the building sewer line from the face of the building to the public sewer lateral.
- 2603 PIPE AND FITTING: Sanitary sewer pipe 4 inches in diameter and less shall be ABS rated sewer pipe per ANSI D 2751-80. Sanitary sewer pipe 6 inches in diameter and greater shall be PW pipé Twinseal PVC or John Mansville Ring-Tite PVC sewer pipe
- 2604 INSTALLATION: Unless specific noted otherwise, pipe shall be laid at a minimum 1/4" per foot slope. Bedding and backfill shall be in accordance with section 2350.
- 2605 TESTING: Testing of underground pipe shall be in accordance with
- 2607 CLEAN OUTS: Cleanouts shall be installed in accordance with CPC

2700 ELECTRICAL, CABLE, TELEPHONE SYSTEM

- 2701 EXISTING SYSTEM: The exact extent and location of the existing underground electrical, cable or telephone system has not been on the drawings but is known to exist.
- 2702 DRAWINGS BY OTHER: A seperate underground electrical, cable and telephone plan has been prepared by others.

shall be installed before electrical, cable and telephone conduits

- 2703 COORDINATION: The contractor shall coordinate the electrical, cable and telephone conduit and wire installation with the other underground pipes shown on the plan. Gravity flow pipeline systems
- 2704 UTILITY COMPANY APPROVAL: All electrical, cable TV, and telephone conduit shall be approved by the respective utility
- 2706 PROPOSED ELECTRICAL DISTRIBUTION: Information shown on these drawings is approximate only and must be supplemented prior to construction with accurate electrical drawings provided by an electrical consultant or licensed electrical contractor.

2800 PORTLAND CEMENT CONCRETE PAVING

- 2801 CODE REQUIREMENTS: All concrete construction shall be
- installed in accordance with the ACI standards of practice. 2802 EXPANSION JOINTS: Expansion joints shall be installed wherever concrete is restricted from moving such as where it abuts other concrete surfaces, curbs. Existing structures, etc. These joints
- 2803 CONTROL JOINTS: Control joints shall be placed at 15-20 ft. intervals and shall be 3/4" to 1" deep. Consult with engineer for

shall be premolded elastic resilient material 3/4" thick.

- 2804 CURING: The contractor shall determine curing methods to
- 2805 LAYOUT: Plan elevations shown on walkways are for drainage and rough grading design only. It shall be contractor's responsibility to perform detailed layout for and to construct walkways, ramps, and steps in conformance with all building code requirements including those for dimensioning and surface

provide complete and carefull curing of all concrete work.

- 2807 PAVEMENT DESIGN: Concrete flatwork shall have a minimum structural section as follows: LOCATION SUBGRADE BASE STRENGTH REINFORCMENT Sidewalk/ 9" recomp 4" CLII 5"/2500 psi #4's @ 18" OCEW
- 2808 RECOMPACTION: Earth subgrade and base shall be recompacted to 95% relative compaction per ASTM D-1557.
- 2809 FINISHING: Concrete flatwork shall have the following finishes unless specified otherwise by the Architect: FINISH COLORED CONCRETE Steel Trowel White pigment curing seal Light Broom or colored stamped concrete per drawings Steel Trowel Colored, stamped concrete Patios Liaht Broom Stairs Steel Trowel Colored, stamped concrete Light Broom

Trash Enclosure Light Broom White pigment curing seal 2875 RETAINING WALL

- 2876 EXCAVATION: Footings shall be placed in firm natural ground or certified recompaction earth.
- 2877 EXCAVATION INSPECTION: Soils engineer shall inspect all footing excavations before installing reinforcing steel or forms.
- 2879 REBAR: Shall be ASTM A 615, grade 60 for #5 bars and larger, arade 40 for #4 bars and smaller
- 2880 REBAR SPLICE: Minimum rebar splice shall be 40 bar diameters. Stagger splices in adjacent bars by the length of the splice.
- 2881 MORTAR: Mortar shall be type M or S.
- 2882 MASONRY: Masonry shall be Fm = 1500 psi. 2883 GROUT: Grout all cells. Grout shall be 2000 psi @ 28 days.
- 2884 EXPANSION JOINTS: Walls which are not adjacent to the building shall have expansion joints at a maximum spacing
- 2885 WATERPROOFING: Walls shall be waterproofed per the architects requirements. Assume miradrain with gravel backfill over miradry or equal.
- 2886 BACKFILLING: Backfill shall be free-draining granular soil. Compact to 90% relative density. Allow wall to gain 21 days strength before backfilling. If backfill supports driveway compact to 95% relative density.
- 2887 DRAINAGE: Provide continuous 4" perforated ABS drain line behind all walls. Drain shall be sloped 1% towards outlet. Contractor shall provide filter blanket in gravel bed to prevent plugging of perforations.

full vertical hgt. of wall. Longitudinal steel to terminate

- 2888 BLOCK: Concrete blocks shall conform to ASTM C90. 2889 EXPANSION JOINTS: Provide vertical expansion joints at max. 30' intervals by placing rubber-strip or celotex for
- on each side of joint. 2890 CODE: All work shall be accordance with 2019 CBC.
- 2884 EXPANSION JOINTS: Walls which are not adjacent to the building shall have expansion joints at a maximum spacing of 30 feet on center.
- backfill over miradry or equal. 2886 BACKFILLING: Backfill shall be free-draining granular soil. Compact to 90% relative density. Allow wall to gain

2885 WATERPROOFING: Walls shall be waterproofed per the

architects requirements. Assume miradrain with gravel

21 days strength before backfilling. If backfill supports driveway compact to 95% relative density. 2886 BACKFILLING: Backfill shall be free-draining granular soil. Compact to 90% relative density. Allow wall to gain

21 days strength before backfilling. If backfill supports

driveway compact to 95% relative density. 2887 DRAINAGE: Provide continuous 4" perforated ABS drain line behind all walls. Drain shall be sloped 1% towards outlet. Contractor shall provide filter blanket in gravel bed to prevent plugging of perforations.

intervals by placing rubber-strip or celotex for full vertical hgt. of wall.

- 2888 BLOCK: Concrete blocks shall conform to ASTM C90. 2889 EXPANSION JOINTS: Provide vertical expansion joints at max. 30'
- Longitudinal steel to terminate on each side of joint. 2890 CODE: All work shall be accordance with 2019 CBC.

2900 FENCING

- 2901 REMOVAL: Where fence removal is required per the drawings, fence material shall be saved to the practical extent and neatly stockpiled in a location approved by the owner. Damage fencing and/or concrete bases shall be properly disposed off-site.
- 2902 RELOCATION: When fences are required to be relocated per the drawings, new fence post shall set in new concrete footings per the fence manufacturers standard requirements or at the spacing currently existing. Chain link fencing shall be reinstalled per the manufacturers
- standard requirements. 2903 MATERIALS: Fencing and gates within fencing shall be metal, painted black and design shall match existing house quardrail. Gates shall include hardware to provide for self closing and self latching.

2975 EROSION CONTROL (CITY)

See Architectural Drawings and Specifications. 2920 LANDSCAPING

- 2921 DRAWINGS: Landscape drawings showing planting, irrigation, and miscellaneous features have been prepared by others.
- 2922 COORDINATION OF IMPROVEMENTS: The contractor shall be responsible for the coordination of the landscape drawings with the other site development drawings.

2930 LANDSCAPING IRRIGATION SYSTEM

- 2931 SYSTEM DRAWINGS: A landscape irrigation system drawing has been prepared by others for this project.
- 2932 EXISTING SYSTEMS: The exact extent and location of the existing irrigation has not been shown on the drawing. The contractor and owner shall locate the existing underground pipe, heads and valves prior to

2933 CONNECTION TO POTABLE WATER SYSTEM: A tee connection shall be provided for in the potable water system for connection of the andscape irrigation systems. The landscape irrigation system will require a backflow prevention device.

- 2976 CATCH BASIN PROTECTION: A filter system shall be used on catch basin (drop inlets) in public and private streets, and parking areas as a means of sediment control. Alternate methods will require the approval
- 2977 SEDIMENT FILTER / BARRIERS: For all projects, a silt fence or straw wattle dike shall be installed along the down slope edge of the disturbed area prior to the commencement of grading. The sediment filter structures will be located so that all runoff from the construction site is filtered, or passes through a sediment detention basin prior to crossing a property line, entering a creek or entering the city storm drain basin. Sediment filter structures are to be inspected requiarly by the city inspection staff during inspection scheduled by the contractor of engineer of record, and sediment removed when the depth of sediment is no more than one half the height of the structure. Silt fences and straw wattles shall be installed according to the standard references
- 2978 SILT FENCES: Should be installed where sediment from sheet flow or rill and gully erosion will enter directly onto adjacent property. When installing, it is important the fabric material be anchored into a trench and backfilled. Maintenance of filter fences is similar to that of straw wattles in that the fabric must be inspected and needed repairs implemented after every storm event. Sediment deposits should be removed when material reaches no more than a depth of one-half the fence height.

2979 PROTECTION OF EXISTING VEGETATION: As far as is practicable,

existing vegatation shall be protected and left in place, in accordance with the clearing limits shown on the approved building, grading or public works and erosion control plans. The exception is where exotic plant materials are to be removed, or fire fuels to be reduced in accordance with an approved plan. 2980 MAINTAINANCE RESPONSIBILITY: It shall be the owner's responsibility to maintain control of the entire construction operations and to keep the

entire site in compliance with the soil erosion control plan. Owner /

contractor shall be responsible for monitoring erosion and sediment

control measures prior, during and after storm events. Monitoring includes

maintaining a file. Documenting on-site inspections, problems encountered,

- corrective actions, and notes and a red-line map of remedial implementation measures. HAULING OF MATERIALS: Reasonable care shall be taken haulina anu earth, sand, gravel, stone, debris or any hazardous substances over publi street, alley of other public street. Should any blow, spill or track over and upon said public or adjacent private property, immediate clean-up shall occur. Construction entrances shall be installed prior to commencement of grading. All construction traffic entering onto the paved
- 2982 SANITARY FACILITIES: Shall be maintained on-site as appropriate. 2983 EROSION PREVENTION: During the rainy season, all paved areas shall be kept clear of earth material and debris. All earth stockpiles over 2.0 CU.YDs shall be covered by a tarp and ringed with straw bales or silt fencing. The site shall be maintained so as to minimize sediment-laden

roads must cross the stabilized construction entranceway.

- runoff to any storm drainage system including existing drainage swales 2984 POLLUTION ABATEMENT: Construction operations shall be carried out in such a manner that erosion and water pollution will be minimized. State and local laws concerning pollution abatement shall be complied with.
- 2985 RAINY SEASON RESTRICTIONS: The facilities shown on this plan are designed to control erosion and sediment during the rainy season, November I to April 15. Facilities are to be operable prior to October 15 of any year. Grading operations during the rainy season which leave denuded slopes shall be protected with erosion control measures immediatley following grading on the slopes. This will include use of straw

2986 WINTERIZATION: This plan covers only the first winter following grading

with assumed site conditions as shown on the detailed erosion contro

evaluated and revisions made to this plan as necessary with the

August 15 of each subsequent year until site improvements are

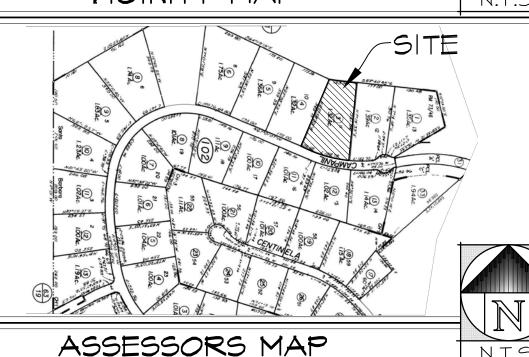
plan. Prior to September 15, the completion of site improvement shall be

approval of the city. Plans are to be resubmitted for approval prior to

mulch and tackifier, and erosion control blankets.

accepted by the city.

VICINITY MAP



PROJECT ADDRESS: 3627 CAMPANIL DR. SANTA BARBARA, CA.

UTILITIES & RELATED SITE WORK &

YES (VERY HIGH FIRE ZONE)

DR. PAULA KISLAK OWNER: STORM WATER MANAGEMENT, PROPOSED WORK: GRADING, DRAINAGE, PAVING,

EROSION CONTROL FOR ADU 047-101-003

LOT AREA: 1.32 ACRES 57,500 S.F. (AREAS BASED ON PROBER SURVEY)

FLOOD ZONE:

SEE SPEC SECTION 2256 THIS SHEET GRADING QUANTITIES:

LAND USE ZONE:

FIRE ZONE:

TOPOGRAPHICAL SURVEY BY: PROBER LAND SURVEYING DATED

4. SOILS REPORT BY PACIFIC MATERIALS LABORATORY DATED

DEC. 22, 2016 AND ADDENDUM DEC. 13, 2017. PH# (805) 964-6901

NOV. 18, 2016. PH# (805) 452-9690 2. ARCHITECTURAL DRAWINGS BY MSSION GROUP ARCHITECT ARCHITECTS. PH# (805) 969-5910

UPDATE LETTER MAY 3, 2022 REFERENCES

5. TELEPHONE

LANDSCAPE DRAWINGS BY

CITY OF SANTA BARBARA MATER CITY OF SANTA BARBARA . SEWER 3. GAS THE GAS COMPANY 4. CABLE T.V. COX COMMUNICATIONS

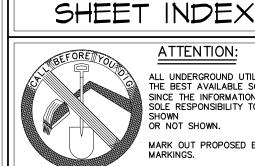
VERIZON

1-805-564-5413 1-805-564-5413 1-800-427-2200 1-805-683-6651 1-800-483-4000

6. ELECTRICITY SOUTHERN CALIFORNIA EDISON 1-800-684-8123 UTILITY COMPANY & CONTACTS

- CI.O SITEWORK SPECIFICATIONS, VICINITY MAP, ASSESSORS PARCEL MAP, SITE DATA, REFERENCES
- CI.I KEY MAP/TOPO/DEMOLITION PLAN
- C2.0 GRADING, DRAINAGE & UTILITIES PLAN, CONSTRUCTION NOTES, SYMBOLS C2.I STORM WATER MANAGEMENT, HYDROLOGY STUDY, BMP
- INSPECTION C3.0 SITE SECTIONS, EROSION CONTROL PLAN AND DETAILS
- C4.0 SITEMORK DETAILS

C5.0 RAIN HARVESTING SYSTEM



ALL UNDERGROUND UTILITIES AND SUBSTRUCTURES SHOWN HEREON WERE OBTAINED FROM THE BEST AVAILABLE SOURCES AND ARE PRESUMED TO BE ACCURATE AND COMPLETE, BUT SINCE THE INFORMATION WAS OBTAINED FROM OTHERS, IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO VERIFY, LOCATE, AND PROTECT ALL UTILITIES AND SUBSTRUCTURES

MARK OUT PROPOSED EXCAVATION AREA IN WHITE PAINT OR PROVIDE OTHER SUITABLE CALL UNDERGROUND SERVICE ALERT AT 811 FROM 6AM TO 7PM, MONDAY THROUGH FRIDAY (EXCLUDING HOLIDAYS) AT LEAST TWO BUSINESS DAYS BEFORE DIGGING. NOTIFYING UNDERGROUND SERVICE ALERT PRIOR TO EXCAVATION IS REQUIRED BY CALIFORNIA STATE

WAIT UNTIL THE UTILITY COMPANY EITHER MARKS ITS PIPELINES AND INDICATES PIPE AND DIAMETER - OR ADVISES YOU THE AREA IS CLEAR OF ITS PIPES BEFORE YOU START USE ONLY HAND TOOLS WITHIN 24 INCHES OF EACH MARKED UTILITY LINE TO DETERMINE TH

EXACT LOCATIONS OF ALL LINES BEFORE USING ANY POWER EXCAVATION EQUIPMENT IN THI AREA. MEASURE THE 24 INCHES FROM THE INDICATED OUTSIDE DIAMETER ON EACH SIDE OF

LAW. FAILURE TO
COMPLY CAN CARRY HEAVY FINES. UNDERGROUND SERVICE ALERT WILL CONTACT LOCAL
UTILITIES TO MARK THE LOCATION OF ALL UTILITY—OWNED LINES FOR FREE.

UNDERGROUND SERVICE ALERT

RCE 38168 Exp. 3/25/2025

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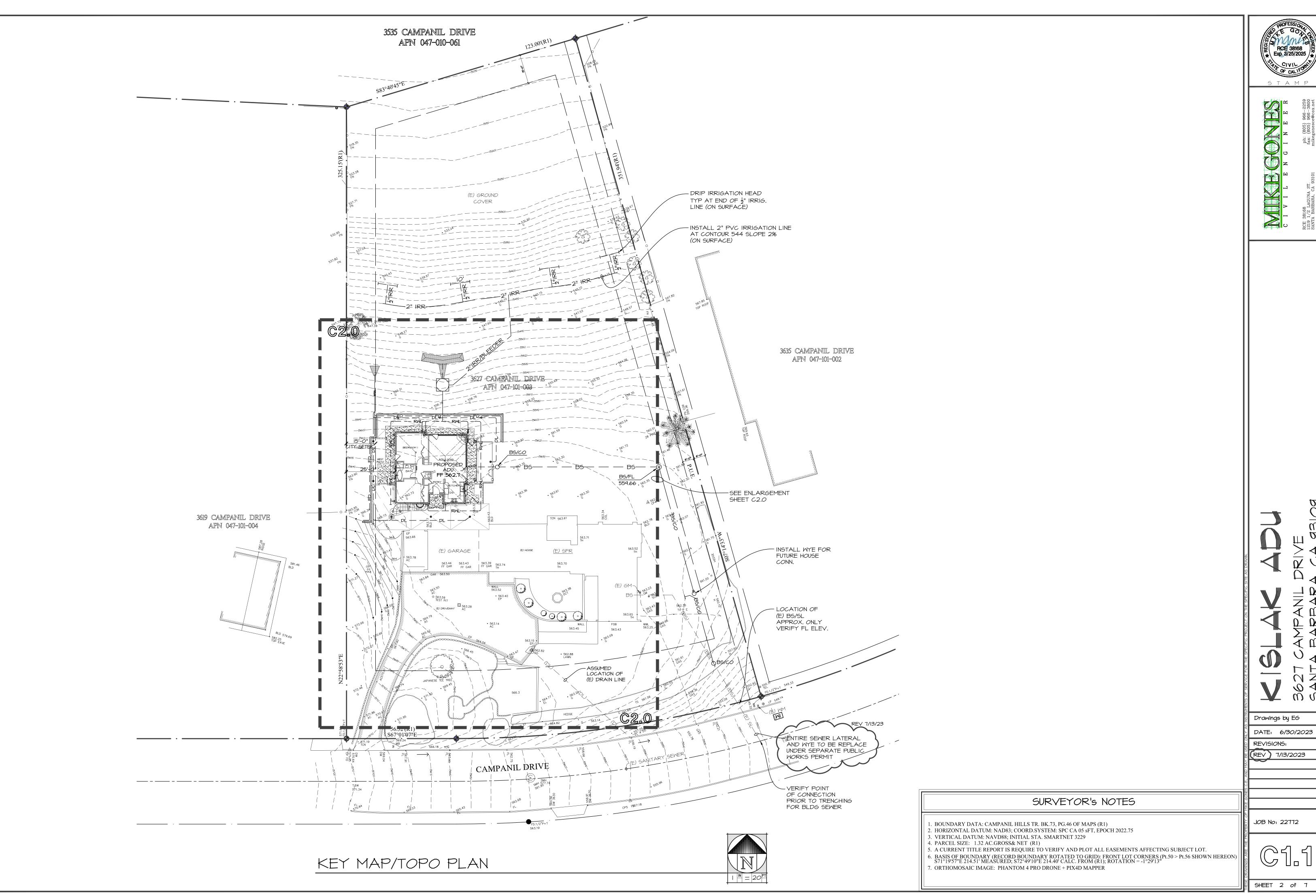
JOB No: 22772

4

Drawings by EG DATE: 6/30/2023

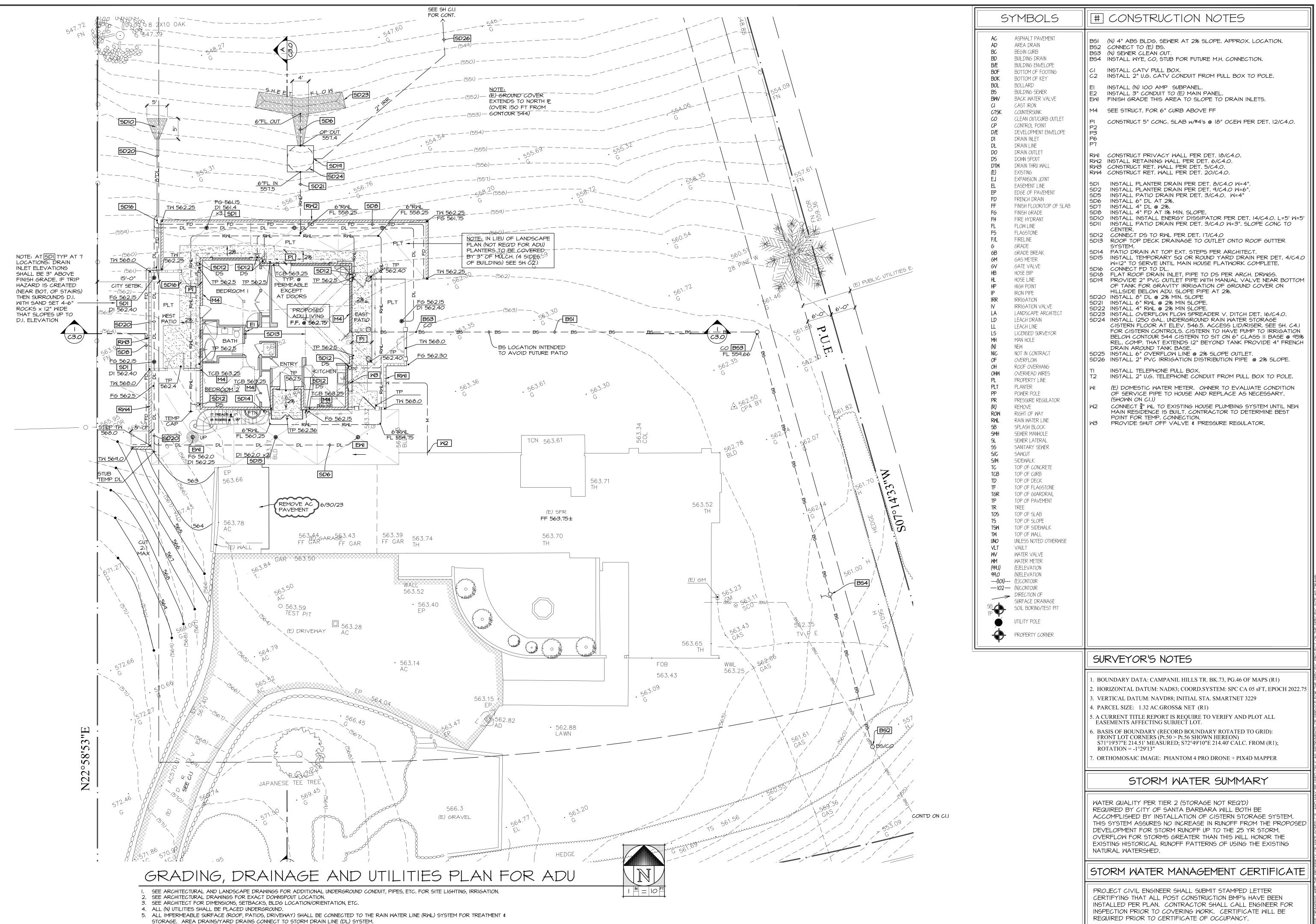
REVISIONS:

SHEET I of 7





(REV) 7/13/2023





V I L E N G I N E

RCE 38168 1219 1/2 LAGUNA SANTA BARBARA,

A DONA DARA A DON 3627 CAMPANIL DRIVE SANTA BARBARA, CA 43109

JOB No: 22772

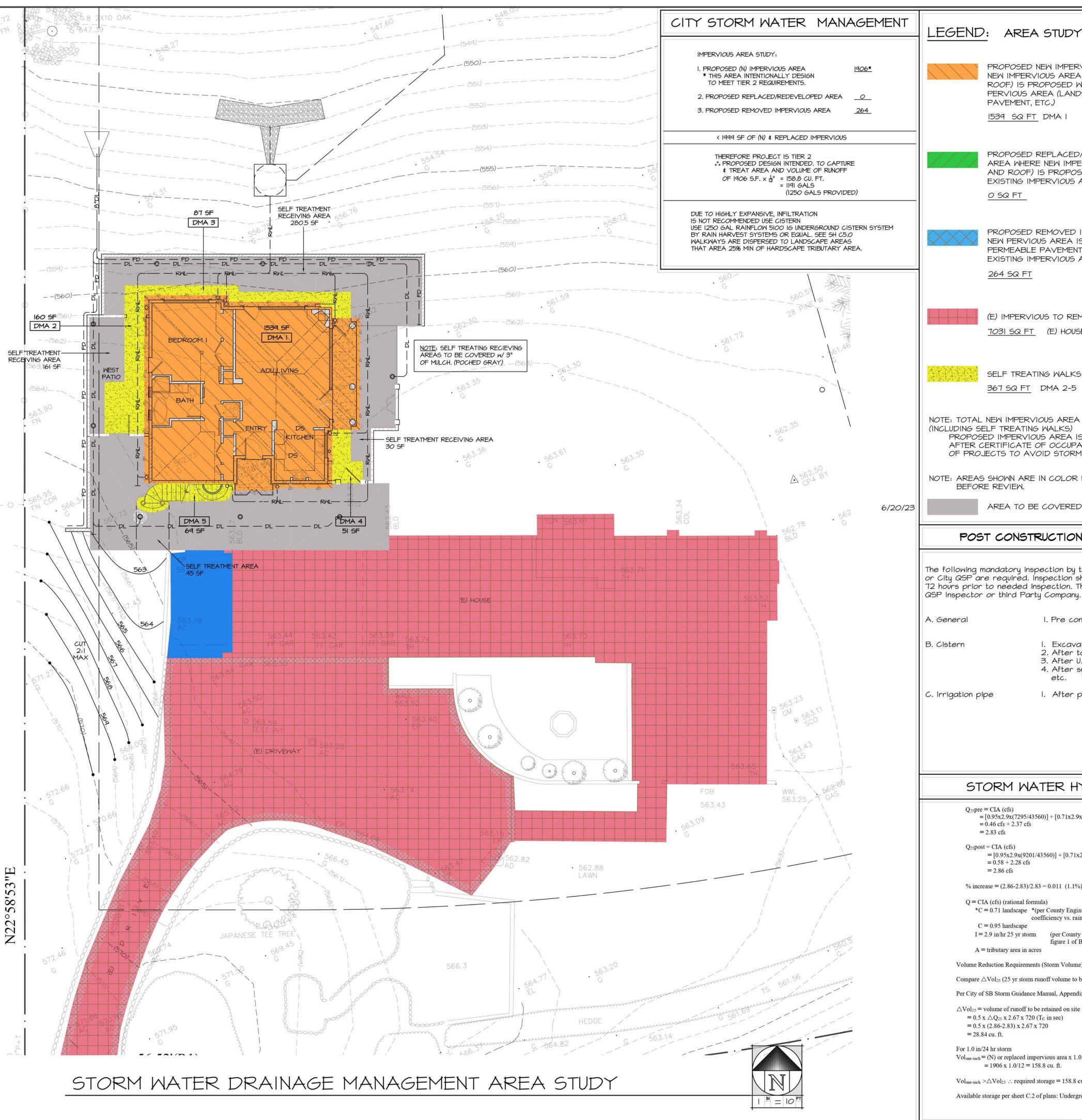
Drawings by EG

REVISIONS:

DATE: 6/30/2023

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SHEET 3 of 7



LEGEND: AREA STUDY

PROPOSED NEW IMPERVIOUS AREA- AREA WHERE NEW IMPERVIOUS AREA (E.G. HARDSCAPE AND ROOF) IS PROPOSED WHERE THERE IS EXISTING PERVIOUS AREA (LANDSCAPING, PERMEABLE

PAVEMENT, ETC.)

1539 SQ FT DMA 1

PROPOSED REPLACED/REDEVELOPED IMPERVIOUS AREA-AREA WHERE NEW IMPERVIOUS AREA (E.G. HARDSCAPE AND ROOF) IS PROPOSED WHERE THERE IS CURRENTLY EXISTING IMPERVIOUS AREA (E.G. HARDSCAPE AND ROOF)

O SQ FT

PROPOSED REMOVED IMPERVIOUS AREA- AREA WHERE NEW PERVIOUS AREA IS PROPOSED (LANDSCAPING, PERMEABLE PAVEMENT, ETC) WHERE THERE IS CURRENTLY EXISTING IMPERVIOUS AREA (E.G. HARDSCAPE AND ROOF)

264 SQ FT

(E) IMPERVIOUS TO REMAIN

7031 SQ FT (E) HOUSE & DRIVEWAY)

SELF TREATING WALKS (DRAINS TO VEGETATION YARD/SELF TREATMENT AREAS) 367 SQ FT DMA 2-5 AREA REQ'D = .25x367 = 92 SF AREA PROVIDED = 517 SF

(INCLUDING SELF TREATING WALKS) = 1539 + 367 = 1906 SF PROPOSED IMPERVIOUS AREA IS CUMULATIVE FOR TWO YEARS AFTER CERTIFICATE OF OCCUPANCY TO PREVENT PIECE MEALING OF PROJECTS TO AVOID STORM WATER MANAGEMENT.

NOTE: AREAS SHOWN ARE IN COLOR PLEASE PRINT IN COLOR BEFORE REVIEW.



AREA TO BE COVERED IN 3" OF MULCH.

POST CONSTRUCTION BMP INSPECTION

The following mandatory inspection by the city Building inspector or City QSP are required. Inspection shall be called in by Contractor 72 hours prior to needed inspection. The City will then route to the QSP Inspector or third Party Company.

A. General

I. Pre construction meeting attendance.

B. Cistern

 Excavation 2. After tanks set on certified class II base.

3. After U.G. piping is complete. 4. After system is complete with pumps/controls

1. After pipe \$ irriq. heads are installed.

STORM WATER HYDROLOGY STUDY

 Q_{25} pre = CIA (cfs) = [0.95x2.9x(7295/43560)] + [0.71x2.9x(50205/43560)]

= 0.46 cfs + 2.37 cfs= 2.83 cfs

 $Q_{25}post = CIA (cfs)$

= [0.95x2.9x(9201/43560)] + [0.71x2.9x(48299/43560)]= 0.58 + 2.28 cfs

= 2.86 cfs

% increase = (2.86-2.83)/2.83 = 0.011 (1.1%)

Q = CIA (cfs) (rational formula) *C = 0.71 landscape *(per County Engineering Design Standards, runoff

coefficiency vs. rainfall figure 2 of Blue Book) C = 0.95 hardscape

I = 2.9 in/hr 25 yr storm(per County Flood Control intensity duration curves, figure 1 of Blue Book)

A = tributary area in acres

Volume Reduction Requirements (Storm Volume):

Compare △Vol₂₅ (25 yr storm runoff volume to be retained on site) to 1.0 in/24 hr storm.

Per City of SB Storm Guidance Manual, Appendix C p. C-3:

 $\triangle Vol_{25}$ = volume of runoff to be retained on site (cu. ft.) = $0.5 \times \Delta Q_{25} \times 2.67 \times 720$ (T_C in sec) = 0.5 x (2.86-2.83) x 2.67 x 720

= 28.84 cu. ft.For 1.0 in/24 hr storm

 $Vol_{one-inch} = (N)$ or replaced impervious area x 1.0 in/12 in/ft $= 1906 \times 1.0/12 = 158.8 \text{ cu. ft.}$

 $Vol_{one-inch} > \triangle Vol_{25}$: required storage = 158.8 cu. ft. x 7.5 gal. ft3 = 1191 gal.

Available storage per sheet C.2 of plans: Underground Cisterns = 1 x 1250 gal. each = 1250 gal.

STORM WATER MANAGEMENT STUDY

THE FOLLOWING INFORMATION IS BASED ON COUNTY STORM WATER BMP GUIDANCE MANUAL & GRADING PLAN AND DRAINAGE BY MIKE GONES CIVIL ENGINEER, DATED MARCH 2023.

PROJECT IS HILLSIDE RESIDENTIAL WITH LESS THAN 1999 SF NEW IMPERVIOUS THEREFORE TIER 2.

2. SITE ASSESSMENT A. HILLSIDE DESIGN DISTRICT

B. ARROYO BURRO WATERSHED

I. POLLUTANTS OF CONCERN:

C. TYPE SOIL D / CLAY D. SITE SLOPES MOSTLY NORTH > BLDG PAD AREA = 5%

E. NO GROUND WATER ANTICIPATED F. NO FLOOD HAZARD

NUTRIENTS

BACTERIA

- NOT AN ISSUE, PROPERTY LINE IS FENCED - NOT AN ISSUE CONNECTED TO PUBLIC SEWER - NOT AN ISSUE OTHER THAN FROM DURING CONSTRUCTION (EROSION CONTROL

PLAN WILL BE PREPARED) HYDROCARBON - DRIVEWAY RUNOFF (TREATED BY FILTRATION SYSTEM)

- NOT AN ISSUE FOR RESIDENTIAL PROJECT

PESTICIDE

3. SOILS REPORT: SOIL TYPE: EXPANSIVE CLAY. STEEP HILLSIDE, MINIMAL INFILTRATION. 4. SITE DESIGN BMP OPTIONS: (E) RUNOFF PATTERNS TO BE HONORED. GRADING TO RESPECT (E)

5. BASIC BMP'S - TIER 2, SEE STORM WATER RUNOFF BMP's.

6. STORM WATER BMP OPTION SELECTED: CISTERN.

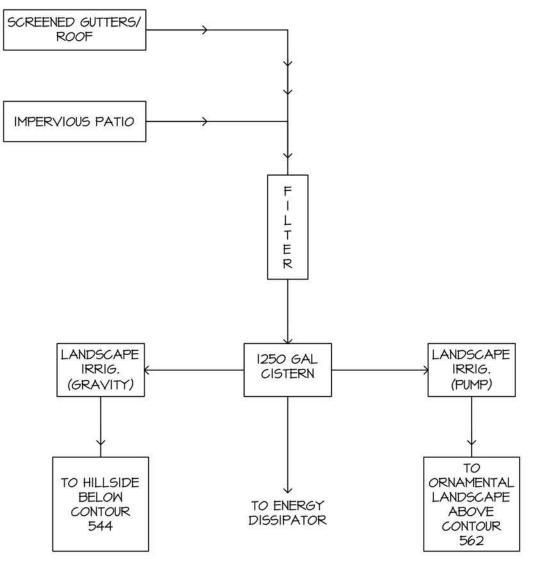
7. AREA STUDY FOR HYDROLOGY STUDY SEE SEPARATE STUDY THIS SHEET FOR CITY SWMP AREAS.

A. IMPERVIOUS AREAS (SQ. FT.)	(E)	PROPOSED	CHANGE
I. ROOF AREA	2994	4533	+ 1539
2. PATIO/WALK	1067	1434	+ 367
3. DRIVEWAY	3234	3234	0
4. SUB-TOTAL	7295	9201	+ 1906
B. PERVIOUS (SQ. FT.)	(E)	PROPOSED	PROPOSED
I. PATIO/WALK	0	0	0
2. DRIVEWAY	0	0	0
3. LANDSCAPE	50,205	48,299	- 1906
3. SUB-TOTAL	50,205	48,299	- 1906
TOTAL A4 + B3	57,500	57,500	0

NOTES: I. INCREASE IN IMPERVIOUS < 2000 S.F. .. TIER 2.

8. PROPOSED IMPERVIOUS AREA = 1906 SF x 1/2 = 158.8 CU. FT. (Vwq) x 7.5 GAL/CU. FT. = 1191 GAL. SEE HYDROLOGY STUDY THIS SHEET FOR MORE INFORMATION.

IMPERVIOUS AREA DATA: NEW IMPERVIOUS AREA = 1906 S.F. REPLACED IMPERVIOUS AREA = 0 S.F. REMOVED IMPERVIOUS AREA = 0 S.F.



NOTE: SEE SITE PLAN C2.0 FOR IRRIGATION CONTROL/VALVES THAT USE RAINWATER FOR LANDSCAPE IRRIGATION.

STORM WATER BMP MAINTENANCE AGREEMENT

THE PROPOSED STORM WATER BEST MANAGEMENT PRACTICES WILL BE MAINTAINED PURSUANT TO SBMC 22.87.050. PROPOSED BEST MANAGEMENT PRACTICES: UNDERGROUND CISTERNS CONNECTED TO LANDSCAPING AND PERMEABLE PATH SELF TREATING WALKWAYS.

Parolatis (at 6/30/23

PROPERTY OWNER SIGNATURE PAULA KISLAK

STORM WATER MANAGEMENT CERTIFICATION

BEFORE BUILDING INSPECTOR WILL GRANT CERTIFICATE OF OCCUPANCY AND FINALIZE THE BUILDING PERMIT, THE BUILDING, AND SAFETY DIVISION OR THEIR CONTRACTED QSP SERVICE PROVIDER MUST VERIFY THAT ALL POST CONSTRUCTION STORM WATER BMP'S WERE INSTALLED AS APPROVED AND THAT THEY COMPLY WITH THE CITY'S TIER 3 STORM WATER REQUIREMENTS.

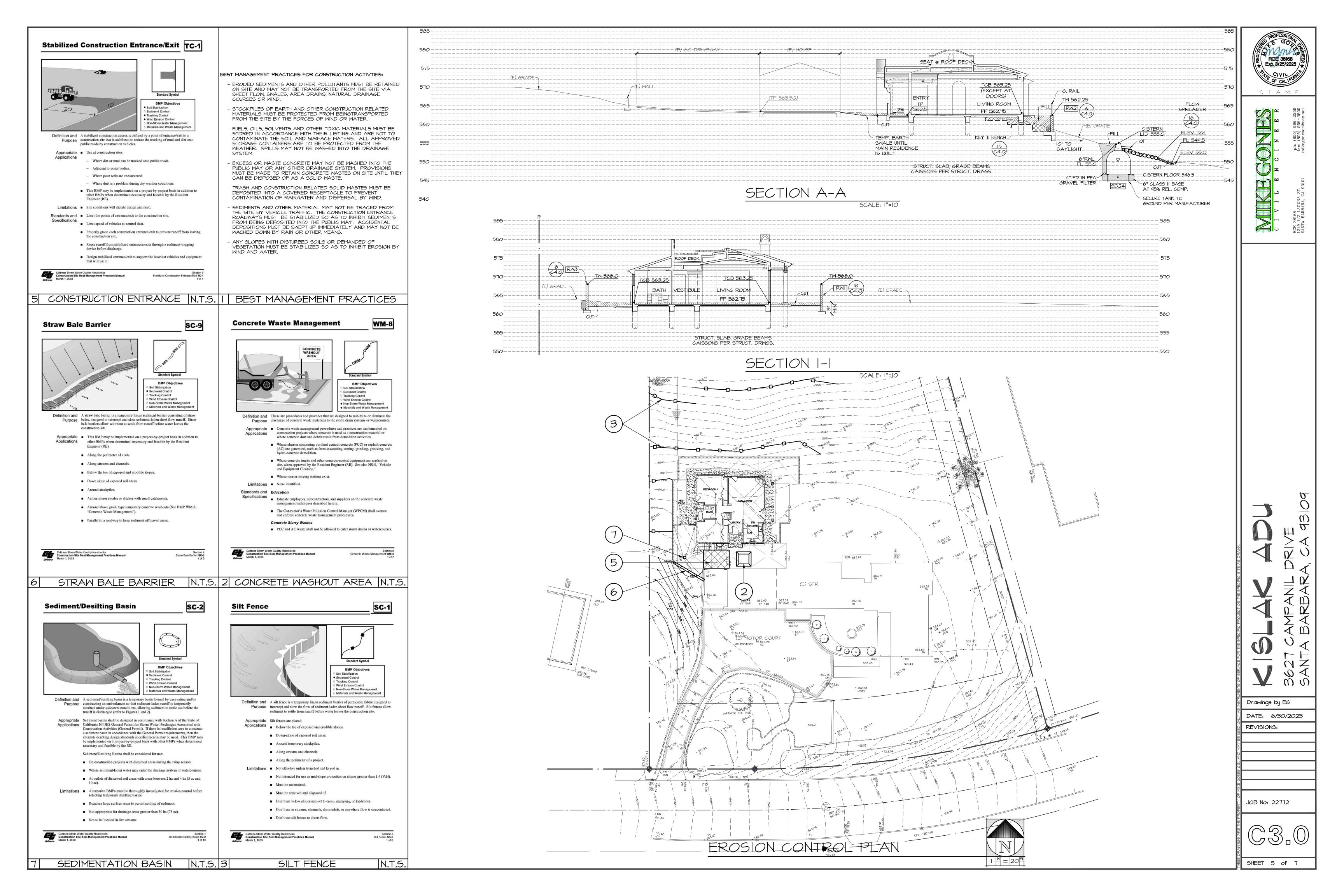


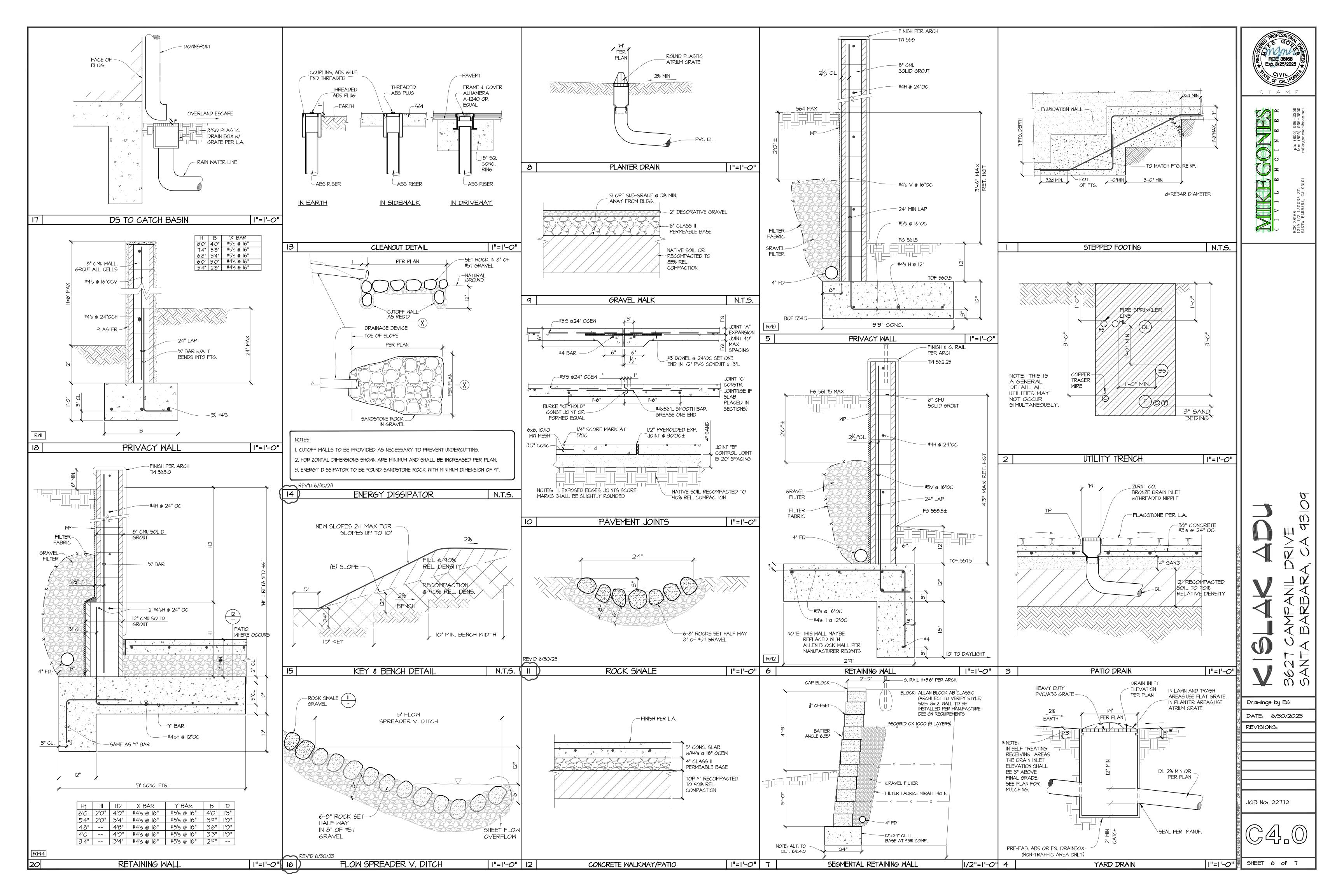
SHEET 4 of 7

Drawings by EG DATE: 6/30/2023

REVISIONS:

JOB No: 22772





4475 Alicia Lane Cumming, GA 30028 770-889-2533 Sales@RainHarvest.com

Rainwater Harvesting Systems Submittal For:

RainFlo 5100 IG

Complete Rainwater Harvesting System

Page 1

NOTES: FOR RAIN HARVESTING SYSTEM

- I. INFO SHOWN HERE IS FOR GENERAL PERMIT PURPOSES. GO TO WWW.RAINHARVEST.COM FOR FULL BROCHURE.COM FOR FULL BROCHURE, DETAILS & INSTALLATION REQT'S.
- 2. CONTRACTOR MAY ELECT TO SUBSTITUTE COMPARABLE SYSTEM IF APPROVED IN ADVANCE BY THE PROJECT ENGINEER.
- 3. THIS SYSTEM IS ANTICIPATED TO REQUIRE I-1250 GAL.
- 4. ALL SYSTEM DETAILS TO BE WORKED OUT IN ADVANCE WITH PROJECT ENGINEER AND LANDSCAPE ARCHITECT PRIOR TO ORDERING AND INSTALLING.
- 5. SEE SHEET CI.I PLAN FOR CONNECTION TO IRRIGATION SYSTEM. FINAL PUMP SIZE TO BE SELECTED BASED ON PLANTING PLAN & IRRIGATION DEMAND.

RainHarvest Systems



Dimensions with Maxi Tank Dome:

<u>n n n n n n</u>

Graf Carat-S Rainwater Tanks

The new generation of Carat rainwater underground tanks has been specially developed for rainwater harvesting

The Carat-S Underground Tank:

The lineup consists of four modular tank units ranging from 700 gallons to 1,700 gallons which are expandable up to thousands of gallons. The precision, modular, and ultra-high strength design of the Carat-S makes it the choice of professionals worldwide. The Graf Carat tank is guaranteed not to collapse when empty when installed per the manufacturer's specifications.



Carat-S Rainwater Tank Features:

- 15 Year manufacturer's warranty Suitable for vehicle loading (when
- combined with the cast iron lid option) Easy to transport and install Variable installation depth with double Ultra-high strength materials and design sealed telescopic riser
- Internal pre-filtration option

Attractive locking green lid

 Frost-proof installation underground Groundwater stable to the middle of the tank Convenient 31-1/2" manway opening

Internal Filter Package Option:

Specially designed for rainwater harvesting, the Graf Optimax Pro® internal self-cleaning filter uses patented filter technology to filter debris from roof areas up to 3,750 sq. ft.

- Greatly simplified installation
- Only one manway and lid in the yard Provides over 95% water yield
- Self-cleaning
- Very low maintenance



RainHarvest Systems | www.RainHarvest.com | 770-889-2533

Dimensions with Mini Tank Dome: 13.25" (Mini Riser)

Tank	700 US Gallons	1000 US Gallons	1250 US Gallons	1700 US Gallons	1700 US Gallons (Expansion Tank)
Part No.	372001	372002	372003	372004	372014
Weight	265 lb.	331 lb.	408 lb.	485 lb.	485 lb.
L	82"	90"	90"	94"	94"
W	62"	69"	78"	86"	86"
Н	55"	62.5"	71.5"	82.5"	82.5"
Htot*	79"	86.5"	95.5"	106.5"	NA
Htot**	66"	73.5"	82.5"	93.5"	NA

RainHarvest Systems | www.RainHarvest.com | 770-889-2533



Overflow Siphon (with tank pkg. only) RainHarvest Systems, LLC 6075 Parkway North Drive Cumming, GA 30040 Tel: 770-889-2533 Fax: 770-889-2577 F www.RainHarvest.com

Part Number

340037

Optimax® Pro Internal Filter

US Adaptation by RainHarvest Systems

Internal filter technology Optimax Pro, self-cleaning Filter **4** 5 2 Advantages Provides over 95 % water yield Low maintenance (self-cleaning) Space-saving filter technology insid the tank Filter housing can be easily removed without tools Transparent lid for filter visibility Can manage roof areas up to 350 sqm Standard 100 mm connections Self cleaning Opticlean system available as an optional extra Low offset height of 165 mm between inlet and outlet 4" Overflow 4" Filtered water

Optimax-Pro Filter internal Order no. 340037

outlet to tank

Accessories Cleaning unit Opticlean® internal without hose Order no. 340040

Quick assembly sleeve Spannfix patented (Page 47) Order no. 340502

XL lift out device for a convenient withdrawal of filter cover and filter insert, length 505 mm Order no. 330220

Q Webcode G2102

Cleaning unit Opticlean® Very intense water jet for cleaning the filter sieve

Very smooth surface and, therefore, max. self cleaning, mesh width 0.35 mm Routine maintenance intervals are kept to a minimum · An automatic activation of the cleaning unit is carried out together with the automatic filter cleaning unit and the Aqua-Center-Silentio

3-layer filter Diamond pattern as carrying surface Stainless steel fine filter

Connecting dimensions for telescopic dome shaft

Filter cartridge

Dome shaft Tank overflow overflow 19.5-27.4" 25.9-33.9" Mini Maxi 405-705 mm 660-060 mm

(RAINFLO

Universal Rainwater Pumps

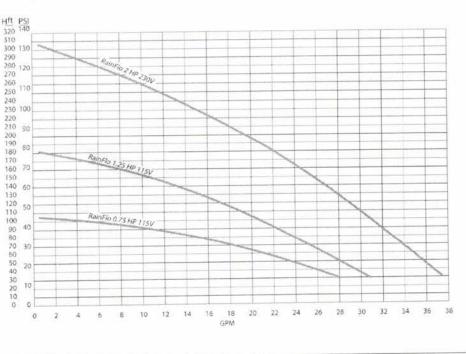
High performance multi-stage rainwater pumps for residential, commercial, and lightindustrial rainwater collection systems.

Submersible and External Mounted Capability:

RainFlo universal pumps are specially designed for the unique requirements of rainwater collection systems. Equipped with a large threaded bottom inlet large for internal flow-based cooling and connection to a floating filter, these pumps can be installed vertically or horizontally and they can either be submersed inside a tank or mounted externally on the ground or other platform. Other features include a stainless steel base, adjustable float switch for run-dry protection, external capacitor housed in a wiring box with circuit breaker and master on/off switch for long life and ease of maintenance.



Pump Performance:



RainHarvest Systems | www.RainHarvest.com | 800-654-9283

Durable, Dependable and High Performance:

Available in 0.75HP/115V, 1.25HP/115V and 2.0HP/230V models, construction consists of 304 stainless steel housings, dual Italian mechanical seals, American thermal protection, GE-Noryl diffuser and impellers, external starting capacitor and a 45 foot power cord. The pump is particularly quiet and durable from its solid construction. The water end is installed under the motor which keeps the motor cooled with the pumped water. The Noryl impellers and diffusers offer high abrasion resistance while the Italian double mechanical seals ensure long life and enhanced reliability.

The oil chamber is filled with non-toxic cooling oil. Ball bearings are self-lubricating and internal cast iron components are electrocoated with polybutadiene varnish to prevent corrosion which is sometimes associated with the typical lower pH of rainwate r.

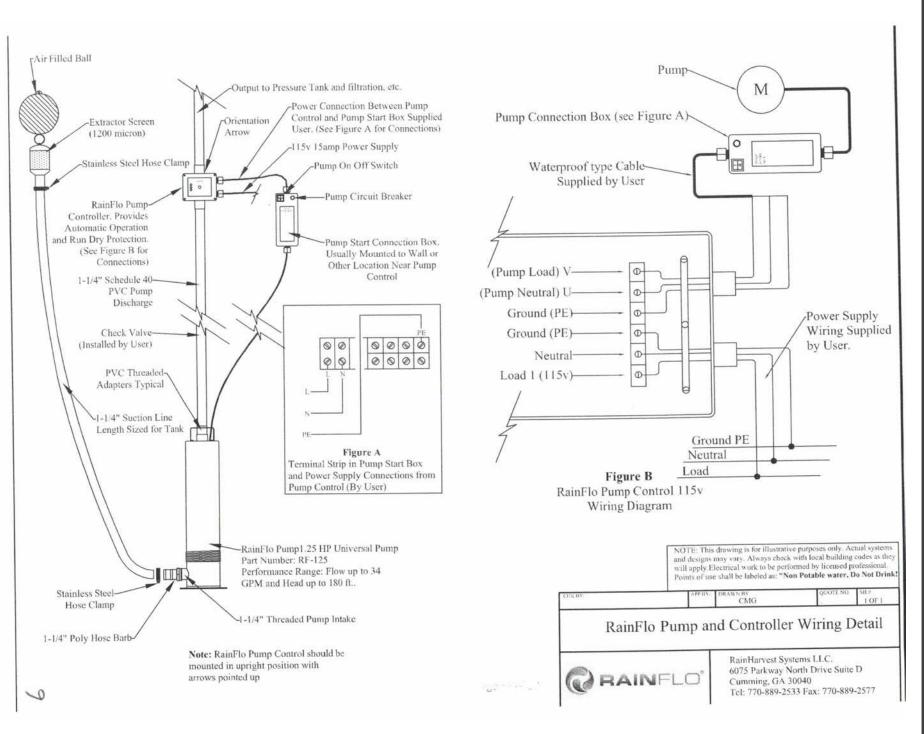
Installation may be oriented either vertical or horizontal so long as water is available at the intake to prevent a rundry condition.

Specifications:

RainFlo Submersible Pumps								
Model No.	RF075-S	RF125-S & SC	RF200-S					
Horsepower:	0.75	1.25	2.0					
Nominal Voltage Range:	115V, 60Hz, 8.6A max.	115V, 60Hz, 14A max	230V, 60Hz, 9A max.					
P1kW:	1.0	1.24	2.3					
P2kW:	0.6	0.95	1.6					
Impeller stages:	2	3	5					
Maximum flow:	29 GPM	34 GPM	36 GPM					
GPM at 50 psi (0 Head):	See Curve	17 GPM	29 GPM					
GPM at 40 psi (0 Head):	8 GPM	22 GPM	31 GPM					
Maximum head:	105' TDH	180' TDH	310' TDH					
System pressure:	Up to 46 PSI	Up to 78 PSI	Up to 135 PSI					
Inlet/Outlet size:	1-1/4" FPT	1-1/4" FPT	1-1/4" FPT					
Weight:	36 Lbs.	41 Lbs.	46 Lbs.					
Dimensions:	7" X 7" X 20"(incl. base)	7" X 7" X 22" (incl. base)	7" X 7" X 24" (incl. base					
Thermal protection:	Yes	Yes	Yes					
Motor:	2-pole induction, Continuous duty	Same	Same					
RPM:	3450	3450	3450					
Cooling:	Water cooled/intake	Water cooled/intake	Water cooled/intake					
Insulation class:	F	F	F					
Protection:	IP68	IP68	IP68					
Certifications:	CE	CE	CE					
Warranty:	1 Year	1 Year	1 Year					

RainHarvest Systems | www.RainHarvest.com | 800-654-9283





Carl-Zeiss-Str. 2-6 Telefon 07641/589-0 Telefax 07641/589-50

Email: info@graf-online.de

Otto Graf GmbH

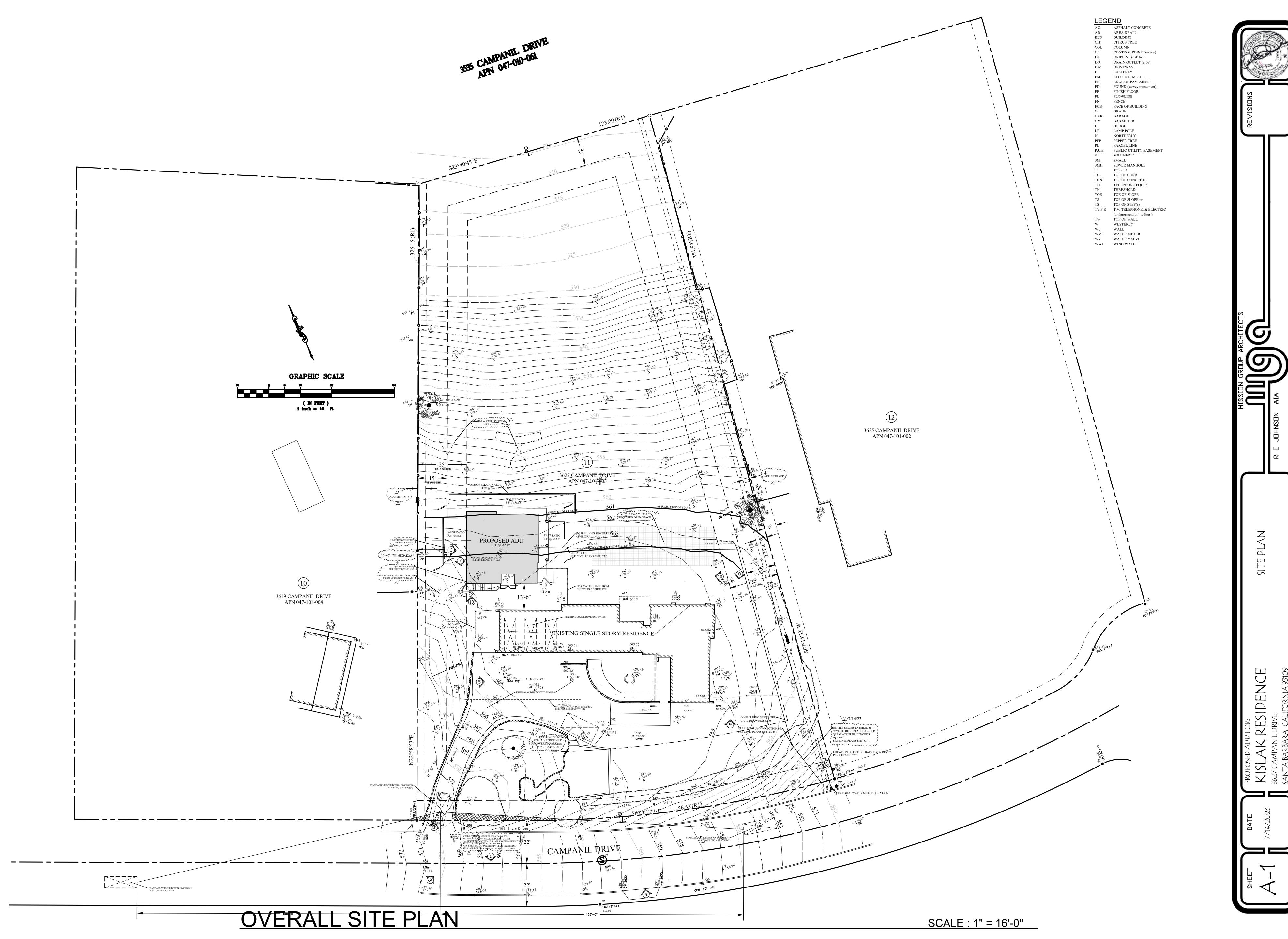
'9331 Teningen

Drawings by EG

DATE: 6/30/2023 REVISIONS:

JOB No: 22772

SHEET 7 of 7



					DC	OR S	CHED	ULE			
ŤO	TYPE		SIZE		MATL		DETAILS A-6.1		REMARKS		
MARK NO.	L	W	Н	T	MA	Н	H J S		TEMARIO		
1		3'-6"	9'-0"	1¾"	SC WD.	X/AX	X/AX	X/AX	SOLID CORE WOOD PLANK FINISH, RADIUS ARCH TOP		
2		PR. 6'-0"	9'-0"		STL,/GLS	5/A6.1	8/A6.1	11/A6.1	THREE LIGHT FRENCH DOOR w/TEMPERED GLASS		
3		3'-6"	9'-0"	1¾"	SC WD.				INTERIOR POCKET DOOR		
4		3'-0"	8'-0"	1¾"	STL,/GLS	5/A6.1	8/A6.1	12/A6.1	3 LIGHT FRENCH DOOR w/TEMPERED GLASS		
5		PR. 5'-0"	8'-0"	1¾"	SC WD.	10/A6.1			INTERIOR DOOR		
6		3'-0"	8'-0"	1¾"	SC WD.	10/A6.1			INTERIOR DOOR		
7		2'-8"	8'-0"	1¾"	SC WD.	10/A6.1			INTERIOR DOOR		
8		2'-0"	6'-8"	1" 2	TEMP GL.				TEMPERED GLASS SHOWER DOOR		
(ý)	\triangle	2'-8"	8'-0"	1¾"	SC WD.	10/A6.1			50% OPEN LOUVER INT. DOOR, APPROX. 950 S.I. NFA		
10		3'-0"	8'-0"	1¾"	SC WD.	10/A6.1			INTERIOR DOOR		
11		PR.2'-0"	8'-0"	1¾"	SC WD.	10/A6.1			INTERIOR DOOR		
12		2'-2"	7'-0"	1¾"	SC WD.				VERT. PLNK. DR. w/LVRED. 12"x12" opn'g tp. and btm.		
13		2'-4"	8'-0"	1¾"	SC WD.				INTERIOR DOOR		
14		PR.1'-6"	5'-0"		WRT. IRON				WROUGHT IRON GATE, DESIGN TBD		
15		4'-0"	5'-8"	1¾"	SC WD.				SOLID CORE WOOD PLANK GATE, FLAT ARCH TOP		
16		2'-9"	6'-0"		WRT. IRON				WROUGHT IRON GATE, DESIGN TBD		
17		4'-0"	6'-0"		WRT. IRON				WROUGHT IRON GATE, DESIGN TBD		
18		4'-0"	6'-0"		WRT. IRON				WROUGHT IRON GATE, DESIGN TBD		
19											
20											

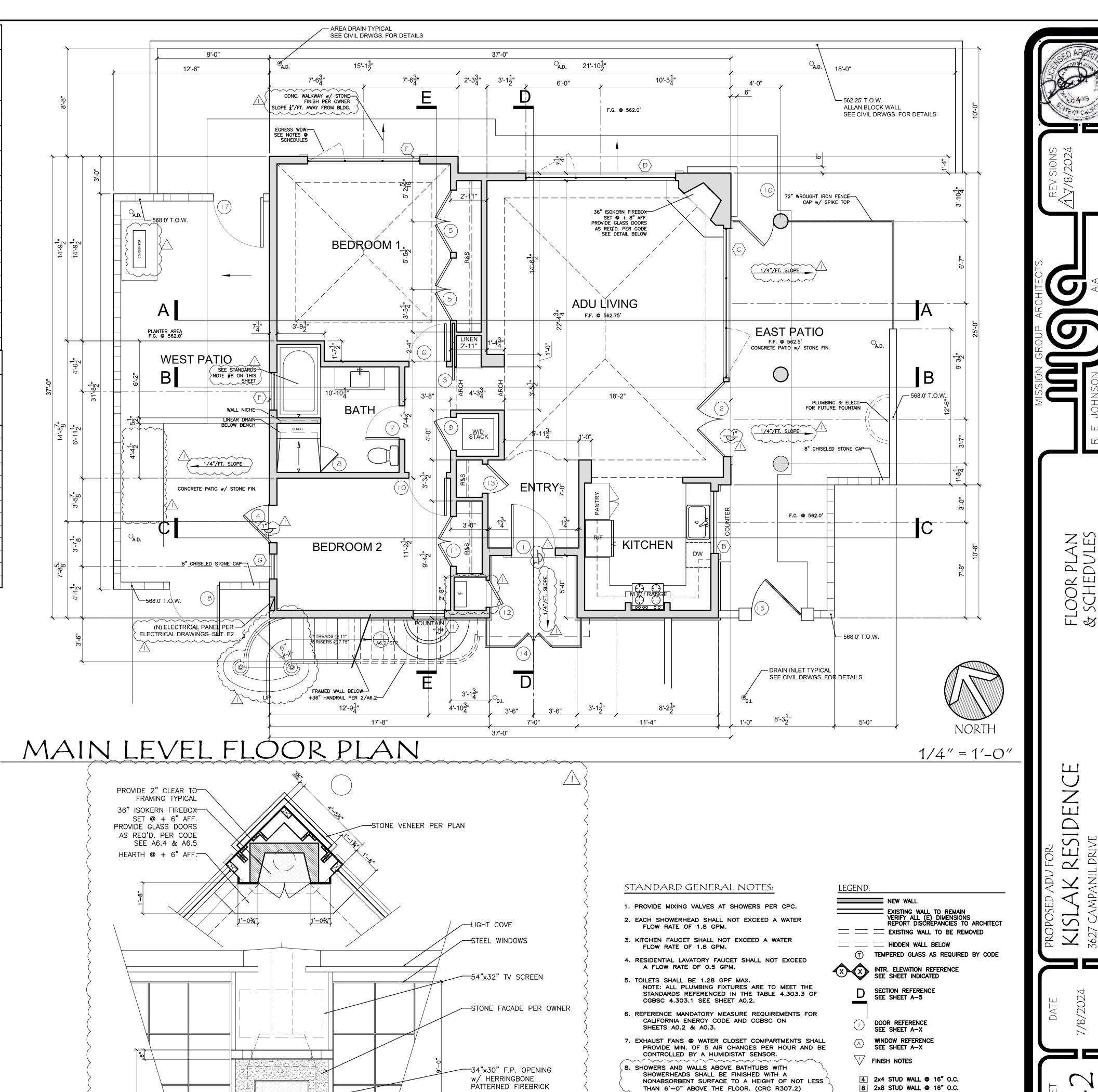
					٧	VINDO/	W SCH	HEDULE			
Ô	FRAME SIZE][DE SHT. A	TAILS -6.1 U	NO	MFG.	QTY.	REMARKS	
MARK NO.	<u> </u>	W	Η	NATL	Ι	J	S	ROOM			TALIWIA TAL
Α											
В	CSMT.	5'-0"	4'-0"	STL/GL	5/A6.1	6/A6.1	2/A6.1	KITCHEN		1	
С	FIXED	12'-0"	8'-4"	STL/GL	5/A6.1	6/A6.1	7/A6.1	LIV. RM.		1	12 LIGHT STL. FRAME w/ TEMP. GLASS
D	FIXED	12'-0"	8'-4"	STL/GL	5/A6.1	6/A6.1	7/A6.1	LIV. RM.		1	12 LIGHT STL. FRAME w/ TEMP. GLASS
E	FIXED	9'-0"	7'-4"	STL/GL	5/A6.1	6/A6.1	7/A6.1	BDRM. 1		1	9 LIGHT STL. FRAME w/ TEMP. GLASS
F	CSMT.	3'-0"	4'-0"	STL/GL	1/A6.1	3/A6.1	1/A6.1	BATH 1		1	6 LIGHT STL. FRAME w/TEMP. GLASS
G	CSMT.	2'-0"	7'-4"	STL/GL	1/A6.1	3/A6.1	1/A6.1	BDRM. 2		1	10 LIGHT STL. FRAME w/ TEMP. GLASS
Н	CSMT.	2'-6"	5 ' -0"	STL/GL	5/A6.1	6/A6.1	7/A6.1	BDRM. 2		1	EGRESS WINDOW-SEE NOTES
1											
J											
K											
L											
				I							

DOOR NOTES:

- 1. THE FOLLOWING DOORS SHALL HAVE MATCHING COLOR ALUMINUM SCREENS: 2.4.
- 2. ALL EXTERIOR DOORS TO BE DUAL GLAZED WITH A MINIMUM OF ONE TEMPERED PANE.
 MUNTINS (IF APPLICABLE) SHALL BE "AUTHENTIC DIVIDED LIGHT".
- 3. PANEL DOORS TO BE (2) PANEL AS MANUFACTURED BY T.M. COBB. TYPICAL INTERIOR DOORS WITH V.G. DOUGLAS FIR RAILS. PANELS TO BE MULTIFIBER BOARD UNLESS OTHERWISE NOTED.
- 4. ALL EXTERIOR DOORS SHALL BE WEATHERSTRIPPED. PEMKO 356DV ASTRAGAL ON INACTIVE LEAF OF EACH PAIR OF DOORS.
- 5. ALL DOORS TO BE PROTECTED FROM MOISTURE AND PRIMED AND SEALED (TOP AND BOTTOM) IMMEDIATELY UPON ARRIVAL TO SITE. GENERAL CONTRACTOR TO ASSUME THE RESPONSIBILITY FOR CONDITION OF DOORS DURING CONSTRUCTION PERIOD. ALL PRIMING OF DOORS TO BE PER DOOR MANUFACTURER'S SPECIFICATIONS. IF DOOR TOPS AND BOTTOMS ARE RECUT AFTER PRIMING OR PAINTING, THEY ARE TO BE REPRIMED OR PAINTED IMMEDIATELY.
- 6. HARDWARE NOT SPECIFIED IN NOTES OR ABOVE SHALL BE INCLUDED IN FINISH HARDWARE ALLOWANCE.
- 7. DOORS WHICH HAVE SPECIAL DECORATIVE GLASS SPECIFIED ARE TO COME FROM MANUFACTURER WITHOUT GLAZING UNLESS OTHERWISE NOTED.
- 8. ALL EXTERIOR DOORS SHALL BE HOPE DOOR/WINDOW SYSTEMS w/ DUAL 20% > LEED-H ENERGY STAR GLAZING. MAX. U-FACTOR OF .35 & MAX. SHGC OF .35 PER ENERGY CALCS. ON SHEET A-X.O.

WINDOW NOTES:

- 1. ALL OPERABLE CASEMENT OR PARTIAL CASEMENT WINDOWS SHALL HAVE REMOVABLE ALUMINUM SCREENS. COLOR: MATCH WINDOW COLOR.
- 2. WINDOWS SHALL RECEIVE SHADING DEVICES PER ENERGY CALCS. (IF REQUIRED IN TITLE 24 NOTES.)
- 3. ALL WINDOWS TO BE DUAL GLAZING WITH MINIMUM ONE PANE TEMPERED. MUNTINS SHALL BE "AUTHENTIC DIVIDED LIGHT".
- 4. ALL WINDOWS TO PROTECTED FROM MOISTURE AND PRIMED AND SEALED (TOP AND BOTTOM)
 IMMEDIATELY UPON ARRIVAL TO SITE. GENERAL CONTRACTOR TO ASSUME THE RESPONSIBILITY FOR
 CONDITION OF WINDOWS DURING CONSTRUCTION PERIOD. ALL PRIMING OF WINDOWS TO BE PER
 MANUFACTURER'S SPECIFICATIONS.
- 5. SCHEDULED "CUSTOM" WINDOWS SHALL CONFORM TO HEAD, JAMB, AND SILL DETAILS. VERIFY DETAILS OF EACH SPECIFIC WINDOW CONFIGURATION AND CONSTRUCTION WITH ARCHITECT PRIOR TO FABRICATION.
- 6. ALL DIMENSIONS SHOWN ON WINDOW SCHEDULE ARE FINISHED OPENINGS. VERIFY R.O.
- 7. HARDWARE NOT SPECIFIED IN NOTES OR ABOVE SHALL BE INCLUDED IN FINISH HARDWARE ALLOWANCE.
- 8. ALL FACTORY WINDOWS TO BE FACTORY PRIMED UNLESS OTHERWISE NOTED.
- 9. WINDOWS WHICH HAVE SPECIAL DECORATIVE GLASS SPECIFIED ARE TO COME FROM MANUFACTURER WITHOUT GLAZING UNLESS OTHERWISE NOTED.
- 10. ALL EGRESS WINDOWS SHALL HAVE AT MINIMUM: 20" WIDTH CLEARANCE, 24" HEIGHT CLEARANCE, 5.7 SF OPENING & SHALL HAVE A MAX. SILL HEIGHT OF NO MORE THAN 44" ABOVE FINISH FLOOR.
- 11. ALL WINDOWS SHALL BE MILGARD ALUMINUM SERIES w/ DUAL 20% ≥ LEED-H ENERGY STAR GLAZING. MAX. U-FACTOR OF.35 AND MAX. SHGC OF .35. SEE ENERGY CALCS. ON SHEET A-X.0



-STONE SILL PER OWNER

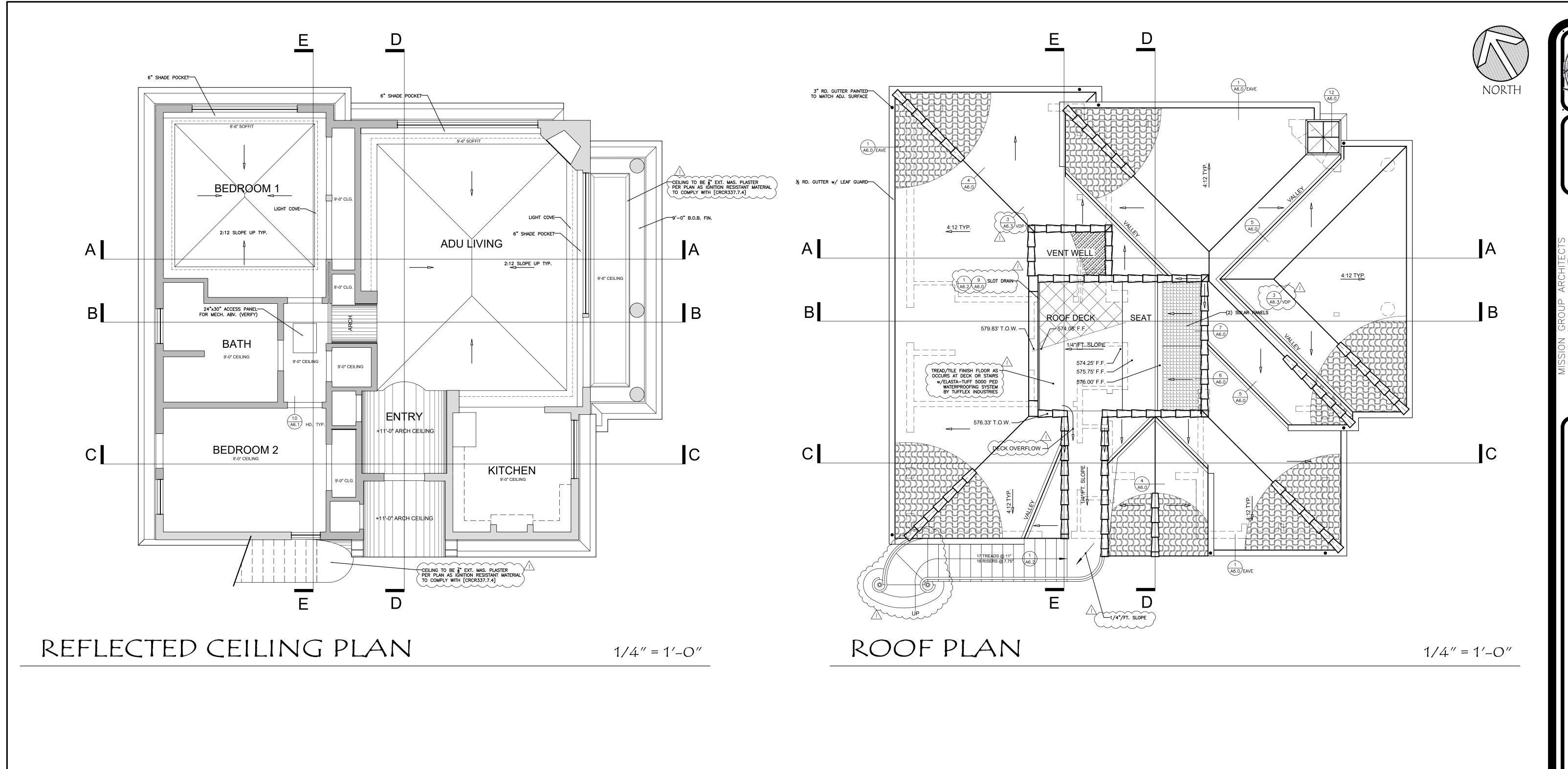
-STONE HEARTH PER OWNER

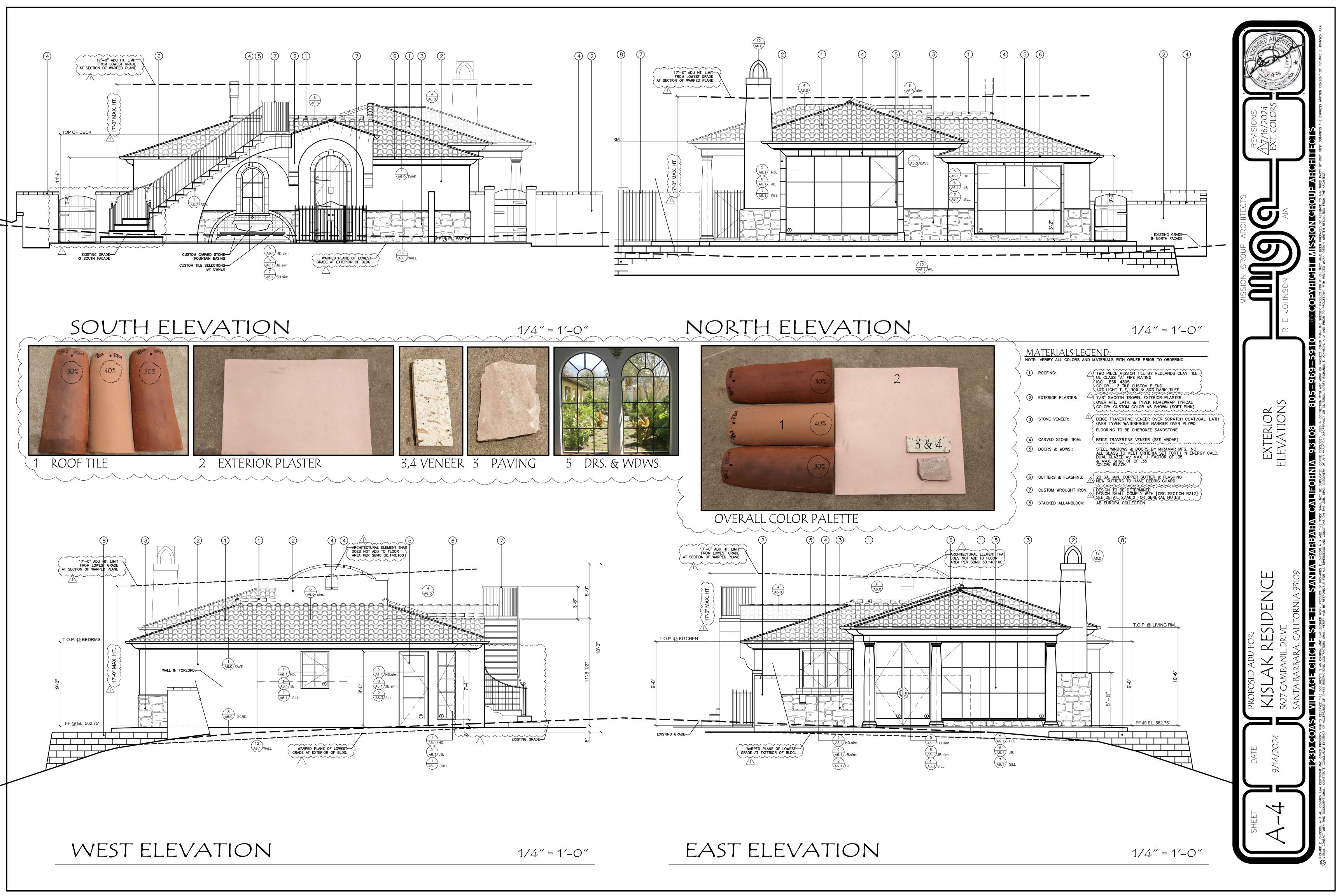
3/8" = 1'-0"

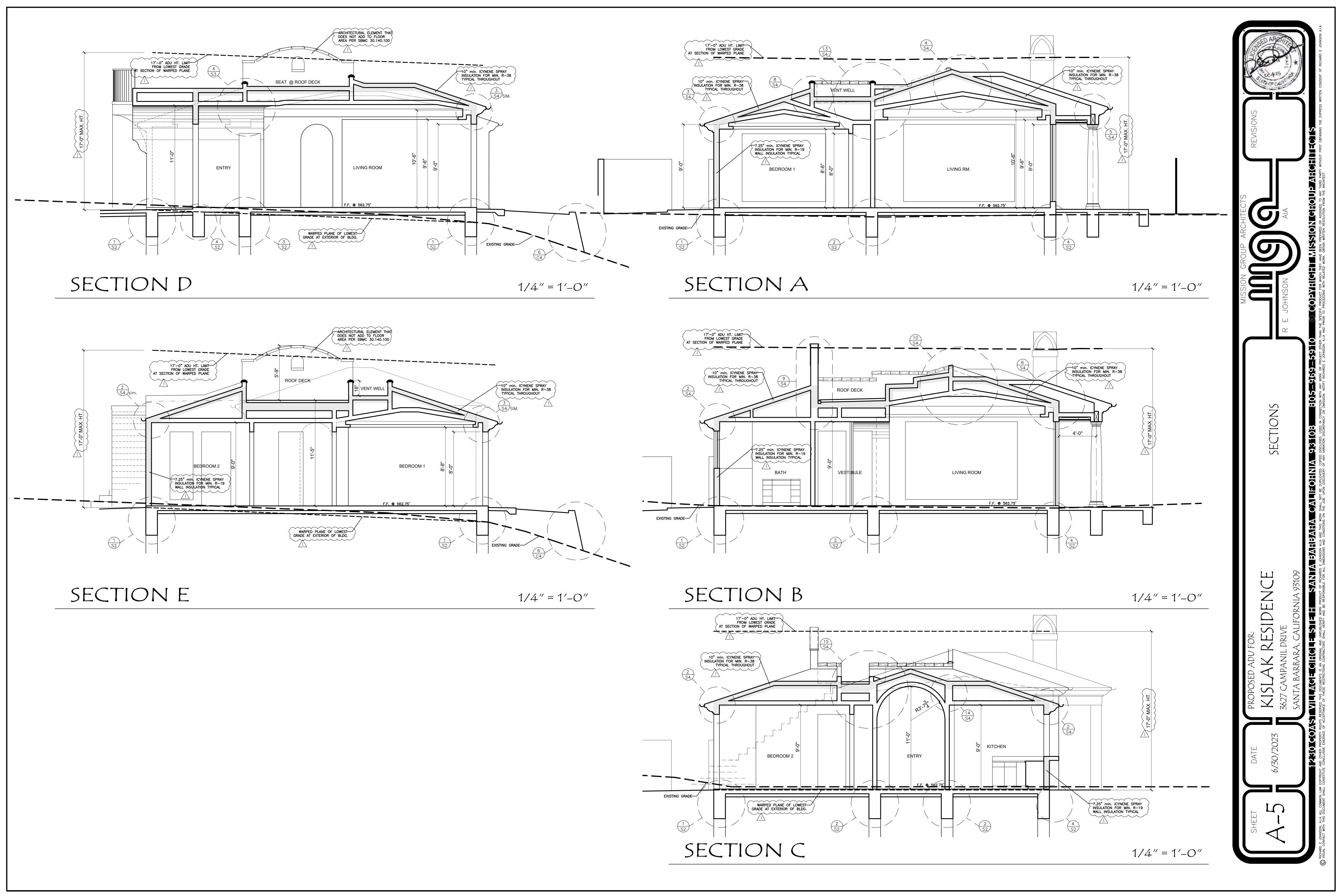
FIREPLACE DETAIL

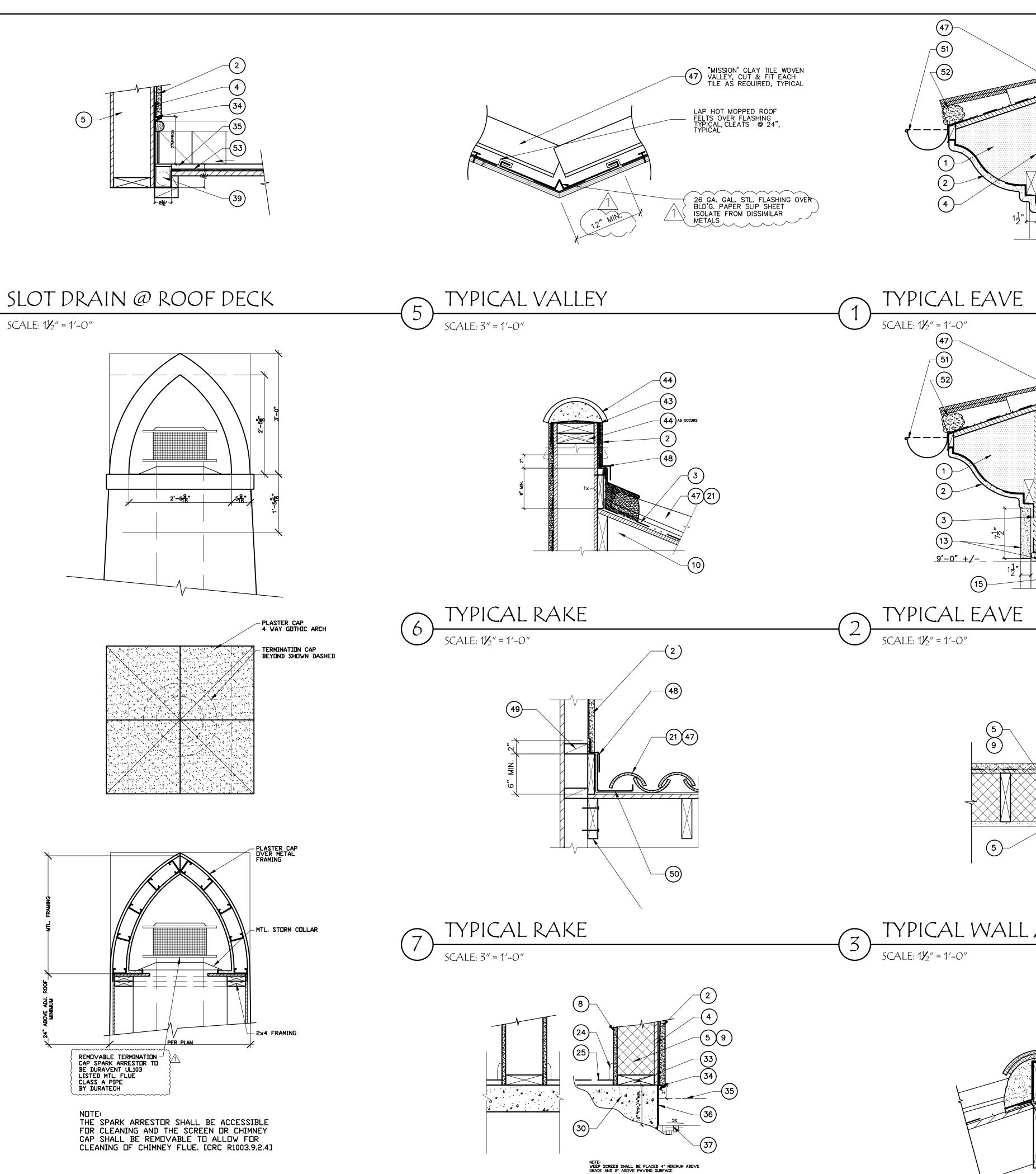
10 2x10 STUD WALL • 16" O.C.

12 2x12 STUD WALL • 16" O.C.









TYPICAL BASE SCREED

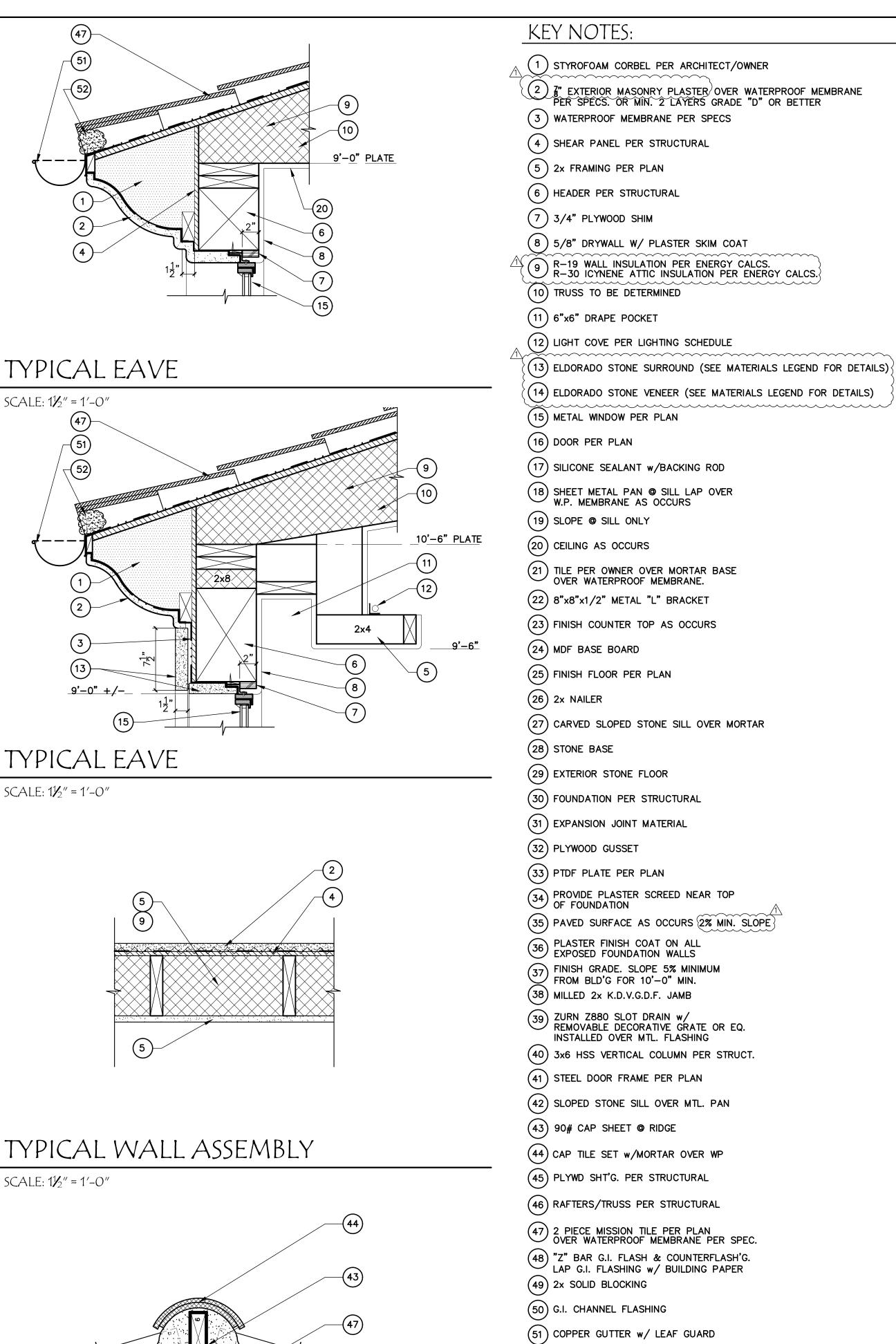
SCAIF: 3'' = 1'-0''

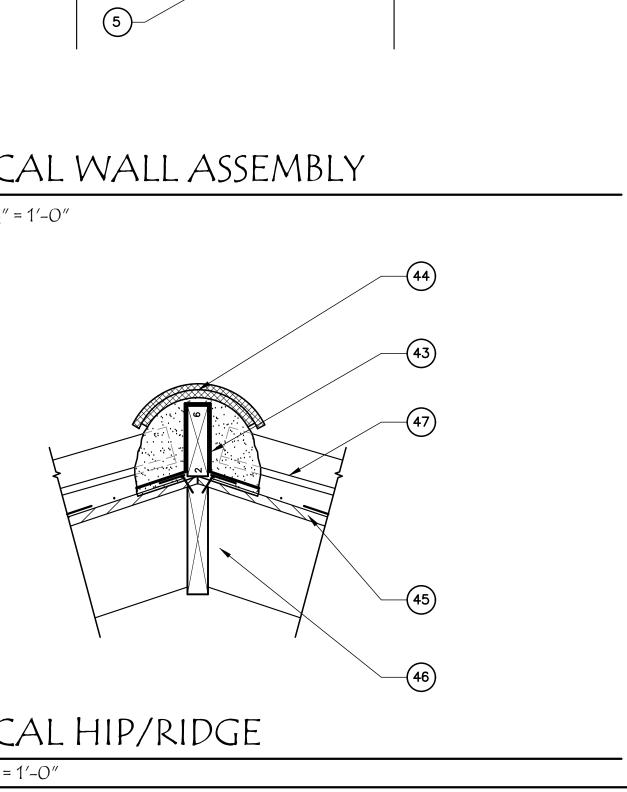
FACTORY BUILT CHIMNEYS SHALL BE LISTED AND LABELED AND SHALL BE INSTALLED AND

TERMINATED IN ACCURDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. [CRC R1005.1]

CHIMNEY CAP

SCAIF: 3/4" = 1'-0"



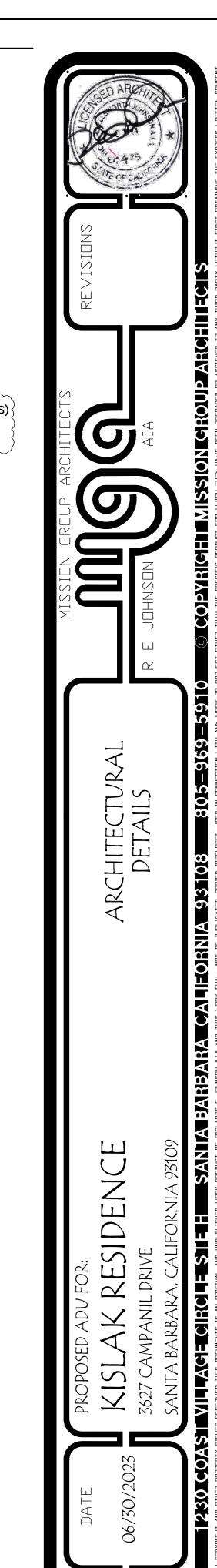


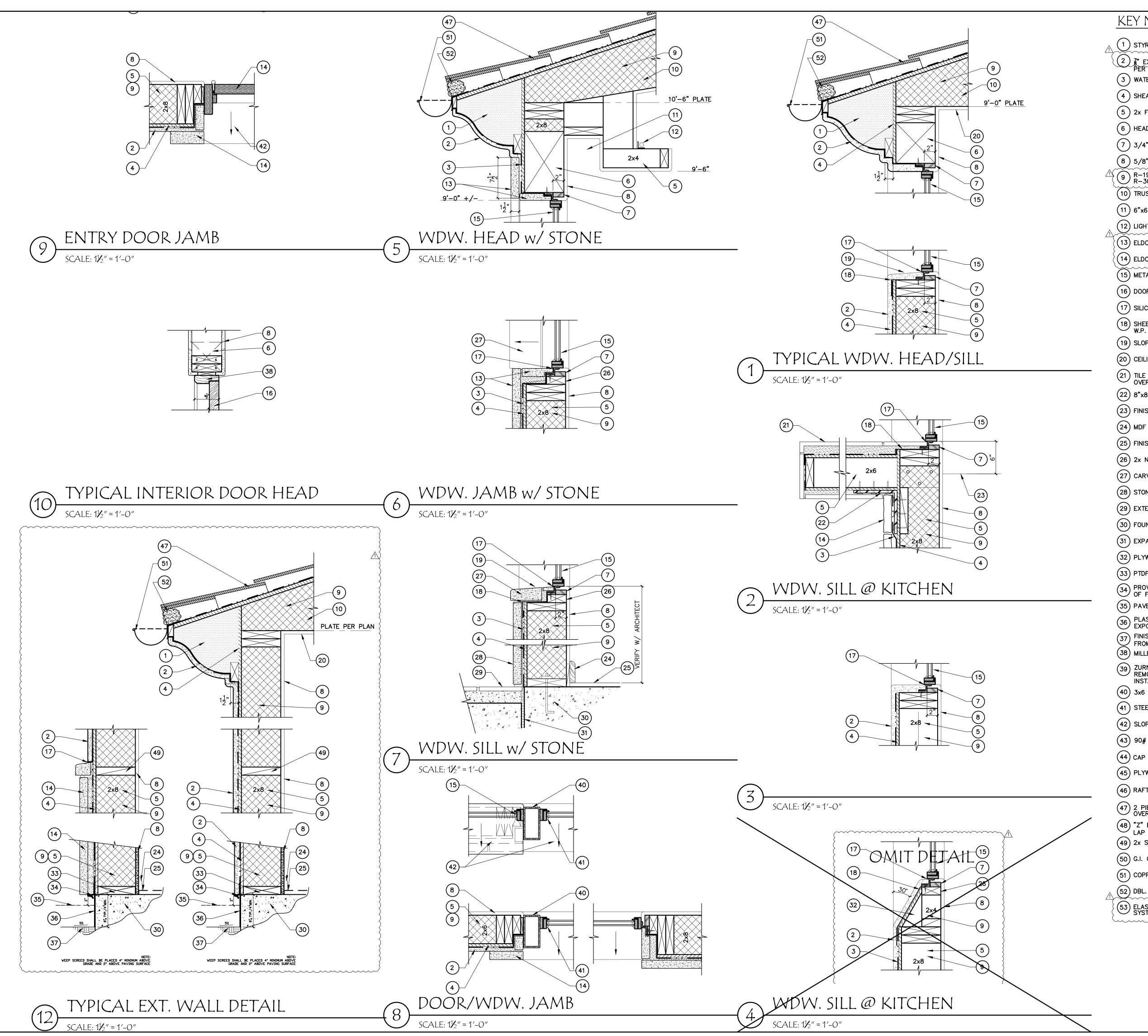
(52) DBL. STARTER TILE W/ BIRD BLOCK MORTAR

53 ELASTA-TUFF 5000/6000 PED WATERPROOFING SYSTEM BY TUFFLEX AT EXT. STAIRS AND ROOFDECK

TYPICAL HIP/RIDGE

SCALE: 3'' = 1'-0''





KEY NOTES:

1) STYROFOAM CORBEL PER ARCHITECT/OWNER

2 F" EXTERIOR MASONRY PLASTER OVER WATERPROOF MEMBRANE PER SPECS. OR MIN. 2 LAYERS GRADE "D" OR BETTER

(3) WATERPROOF MEMBRANE PER SPECS

4 SHEAR PANEL PER STRUCTURAL

(5) 2x FRAMING PER PLAN

6 HEADER PER STRUCTURAL

7 3/4" PLYWOOD SHIM

(8) 5/8" DRYWALL W/ PLASTER SKIM COAT

9 R-19 WALL INSULATION PER ENERGY CALCS. R-30 ICYNENE ATTIC INSULATION PER ENERGY CALCS.

(10) TRUSS TO BE DETERMINED

(11) 6"x6" DRAPE POCKET

(12) LIGHT COVE PER LIGHTING SCHEDULE

(13) ELDORADO STONE SURROUND (SEE MATERIALS LEGEND FOR DETAILS)

(14) ELDORADO STONE VENEER (SEE MATERIALS LEGEND FOR DETAILS)

(15) METAL WINDOW PER PLAN

16 DOOR PER PLAN

(17) SILICONE SEALANT w/BACKING ROD

18) SHEET METAL PAN @ SILL LAP OVER W.P. MEMBRANE AS OCCURS

(19) SLOPE @ SILL ONLY

20 CEILING AS OCCURS

21) TILE PER OWNER OVER MORTAR BASE OVER WATERPROOF MEMBRANE.

(22) 8"x8"x1/2" METAL "L" BRACKET

(23) FINISH COUNTER TOP AS OCCURS

24 MDF BASE BOARD

25) FINISH FLOOR PER PLAN

26) 2x NAILER

(27) CARVED SLOPED STONE SILL OVER MORTAR

(28) STONE BASE

(29) EXTERIOR STONE FLOOR

(30) FOUNDATION PER STRUCTURAL

31) EXPANSION JOINT MATERIAL

32) PLYWOOD GUSSET

(33) PTDF PLATE PER PLAN

PROVIDE PLASTER SCREED NEAR TOP OF FOUNDATION

35) PAVED SURFACE AS OCCURS 2% MIN. SLOPE

PLASTER FINISH COAT ON ALL EXPOSED FOUNDATION WALLS

FINISH GRADE. SLOPE 5% MINIMUM FROM BLD'G FOR 10'-0" MIN.

(38) MILLED 2x K.D.V.G.D.F. JAMB

ZURN Z880 SLOT DRAIN w/ REMOVABLE DECORATIVE GRATE OR EQ. INSTALLED OVER MTL. FLASHING (40) 3x6 HSS VERTICAL COLUMN PER STRUCT.

(41) STEEL DOOR FRAME PER PLAN

(42) SLOPED STONE SILL OVER MTL. PAN

(43) 90# CAP SHEET @ RIDGE

(44) CAP TILE SET W/MORTAR OVER WP

(45) PLYWD SHT'G. PER STRUCTURAL

(46) RAFTERS/TRUSS PER STRUCTURAL

2 PIECE MISSION TILE PER PLAN OVER WATERPROOF MEMBRANE PER SPEC.

"Z" BAR G.I. FLASH & COUNTERFLASH'G. LAP G.I. FLASHING w/ BUILDING PAPER

(49) 2x SOLID BLOCKING

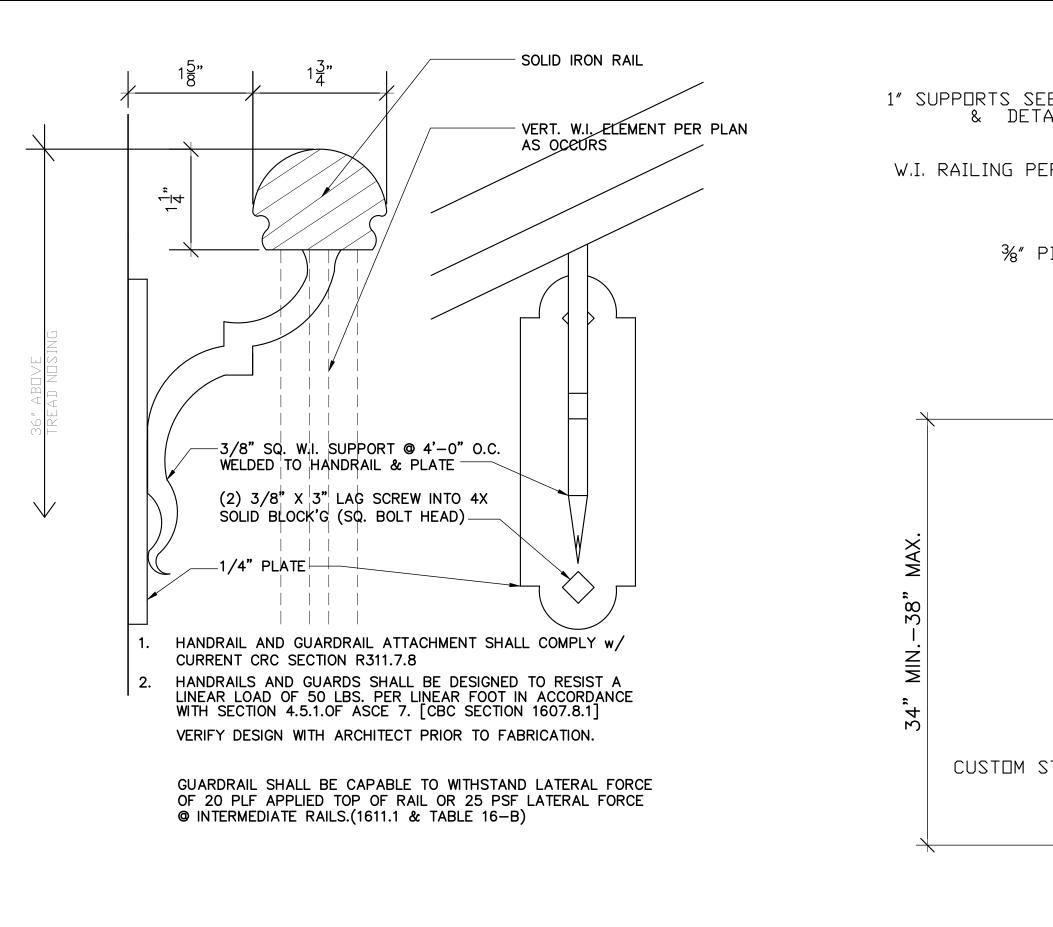
(50) G.I. CHANNEL FLASHING

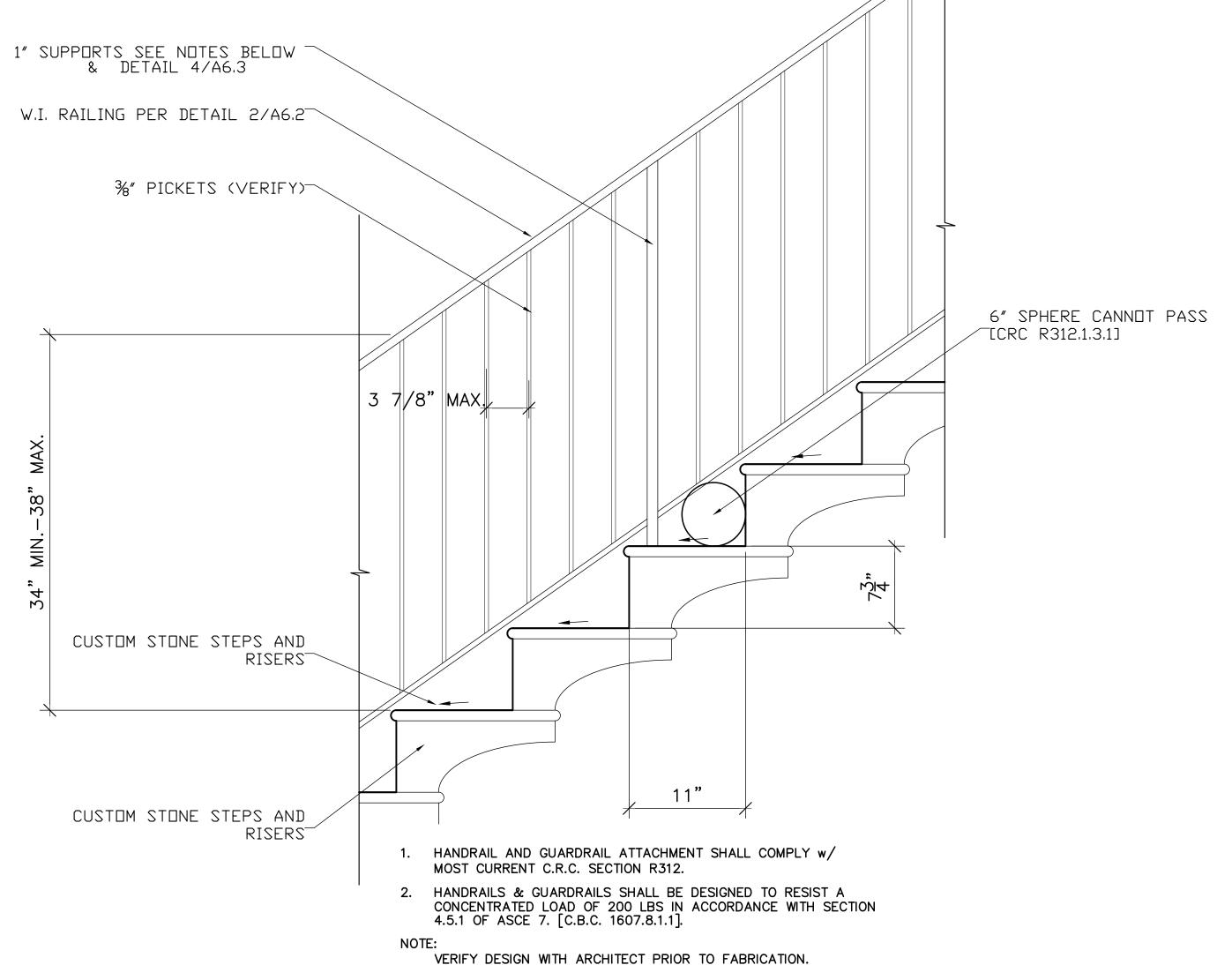
(51) COPPER GUTTER w/ LEAF GUARD

(52) DBL. STARTER TILE W/ BIRD BLOCK MORTAR

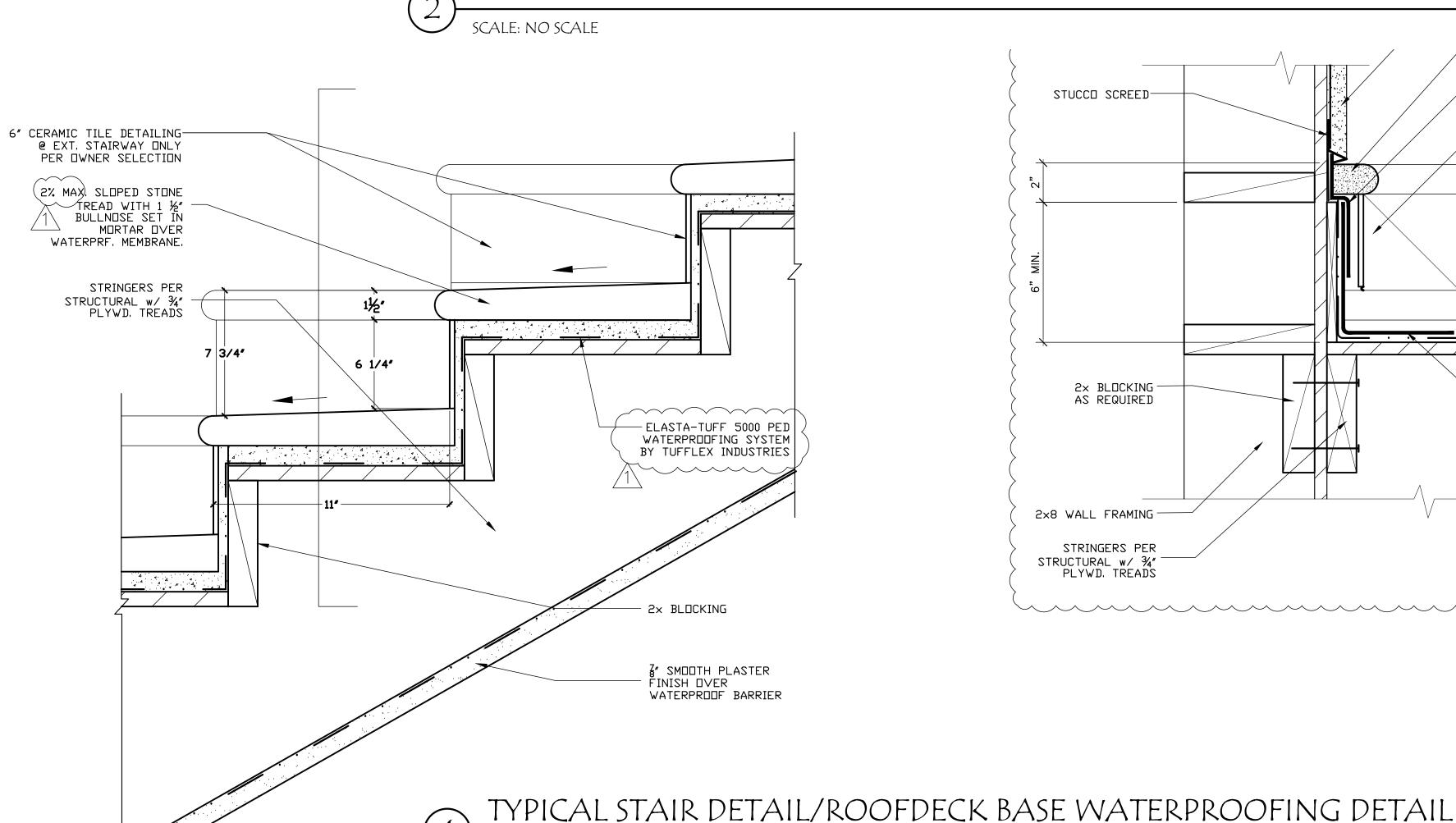
53 ELASTA-TUFF 5000/6000 PED WATERPROOFING SYSTEM BY TUFFLEX AT EXT. STAIRS AND ROOFDECK

SIDENCE

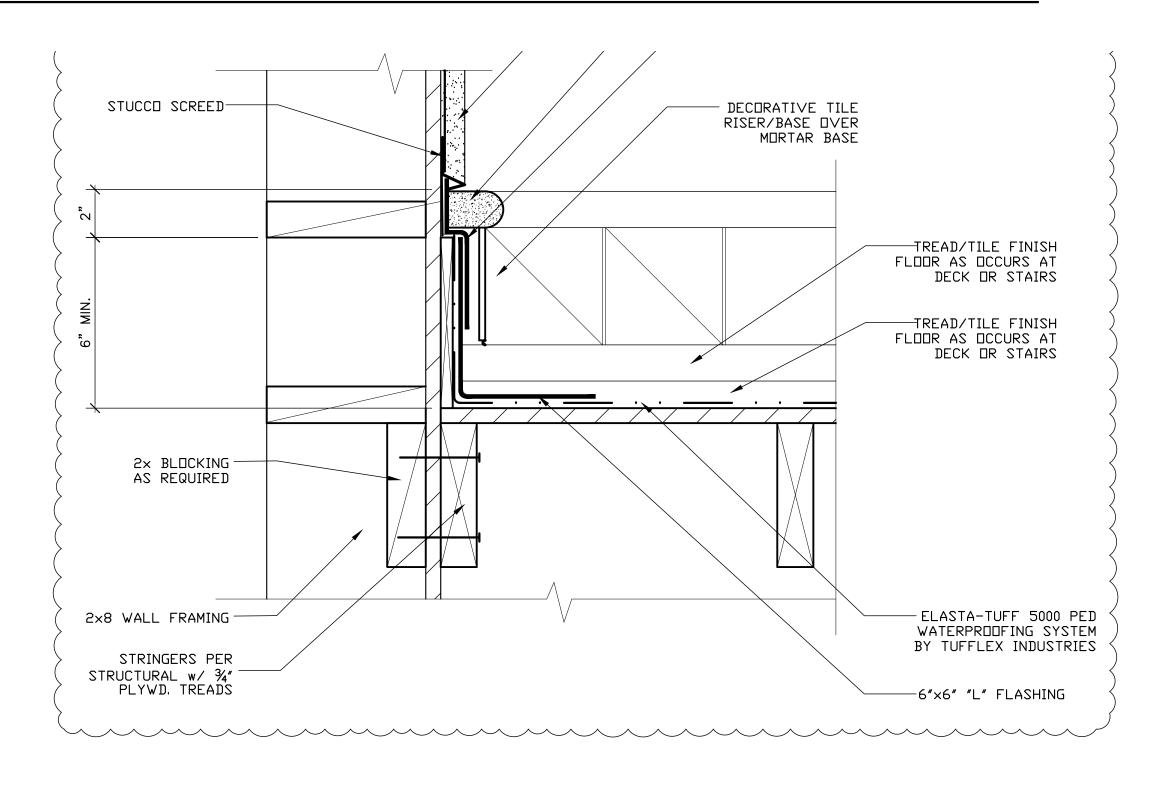


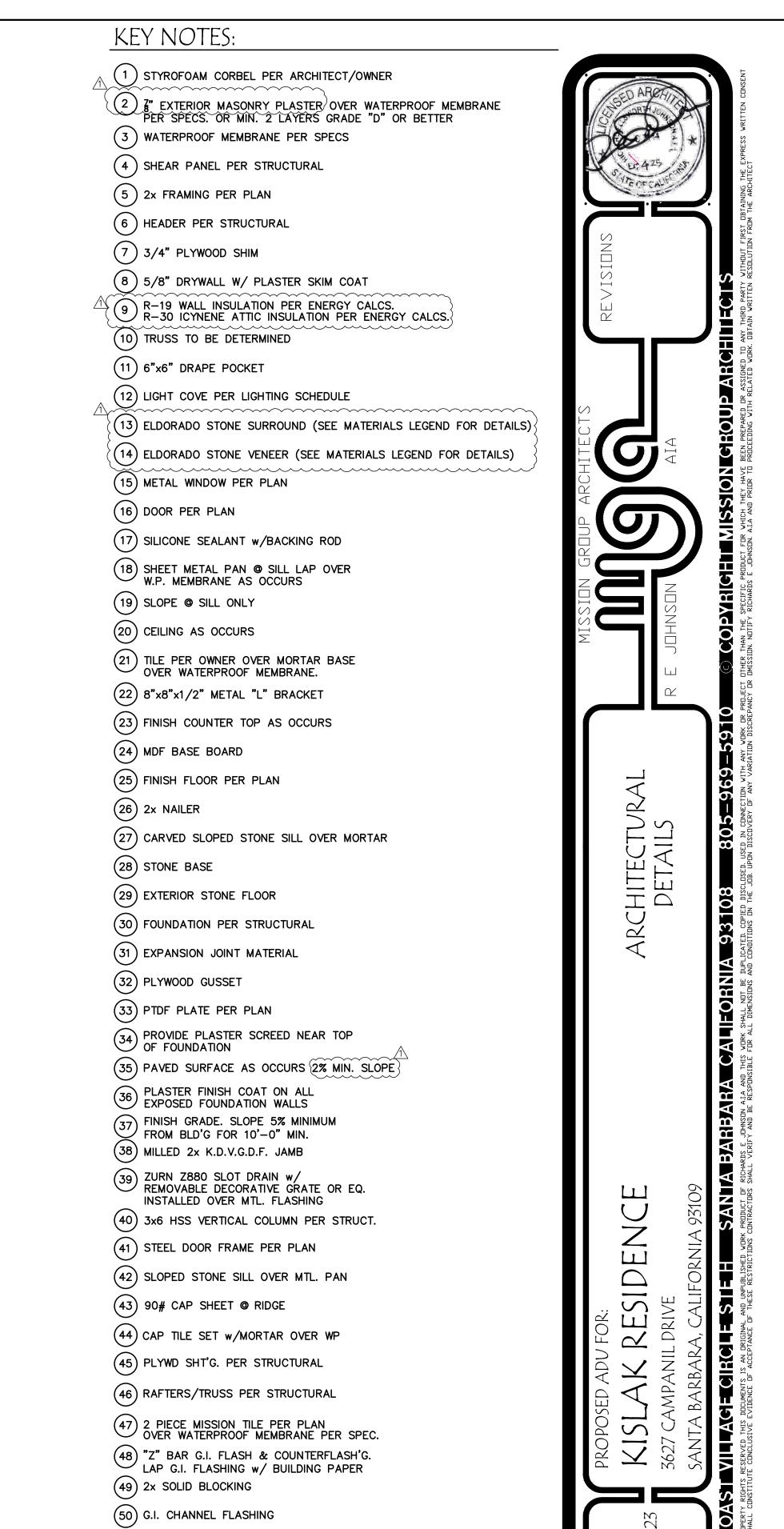






SCALE: 1½" = 1'-0"

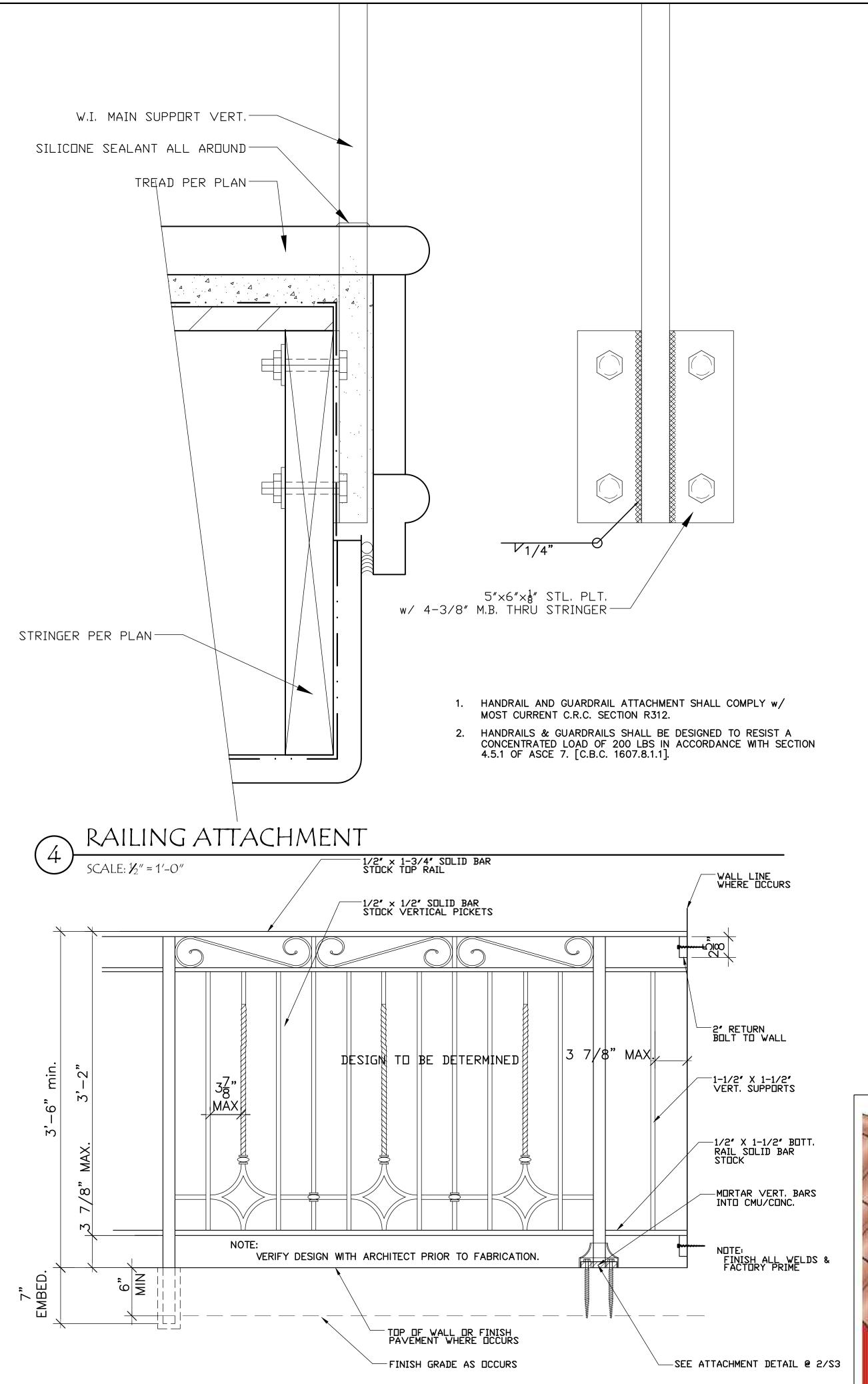




(51) COPPER GUTTER w/ LEAF GUARD

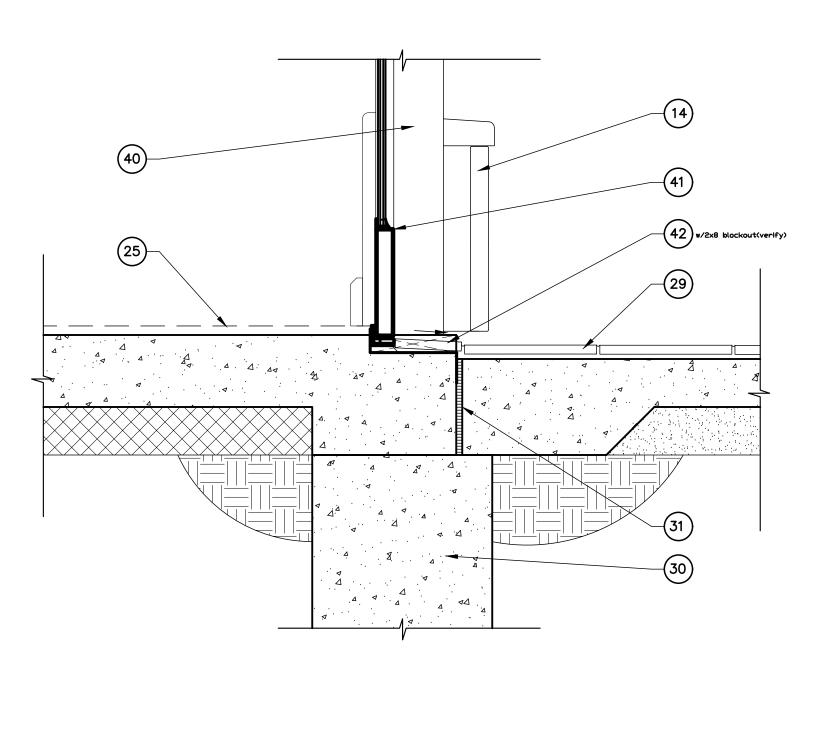
(52) DBL. STARTER TILE W/ BIRD BLOCK MORTAR

ELASTA-TUFF 5000/6000 PED WATERPROOFING SYSTEM BY TUFFLEX AT EXT. STAIRS AND ROOFDECK

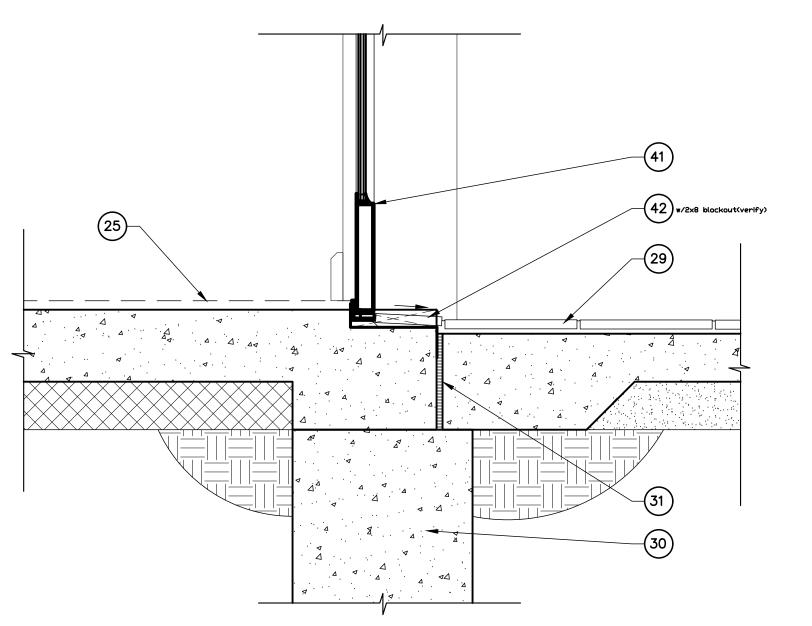


4 HANDDAH AND OHADDDAH ATTAOHAENT CHALL OOMDLY /

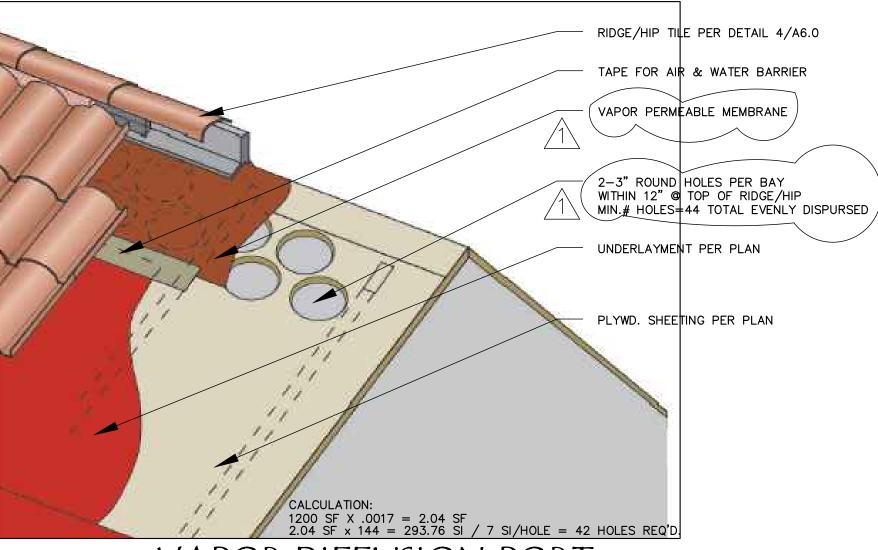
TYPICAL GUARDRAIL DESIGN



TYPICAL EXTERIOR DOOR SILL

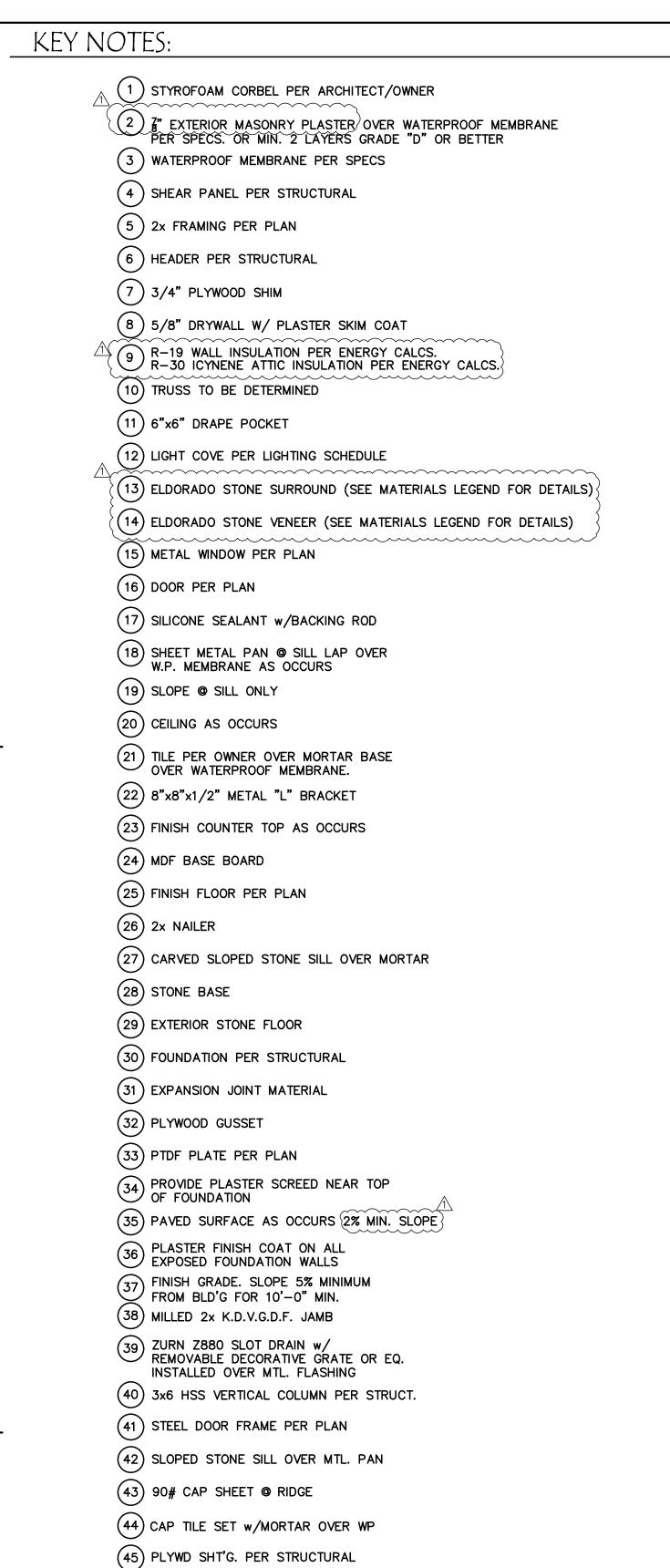


TYPICAL EXTERIOR DOOR SILL



VAPOR DIFFUSION PORT [CRC R806.5]

SCALE: NONE



(46) RAFTERS/TRUSS PER STRUCTURAL

(51) COPPER GUTTER w/ LEAF GUARD

(49) 2x SOLID BLOCKING

(50) G.I. CHANNEL FLASHING

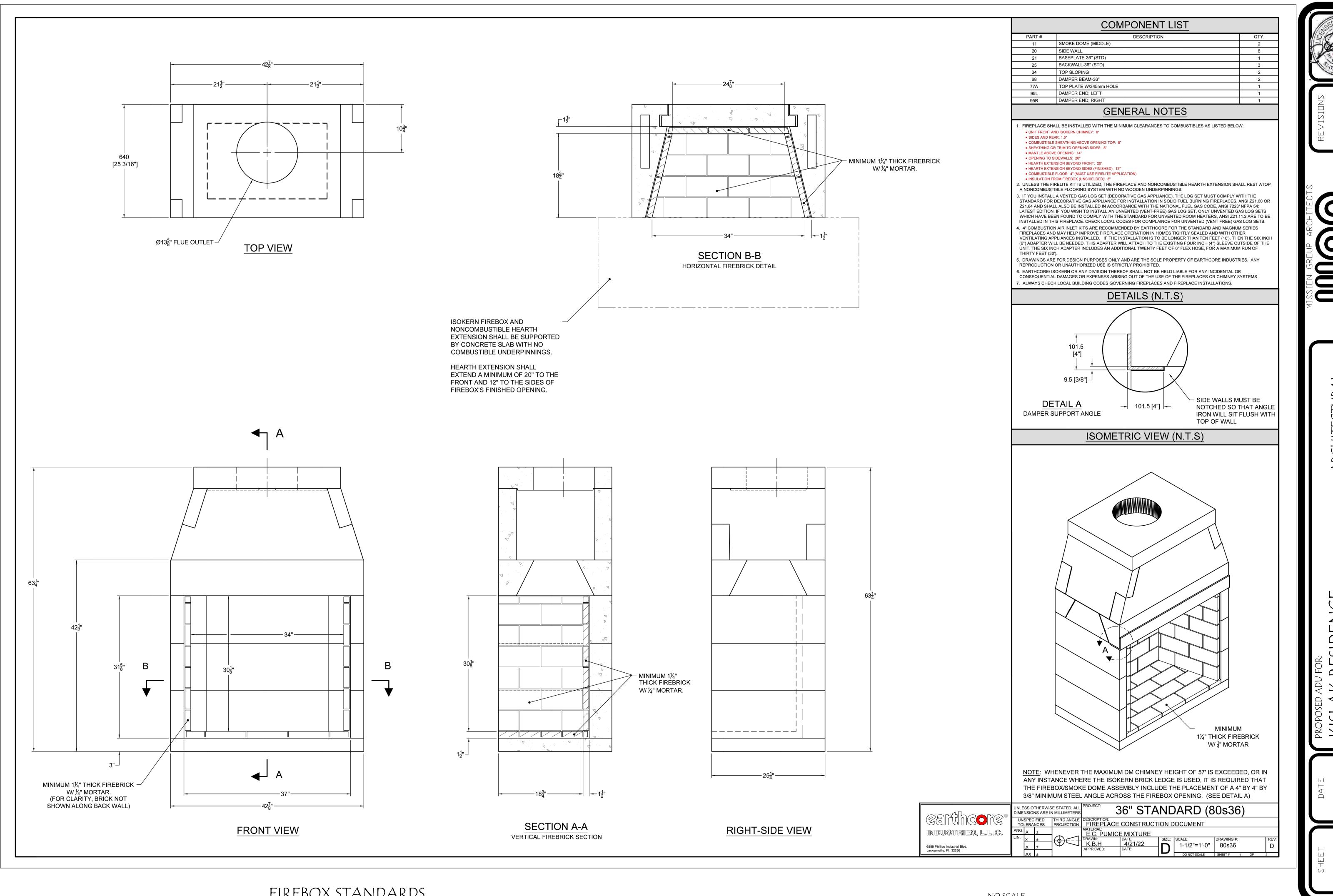
2 PIECE MISSION TILE PER PLAN OVER WATERPROOF MEMBRANE PER SPEC.

"Z" BAR G.I. FLASH & COUNTERFLASH'G. LAP G.I. FLASHING w/ BUILDING PAPER

(52) DBL. STARTER TILE W/ BIRD BLOCK MORTAR

53 ELASTA-TUFF 5000/6000 PED WATERPROOFING SYSTEM BY TUFFLEX AT EXT. STAIRS AND ROOFDECK

SIDENCE

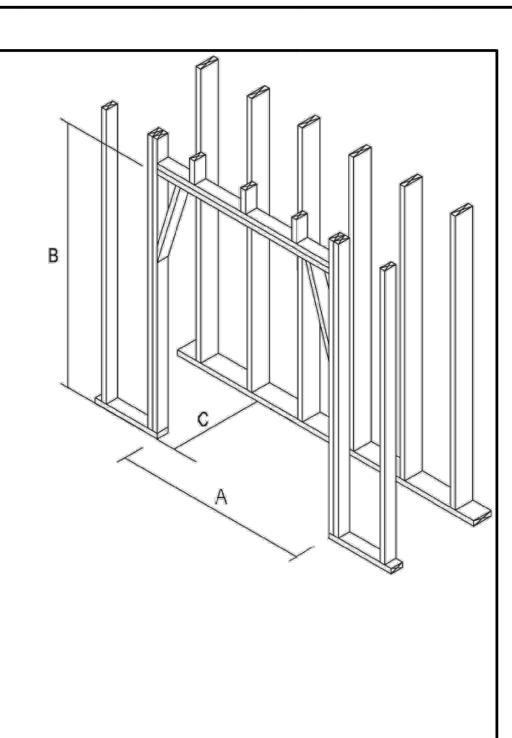


Typical Installation Framing Dimensions

STANDARD	Width - A	Height - B	Depth - C
Model 36	46 "	65"	26 3/4"
Model 42	52 "	65 "	26 3/4"
Model 46	56"	65 "	26 3/4"

Notes:

- 1. "B" includes the 3" thick base plate.
- 2. "Raised hearth" requires additional rough opening height at "B" equal to the height of the raised hearth
- 3. Rough framing dimension for width "A" allows for the required 11/2" clearance at the sides of the Fireplace.
- 4. Rough framing dimension for Height "B" will need to be increased by 4" if the Firelite application is utilized.
- 5. Rough framing dimension for depth "C" allows for the required 11/2" clearance at the back of the Fireplace. 26 3/4" is only for an interior wall as most exterior wall framing have insulation, even if the wall is 2x6, the foam they spray expands so typically 31" is allowed on an exterior wall



Corner Installation Framing Dimensions

The following chart of dimensions detail the positioning of an STANDARD Series fireplace in a corner. It also details the positioning of DM chimney where it must turn 45° degrees, if alignment is needed to overhead framing.

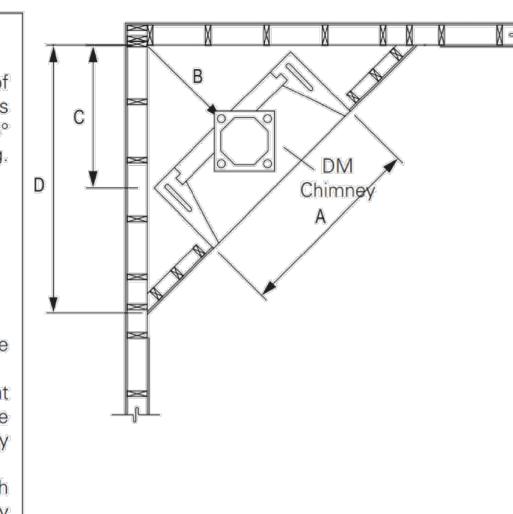
<u>STANDARD</u>	A	В	C	D
Model 36	46"	23 ½"	32"	69"
Model 42	52 "	26 ½"	36"	73"
Model 46	56"	28 1/2"	39"	76 "

To turn flue 45°, first set starting inner liner onto the top plate of the fireplace.

Next, set a DM outer casing onto the inner liner so that the outer casing is at 45° to the firebox and square to the overhead framing system. Run the vertical DM chimney through the overhead framing.

Offset blocks can be used, if necessary, to align with overhead framing before running the vertical DM chimney outer casing and liner.

NOTE: Support the third offset down to footings and at each third offset block thereafter.



Clearance to Combustible Trim - 36, 42 & 46 Models

Hearth Extensions

All STANDARD 36, 42 and 46 Fireplaces shall have hearth extensions of brick, concrete, stone, tile or other code approved noncombustible material. Suitable hearth extension material for the fireplaces shall be placed on the hearth extension's noncombustible substrate and must extend to at least twenty inches (20") in front of the fireplace's finished opening and must extend to at least twelve inches (12") beyond the sides of the finished fireplace opening. (Figure 68)

WARNING: The noncombustible hearth extension, by code, must sit on noncombustible substrate which shall have no wood underpinnings.

This means that off-grade wood floor systems shall be constructed in such a way that all wood floor joists and sub-flooring shall stop twenty inches (20") out from the front of the firebox. (Figure 69)

Mantle and Mantle Shelf Clearances

Fireplaces are subject to the same building code safety clearances to combustible trim as with any radiant

All combustible trim shall be kept at least eight nches (8") from the finished fireplace opening.

Combustible trim located along the sides of the fireplace opening, which project more than one and onehalf inches (1 1/2") from the face of the fireplace, shall have additional clearance from the eight inches (8") equal

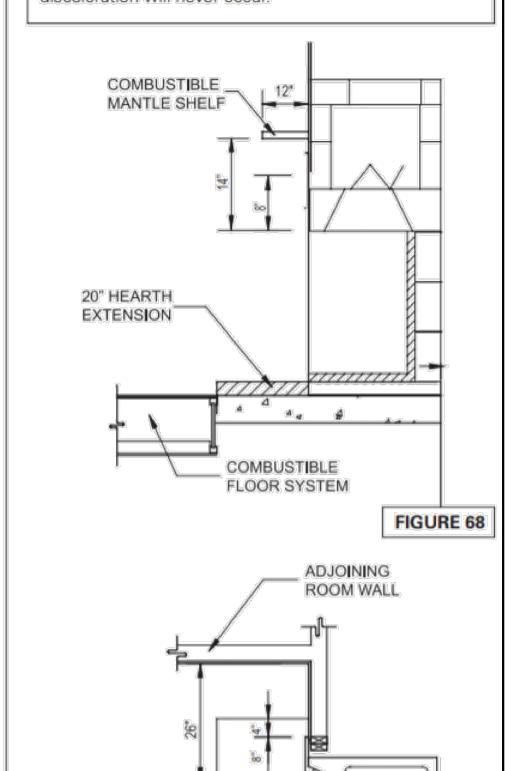
Combustible projecting mantles - up to twelve inches (12") of projection - shall not be placed less than fourteen inches (14") from the top of the fireplace opening. Combustible mantles which project more than twelve inches (12") from the face of the fireplace, shall have additional clearance from the fourteen inches (14") equal to the projection.

Note: The local authority having jurisdiction may require greater clearances for projecting combustible mantle shelves. Be sure to check local building codes regarding required clearances to projecting combustible mantles.

Adjoining Walls. Side walls and walls to rooms adjoining place installations cannot be closer than twentysix inches (26") to the finished fireplace opening.

Note: "Clearance to Combustible Trim" are those distances required to ensure that a fireplace mantle or facing will not catch fire. In most cases the distances should also be adequate to prevent any discoloration or warping due to heat. However each installation presents a unique and completely different set of circumstances involving many variables.

These include paint or finish composition, previous exposure to heat, methods and quality of construction, air flow patterns, etc. Because of these variables, the manufacturer does not guarantee that heat warping or discoloration will never occur.



SCAN QR CODE FOR LATEST

FIGURE 69

EXTENSION

20*

INSTALLATION MANUALS



Standard Series (All Fuel) Specifications

MANUFACTURER:

ISOKERN Fireplace and Chimney Systems, North America distribution by Earthcore Industries, LLC, Jacksonville, Florida (Telephone 800-642-2920)

PRODUCT DESCRIPTION:

Modular refractory masonry precast fireplace and chimney components.

- 1. Designed for field assembly as a fireplace and chimney unit.
- 2. All interlocking parts necessary for assembly of a complete firebox and smoke dome.
- 3. Interlocking double module chimney components (DM).

MATERIALS:

- A. Light weight concrete of a proprietary mixture of Icelandic volcanic aggregate and cement for precast firebox, chimney block and flue liner
 - 1. Compressive Strength: Firebox and Chimney Block: 972 psi.
 - 2. Compressive Strength: Flue Liner: 1175 psi.
- B. Premixed (dry) EARTHCORE ADHESIVE.
 - 1. Tensile strength: 807 psi; Compressive strength: 2460 psi.
 - 2. Tested per ASTM C109, ASTM C307, and ANSI 118.4.
- C. Standard 1-1/8" thick high temperature refractory brick to line firebox
- D. Standard cast iron poker-style damper (optional top mount or in-line damper available).

INSTALLATION:

- A. Reference manufacturer's installation instructions for standard configurations, weights, sizes and installation details.
- B. Suitable masonry foundation and noncombustible hearth extensions must be provided.
- C. Unit to be assembled on site per manufacturer's illustrated instructions.
 - 1. Premixed EARTHCORE ADHESIVE is used at all joints between components.
 - 2. Firebox to be lined with a minimum 1-1/8" thick rated firebrick.
- D. A 1-1/2" minimum clearance to combustible materials is required.
- E. A 3" minimum clearance to insulation required.

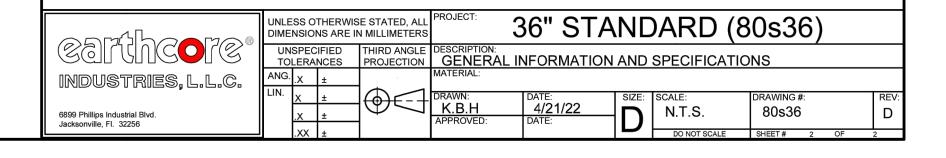
CERTIFICATION:

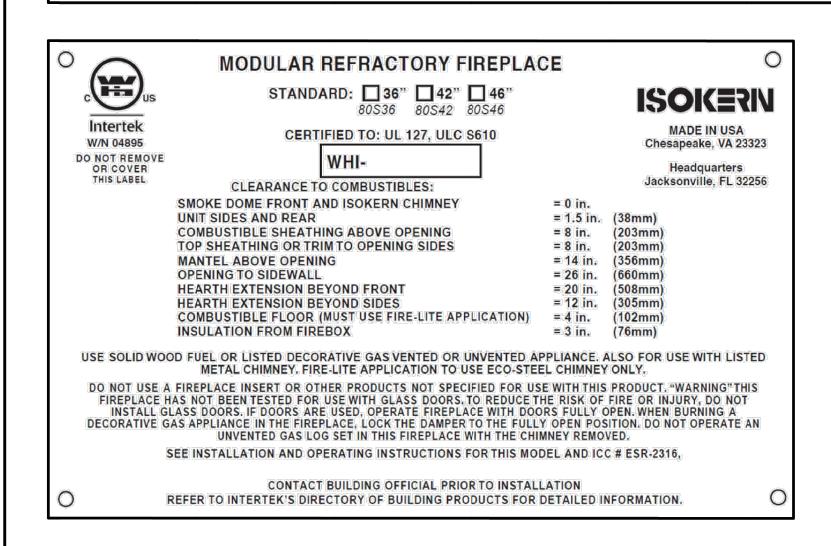
- A. Warnock Hersey/Intertek Testing Services Report No. 632-912500; 3159656MID008; 3082504-T1
- B. Meets or exceeds UL 103HT; UL 103; UL 127; UL 1777; ULC 5610; ANSI 223.1.

CODE COMPLIANCE:

- A. SBCCI NO. 9626
- B. LARR NO. 25483
- C. NYC-MEA 241-90-E
- D. ICC REPORT NO. ESR 2316
- E. IBC 2006, IRC 2006, IMC 2006

NOTE: Isokern components are a natural material and slight variations in dimensions may occur. These should be no more than 1/8".





FIREBOX STANDARDS

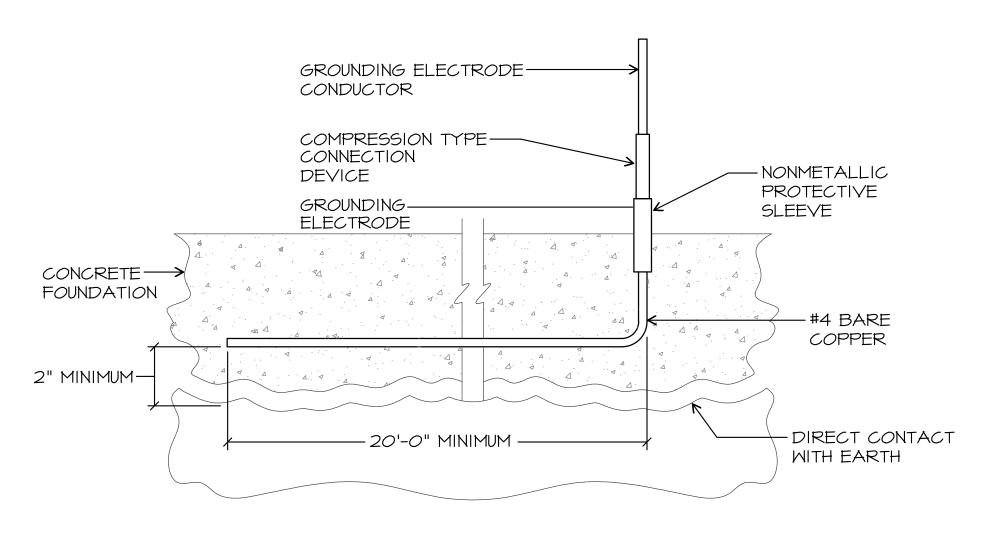
NO SCALE

SIDEN

Item #	DESCRIPTION	QTY.	VA	%VA	TOTAL VA
A.	Lighting & Receptacle, Required Appliance and Laundry				
	Loads				
	1. Lighting & Receptacles: ENTER QTY. IN SQ. FT . =>	4,276	3		12,82
	2. Small Appliance Circuits:	4	1,500		6,00
	3. Laundry Circuits:	2	1,500		3,00
	SUBTOTAL				21,82
	N.E.C. Demand for above loads (per N.E.C. Table 220-11)				A16
	1. First 3,000 VA @ 100%		3,000	100%	3,00
	2. Next 117,000 VA @ 35%		18,828	35%	6,59
	3. Remaining Load @ 25%		0	25%	
	SUBTOTAL FOR ITEM A:				9,59
B.	Fixed appliances				
	Range Hood Fan	2	100	75%	15
	Microwave	2	800	75%	1,20
	Refrigerator	2	850	75%	1,27
	Bath Fan	5	200	75%	75
	Disposal	2	1,000	75%	1,50
	Dishwasher	2	1,000	75%	1,50
	Wine Cooler	1	800	75%	60
				75%	
				75%	
				75%	
				75%	
				100%	
				100%	
	SUBTOTAL FOR ITEM B:				6,97
C.	ADU AC Unit	1	3,000	100%	3,00
D.	AC Units	2	5,000	100%	10,00
E.	Garage Door Opener	1	300	100%	30
F.				100%	
G.				100%	
H.				100%	
I.				100%	
J.				100%	
K.	ABILLIBAWI		1 500	100%	4.50
L.	ADU HP WH	1	4,500	100%	4,50
M.	ADU Stacked W/D	1	5,000	100%	5,00
N.	Main House Gas Dryer			100%	
0.	Main House Gas Water Heater		0.000	100%	9.00
Ρ.	ADU Electric Range	1	8,000	100%	8,00
Q.	Main House Gas Range			100%	20.00
	SUBTOTAL FOR ITEMS C THRU END::	UTLOAD	420/2401/	4 DUACE	30,80
	TOTAL DWELLING UN	III LUAD (120/2407,	IPHASE	47,36

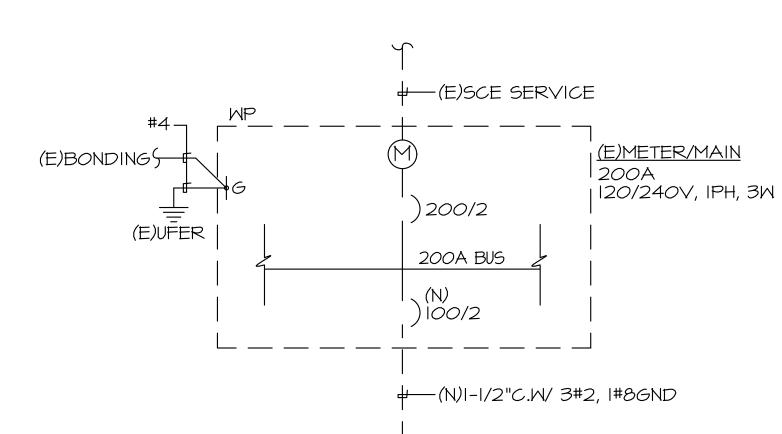
TOTAL DWELLING UNIT LOAD IN AMPS (TOTAL LOAD / 240V)

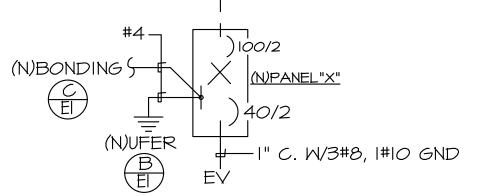
					<u>(N</u>	l) P	AN	EL	SCHI	ED	ULI	E ")	X"					
SERVICE: 120/240V 1Φ	3W		MAI	N B	<u>⟨R.:</u>	100	A-2P)				BUS	S: 10	AOC				LOC.: SEE PLAN
																		MTG.: FLUSH
			R	∟	М	Р	Т	O		O	Т	J	R	L	М			
REMARKS	LO	AD	E	T		0	R				R	0 -	E	T		LO	AD	REMARKS
	ФА	ΦВ	C	G	s C	L E		R C		R C	P	L E	С	G	S C	ФА	ΦВ	
SMALL APPLIANCE						1	20	1		2	30	2	1					WASHER/DRYER
11 11						1	20	3		4	Х	Χ						II .
LAUNDRY						1	20	5		6	30	2			1			AC UNIT
BATHROOM						1	20	7		8	Х	Χ						"
SMOKE DETECTORS						1	20	တ		10	20	1						SPARE
DISPOSAL						1	20	11		12	20	1						II .
DISHWASHER						1	20	13		14	20	1						II .
EXTERIOR REC						1	20	15		16	20	1						п
EXTERIOR LTS						1	20	17		18	40	2						EV CHARGER
LANDSCAPE						1	20	19		20								II
AMPS= SEE ELECTRIC	AL LOA	D CALC	ULĀ	TION	IS						MIN	MUÌ	M B	(R		A.I.C. R	ATING=	= 10,000 AMPS SYM



THE GROUNDING SYSTEM SHALL CONSIST OF A "UFER" TYPE 20' LONG OF #3/O BARE COPPER CONDUCTOR EMBEDDED ALONG THE BOTTOM OF A CONCRETE FOOTING OR GRADE BEAM THAT IS IN DIRECT CONTACT WITH THE EARTH.

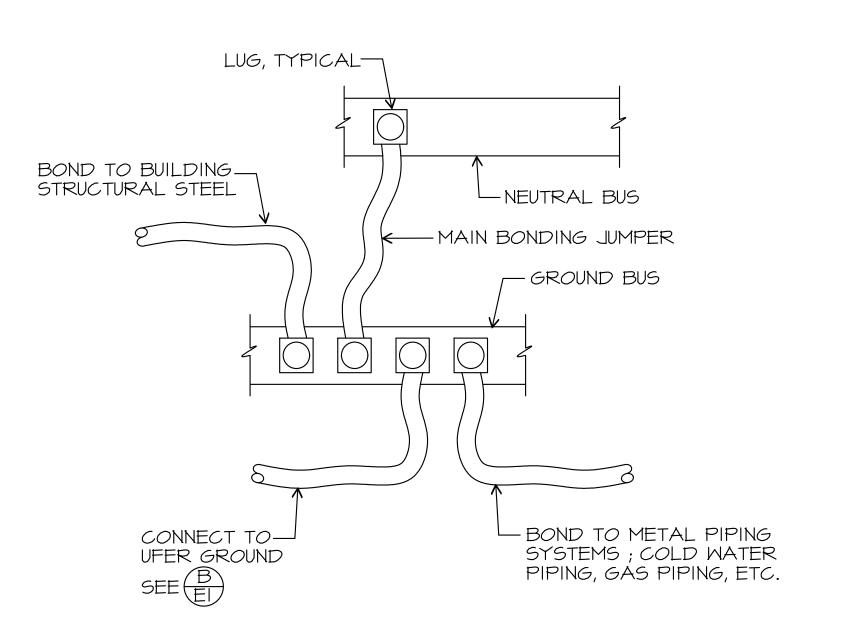






SINGLE LINE DIAGRAM

SCALE: NONE



BONDING DIAGRAM SCALE: NONE

GENERAL NOTES

- 1. VISIT JOB SITE AND VERIFY EXISTING CONDITIONS PRIOR TO BID.
- 2. THE ELECTRICAL WORK WILL BE INSTALLED IN ACCORDANCE WITH THE 2019 CALIFORNIA ELECTRICAL CODE AND ALL APPLICABLE LOCAL ORDINANCES. WHERE PLANS CALL FOR A HIGHER STANDARD THAN APPLICABLE CODES, THE PLANS SHALL
- 3. CONDUIT RUNS ARE SHOWN DIAGRAMMATICALLY. EXACT LOCATIONS SHALL BE DETERMINED IN THE FIELD TO SUIT FIELD CONDITIONS.
- 4. ALL ELECTRICAL EQUIPMENT, APPLIANCES AND LIGHTING FIXTURES SHALL BE LISTED BY A RECOGNIZED TEST LAB AND BEAR THAT LABEL OF APPROVAL.
- 5. CONTRACTOR SHALL FURNISH, INSTALL AND CONNECT ALL MATERIAL AND EQUIPMENT FOR THIS WORK UNLESS OTHERWISE NOTED.
- 6. FURNISH DISCONNECT SWITCHES AT REMOTE MOTORS.
- 7. ALL SPACES AS INDICATED ON PANELS OR SWITCHBOARDS SHALL BE COMPLETE WITH HARDWARE AND BUSSING FOR FUTURE BREAKER OR SWITCH.
- 8. CHECK ARCHITECTURAL PLANS FOR DOOR SWINGS BEFORE INSTALLING SWITCH
- 9. GROUNDING AND BONDING SHALL BE PER CODE PLUS ANY ADDITIONAL PROVISIONS SPECIFIED OR SHOWN ON DRAWINGS.
- 10. ALL CONDUIT RUNS SHALL CONTAIN A CODE SIZED GREEN GROUND WIRE.
- II. THESE PLANS ARE NOT COMPLETE UNTIL APPROVED BY THE AUTHORITY HAVING JURISDICTI*O*N.
- 12. ALL FEEDER CONDUCTORS SHALL BE IN CONDUIT. BRANCH CIRCUITS MAY BE NON-METALLIC SHEATHED CABLE.
- 3. ALL CONDUCTORS SHALL BE COPPER WITH TYPE THHN/THWN INSULATION.
- 14. COORDINATE WITH SERVING ELECTRICAL UTILITY COMPANY AND MAKE PROVISIONS FOR ELECTRICAL SERVICE ACCORDINGLY. INCLUDE ALL SERVICE COSTS AND UTILITY COMPANY CHARGES IN BID.
- 15. COORDINATE WITH SERVING TELEPHONE UTILITY COMPANY AND MAKE PROVISIONS FOR TELEPHONE SERVICE ACCORDINGLY. INCLUDE ALL SERVICE COSTS AND ANY UTILITY COMPANY CHARGES IN BID.
- 16. COORDINATE WITH SERVING CABLE TELEVISION COMPANY AND MAKE PROVISIONS FOR TELEPHONE SERVICE ACCORDINGLY. INCLUDE ALL SERVICE COSTS AND UTILITY
- 17. ALL PERMITS SHALL BE OBTAINED AND PAID FOR BY THE CONTRACTOR.

COMPANY CHARGES IN BID.

- 18. ALL 120-VOLT, SINGLE PHASE, 15 AND 20 AMPERE BRANCH CIRCUITS SUPPLYING OUTLETS OR DEVICES INSTALLED IN DWELLING UNIT KITCHEN, FAMILY ROOM, DINING ROOMS, LIVING ROOMS, PARLORS, LIBRARIES, DENS, BEDROOMS, SUNROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, LAUNDRY AREAS, OR SIMILAR ROOMS OR AREAS SHALL BE PROTECTED BY A LISTED ARC-FAULT/BRANCH CIRCUIT INTERRUPTER, COMBINATION TYPE, A BRANCH/FEEDER TYPE, A LISTED SUPPLEMENTAL ARC PROTECTION CIRCUIT BREAKER INSTALLED TO PROVIDE PROTECTION OF THE BRANCH CIRCUIT. [CEC 210.12(A)(1) THROUGH (6)].
- 19. ALL NON-LOCKING TYPE 125-VOLT. 15 AND 20 AMPERE RECEPTACLES IN A DWELLING UNIT SHALL BE LISTED TAMPER-RESISTANT RECEPTACLES. (EXCEPTIONS: (1) RECEPTACLE MORE THAN 5'-6" ABOVE THE FLOOR, (2) RECEPTACLES PART OF A LUMINAIRE OR APPLIANCE, (3) A SINGLE RECEPTACLE OR A DUPLEX RECEPTACLE FOR TWO APPLIANCES THAT ARE NOT EASILY MOVED AND LOCATED WITHIN DEDICATED SPACE AND ARE CORD-AND-PLUG CONNECTED AS PER CEC 400.10(A)(6), (A)(7) OR (A)(8), AND (4) NON-GROUNDING RECEPTACLES USED FOR REPLACEMENTS AS PERMITTED IN CEC 406.4 (D)(2) (A), [CEC 406.12].
- 20. SMOKE DETECTORS SHALL BE 120V, PHOTOELECTRIC/ION COMBINATION UNITS WITH BATTERY BACK-UP. THEY SHALL BE INTERCONNECTED.
- 21. HALLWAY DETECTORS SHALL BE COMBINATION SMOKE AND CARBON MONOXIDE DETECTOR. THEY SHALL BE INTERCONNECTED WITH SMOKE DETECTORS.
- 22. LIGHT FIXTURES IN BATHTUB OR SHOWER AREAS SHALL BE MARKED AS "SUITABLE FOR DAMP LOCATIONS" PER CEC 410.10.
- 23. WP EXTERIOR RECEPTACLES SHALL HAVE HUBBELL #ML500 EXTRA DUTY COVERS OR EQUAL. ALL RECEPTACLES IN DAMP OR WET LOCATIONS (WP) SHALL BE A LISTED WEATHER-RESISTANT TYPE AND BE GFCI. [CEC 406.9].
- 24. MANUFACTURER'S LITERATURE SHOWING THAT PROPOSED LED LIGHT FIXTURES ARE HIGH EFFICACY AND CALIFORNIA CERTIFIED IS TO BE ON SITE AT THE TIME OF FIELD INSPECTION. CALIFORNIA ENERGY CODE 150.
- 25. AT LEAST ONE FIXTURE INSTALLED IN GARAGES, CLOSETS, LAUNDRY ROOMS, AND UTILITY ROOMS SHALL BE CONTROLLED BY A VACANCY SENSOR PER CALIFORNIA ENERGY CODE 150(K)2.
- 26. AT LEAST ONE LIGHT FIXTURE IN EACH BATHROOM SHALL BE CONTROLLED BY A VACANCY SENSOR PER CALIFORNIA CODE 150(K)2.
- 27. TWO MINIMUM I" DIAMETER METALLIC CONDUITS SHALL BE PROVIDED THAT ORIGINATE AT A READILY ACCESSIBLE ATTIC LOCATION WITH PROXIMITY TO A SOLAR ZONE AREA COMPLYING WITH CALIFORNIA ENERGY CODE, SECTION 110.10 AND TERMINATE AT A MINIMUM 4" FROM THE ELECTRICAL PANEL. THE ELECTRICAL JUNCTION BOX AND SEGMENT OF CONDUIT RUN IN THE ATTIC SHALL BE PERMANENTLY AND VISIBLY MARKED AS "FOR FUTURE SOLAR PHOTOVOLTAIC"
- 28. RECESSED LUMINARIES INSTALLED IN INSULATED CEILINGS SHALL HAVE AN I.C. RATING AND SHALL BE CERTIFIED AS AIR TIGHT.
- 29. LUMINARIES WITH SCREW BASE SOCKETS SHALL NOT BE RECESSED IN A CEILING.
- 30. LUMINARIES WITH SCREW BASE SOCKETS SHALL BE MARKED AS JA8-2019-E
- COMPLIANT AND SHALL ONLY CONTAIN JAS COMPLIANT LAMPS.
- 31. ALL JAS COMPLIANCE LUMINARIES SHALL BE CONTROLLED BY DIMMERS OR
- VACANCY SENSORS.
- 32. ALL KITCHEN COUNTERTOP RECEPTACLES ARE TO BE GFCI PROTECTED. RECEPTACLES WITHIN 6 FEET FROM THE TOP INSIDE EDGE OF THE BOWL OF THE SINK, RECEPTACLES WITHIN 6 FEET OF THE OUTSIDE EDGE OF ANY BATHTUB OR SHOWER STALL, AND RECEPTACLES IN LAUNDRY AREAS ARE TO BE GFCI PROTECTED. [CEC 210:8].

SYMBOLS

— — CONDUIT EXISTING - - CONDUIT CONCEALED UNDER FLOOR OR BELOW GRADE

------ CONDUIT STUBBED OUT AND CAPPED

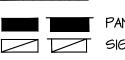
O- CONDUIT TURNED UP ----- CONDUIT TURNED DOWN

HATCH MARKS INDICATE NO. OF #12 WIRES IN CODE SIZED CONDUIT (3) MAX. IN 1/2" C., (5) MAX. IN 3/4" C., (8) MAX. IN 1"C., NO MARKS = 2 # 12HOME RUN: LETTER INDICATES PANEL, NUMBER(S) INDICATES

CIRCUIT(S). ======== SAWGUT

GROUND CONNECTION

DISTRIBUTION SWITCHBOARD OR PANEL



PANEL, BRANCH CIRCUIT TYPE, SURFACE AND FLUSH

SIGNAL TERMINAL CABINET, SURFACE & FLUSH

__________FLUORESCENT FIXTURE

- OUTLET DATA: BAR INDICATES WALL MOUNT, LETTER INDICATES SWITCH CONTROL, NO. INDICATES CIRCUIT.
- SURFACE FIXTURE ON FLUSH OUTLET.
- RECESSED FIXTURE WITH JUNCTION BOX FOR THRU WIRING EXIT LIGHT WITH ARROWS AS SHOWN ON PLANS, WALL AND
- CEILING MOUNT.
- \bowtie LOW LEVEL EXIT SIGN, +6" AFF, +4" FROM DOOR JAMB LIGHT FIXTURE DESIGNATION, LETTER INDICATES TYPE. NO. INDICATES WATTAGE. SEE FIXTURE SCHEDULE.
- MECHANICAL EQUIPMENT DESIGNATION. SEE MECHANICAL DRAWINGS.
- SPECIAL RECEPTACLE SEE PLAN
- METER
- \odot FLUSH FLOOR RECEPTACLE RECEPTACLE, DUPLEX, 15A, 125V, NEMA 5-15R +18" U.N.O.
- DUPLEX RECEPTACLE MTD. ABOVE BACKSPLASH
- DUPLEX RECEPTACLE W/LOWER HALF SWITCHED
- GROUND FAULT CIRCUIT INTERRUPTING RECEPTACLE
- DOUBLE DUPLEX RECEPTACLE
- CEILING RECEPTACLE
- RECEPTACLE, DUPLEX, 20A, 125V, NEMA 5-20R +18" U.N.O.
- JUNCTION BOX 4" SQUARE, 1-1/2" DEEP U.N.O.
- \bigcirc THERMOSTAT F.B.O. +48"
- MOTOR, NO. INDICATES HORSEPOWER
- CLOCK OUTLET +7'-6" U.N.O.
- DISCONNECT SWITCH, NON-FUSED DISCONNECT SWITCH FUSED HORSEPOWER RATED OR SIZED AS
- COMBINATION MAGNETIC STARTER WITH DISCONNECT SWITCH AND
- MAGNETIC MOTOR STARTER W/OVERLOADS IN EACH PHASE DIMMER W/INTEGRAL "ON-OFF" SW.
- PUSHBUTTON
- SPEAKER PHOTOCELL
- SMOKE DETECTOR
- TELEPHONE/COMPUTER/DATA OUTLET, TWO GANG BOX W/I GANG
- COVERPLATE & GROMMETED OPENING +18" U.N.O.

QUIET TOGGLE TYPE RATED AT

20A |20/2TTV A.C. +42" U.N.O.

- CABLE TV OUTLET +18" U.N.O.
- MOTION SENSOR
- EXISTING SWITCH
- SINGLE POLE SWITCH
- DOUBLE POLE SWITCH
- THREE WAY SWITCH
- SWITCH W/PILOT LT.
- MANUAL MOTOR STARTER
- FACP FIRE ALARM CONTROL PANEL
- GROUND FAULT CIRCUIT INTERRUPTING
- LABOR SAVING TANDEM
- MAIN LUGS ONLY
- M/
- CONDUIT ONLY
- WEATHERPROOF
- FURNISHED BY OTHERS, INSTALL & CONNECT
- U.N.O. UNLESS NOTED OTHERWISE
- NATIONAL ELECTRICAL CODE
- NOT IN CONTRACT
- (E) EXISTING NEW
- (R) REM*O*VE
- REL*OCA*TE
- SURFACE MOUNT
- UNDERGR*O*UND
- COLD WATER PIPE ABOVE FINISHED FLOOR
- HEATING AND AIR CONDITIONING RATED CIRCUIT BREAKER
- NOTE: NOT ALL SYMBOLS SHOWN ARE USED ON THIS PROJECT.

LIGHT NG DESIGN CA REGIST ATION NO E13083 22311 627 OLIVE STREET SANTA BARBARA CA 93105 (805) 569-9216 FAX (805) 569-2405 email : maloney@jmpe.net www.jmpe.net $\mathbb{A}_{\mathcal{L}}$ 屈取り

REVISIONS BY

GRAM \Box 9 ENERAL YMBOLS INGLE 1

FROP W62 SAN

うらら 05-08-2023 NONE ARRED SHEET

Registration Number:

CA Building Energy Efficiency Standards - 2019 Residential Compliance

Registration Date/Time:

HERS Provider:

January 2020

CA Building Energy Efficiency Standards - 2019 Residential Compliance

ELECTR AL ENGINEERING
LIGHTING DESIGN
CA REGISTRATION NO E13083

22311

627 OLIVE STREET
SANTA BARBARA CA 93105
(805) 569-9216
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PROFESS/ON
MALON
ED13081
EXP. D6/30/2021

REVISIONS BY

PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CA 4310

SHTING COMPLIANCE FORMS

DATE 05-08-2023
SCALE NONE

January 2020

ARRED

IEET

1

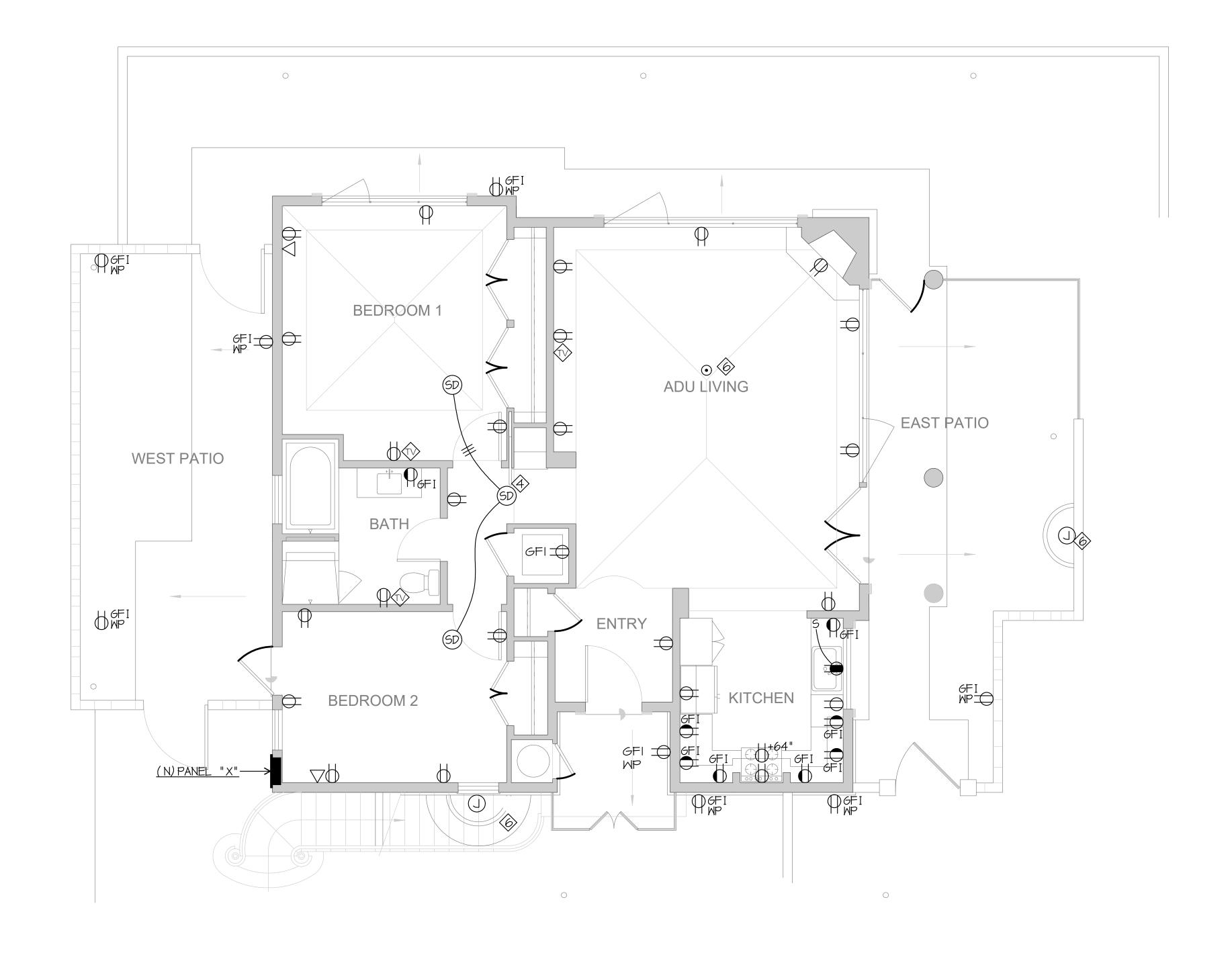
NORTH

DATE 05-08-2023

SCALE 1/4"=1'-0"

ELECTRICAL NOTES

- I. ALL OUTLETS IN DWELLING UNITS SHALL BE PROTECTED BY A LISTED ARC FAULT CIRCUIT INTERRUPTER, COMBINATION TYPE PER CEC 210.12
- 2. TAMPER RESISTANCE RECEPTACLE SHALL BE INSTALLED IN DWELLING UNITS PER CEC 406.II
- 3. SMOKE DETECTORS SHALL BE 120V, INTERCONNECTED, PHOTOELECTRIC/ION UNITS WITH BATTERY BACK UP
- 4 HALLWAY DETECTORS SHALL BE COMBINATION SMOKE AND CARBON MONOXIDE DETECTOR. PART #KIDDE KN-COSM-BA
- S VERIFY LOCATION OF FLUSH FLOOR RECEPTACLE PRIOR TO ROUGH IN
- 6 CONNECT FOUNTAIN VIA GFCI



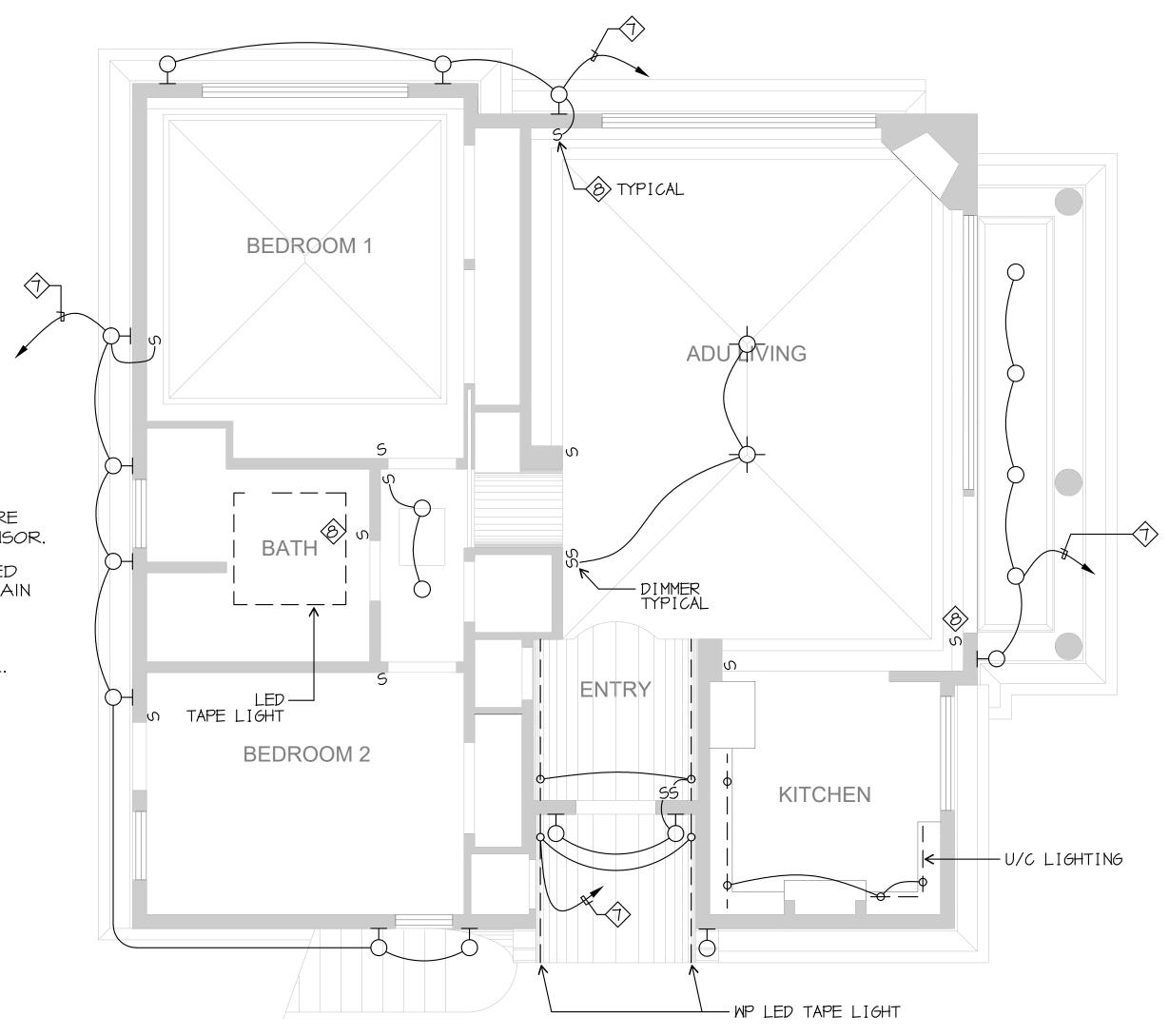
POWER PLAN

SCALE: 1/4" = 1'-0"

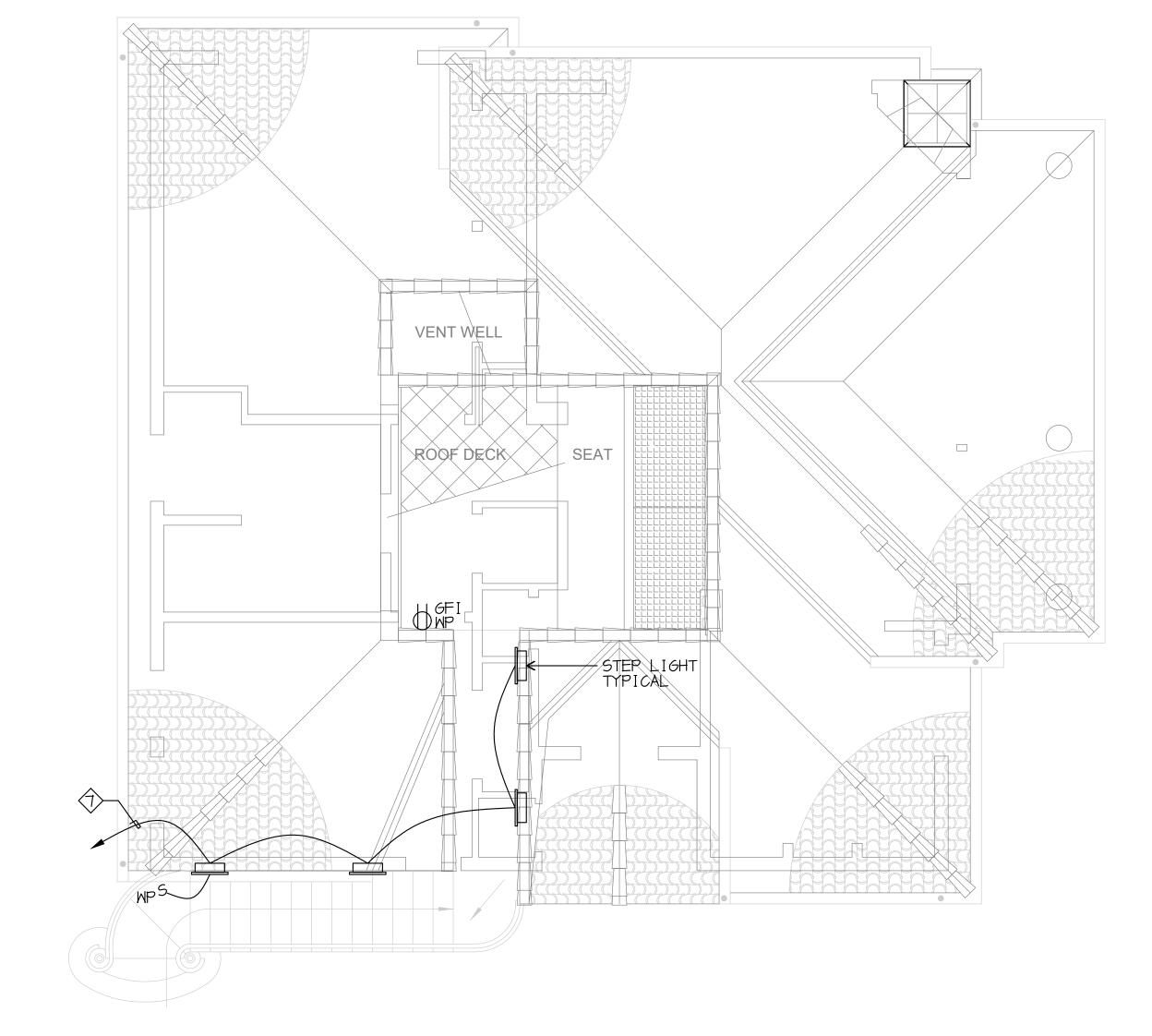
- I. ALL INSTALLED LIGHTING
 TO BE HIGH EFFICACY PER
 REQUIREMENTS OF 2019 CEC SECTION
 150.0(k) AND JOINT APPENDIX JA8.
- 2. BUILDER SHALL PROVIDE THE HOMEOWNER WITH A LUMINAIRE SCHEDULE THAT INCLUDES A LIST OF ALL INSTALLED LAMPS AND LUMINAIRES.
- 3. ANY JA8 COMPLIANT LAMP MUST BE CONTROLLED BY A VACANCY SENSOR OR DIMMER BASED ON TYPE OF LUMINAIRE OR LAMP INSTALLED.
- 4. RECESSED FIXTURES SHALL BE IC RATED AND ASTM E283 CERTIFIED.

 RECESSED FIXTURES SHALL NOT CONTAIN A SCREW BASE SOCKET AND MUST BE INSTALLED WITH A LIGHT SOURCE THAT IS JA8 CERTIFIED, SHALL NOT CONTAIN LIGHT SOURCES THAT ARE LABELED "NOT FOR USE IN ENCLOSED FIXTURES" OR "NOT FOR USE
- 5. IN BATHROOMS, LAUNDRY ROOMS, GARAGES, AND UTILITY ROOMS, AT LEAST ONE LUMINAIRE SHALL BE CONTROLLED BY A VACANCY SENSOR.
- 6. THE NUMBER OF ELECTRICAL BOXES LOCATED ABOVE FINISHED FLOOR THAT DO NOT CONTAIN A LUMINAIRE OR OTHER DEVICE SHALL NOT EXCEED THE NUMBER OF BEDROOMS. THESE BOXES MUST BE SERVED BY A DIMMER, VACANCY SENSOR, OR FAN SPEED CONTROL.
- TO HOMERUN VIA LIGHTING CONTROL SYSTEM
- & LIGHTING CONTROL KEY PAD

IN RECESSED FIXTURES."







ELECTRICAL ROOF PLAN

SCALE: 1/4" = 1'-0"

NORTH

LIGHTING PLAN ELECTRICAL ROOF PLAN

DATE 05-08-2023

SCALE 1/4"=1'-0"

DRAWN ARRED

CA REGISTRATION NO E13083

22311

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PROPOS KISL/ 3627 SANT/

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6. Structural Wood

- A. General: All structural wood work shall be done in accordance with the National Design Specification for Wood Construction (NDS-2018 \$ SDPWS 2018) and the CBC 2019 edition, chapter 23.
- B. Structural members shall not be cut or drilled unless specifically noted or detailed.
- 1. Foundation sill, nailers, and ledgers embedded in or in direct contact with concrete or masonry shall be pressure treated D.F. 2. 6x Beam: No. 1 D.F.

C. All lumber materials, unless otherwise noted or shown, shall be as follows:

- 3. Framina Members:
- a. Joists, headers, plates, subpurlins, nailers, and blocking shall be D.F. #2 or better unless noted otherwise
- b. 2x4 studs shall be D.F. construction grade or better. c. All 2x joists and rafters are to be solid blocked at all points of bearing. Solid blocking is required at 8'-0"oc unless continuously braced on bottom edge. Blocking may be omitted for ceiling joists and roof rafters eight inches and
- 4. Glue-Laminated Beams:
- a. Glulam beams shall conform to industrial grade 24F-V8 unless noted otherwise. Use architectural grade where noted on architectural plans. b. All glulam beams shall be fabricated by a licensed fabricator and shall have
- AITC certifications. c. Adhesives and laminations shall conform to AITC Standard A190.1.

5. Prefabricated Joist Notes:

- a. TJI joists shall be as manufactured by Trus-Joist (by Weyerhaeuser) and shall have solid plywood or Oriented Strand Board webs as recommended by the manufacturer for the loads and spans indicated.
- b. Microlam beams shall be as manufactured by Trus-Joist (by Weyerhaeuser). Multiple pieces shall be joined as follows: 2 pieces (3) rows 16d nails @ 12"oc
- 3 or more pieces (2) rows 1/4 M.B. @ 12"oc

less in depth, unless noted otherwise on plans.

- c. The joists indicated on the plans are based on the manufacturer's load tables and must be verified by the manufacturer. Submit shop drawings and
- calculations for each different joist type, span, and loading condition. d. Install all joists and beams per the manufacturer's recommendations including
- all necessary stiffeners, bridging, blocking, and hangers. e. Flanges shall be designed to accommodate closely spaced plywood roof or floor diaphragm nailing where occurs on plans.
- f. Design loads shall be as indicated on plans. a. Parallam beams shall be as manufactured by Trus-Joist (by Weyerhaeuser) and shall have an E minimum = 2000 ksi.

6. Plywood Sheathing:

- a. Roof Sheathing shall be USDOC PSI or PS2 rated plywood or OSB as indicated on the plans. Plywood shall be fabricated using exterior glue.
- b. Floor Sheathing shall be USDOC PSI or PS2 rated plywood or OSB as indicated on the plans. Glue all contact surfaces. Nail with ring shank or screw shank nails. c. Wall Sheathing shall be USDOC PSI or PS2 rated plywood or OSB as scheduled
- on the plans. Block all panel edges. d. Plywood machine nailing will be subject to satisfactory job site demonstration and approval by the structural engineer. The approval is subject to continued satisfactory performance. If nail heads penetrate the outer ply more than would be normal for a hand hammer or if minimum allowable edge distances
- e. At all unblocked plywood roof sheathing edges, provide one ply clip between each joist unless T & G plywood is used.
- f. Plywood sheet minimum dimensions shall be 24". Minimum area shall be 8 sq. ft.

are not maintained, the performance will be deemed unsatisfactory.

- a. Face nail 2x6 T \$ G decking with (2) 16d to each support.
- 7. Exposed Beams and Joists: a. All exposed beams and joists shall be Select Structural D.F., Free of Heart Center and Void of Defects.

D. Bolted Joints

Bolts shall be ASTM A-307, unless noted otherwise on plans.

- 2. Bolt Holes are to be 1/32" to 1/16" larger than bolt diameter. Locate
- 3. Washers are to be provided at each bolt head and nut. Place washer next to wood 4. Lag or Wood Screws shall be screwed and not driven into place.
- 5. Tighten bolts up snug and re-tighten at the latest practical time during construction.

E. Nailed Joints

- . Size and spacing shall be as shown on the drawings and nailing schedule, Sheet S1.3. 2. Sub-bore when nails tend to split wood. Sub-bore for 20d and larger
- nails. Drill diameter shall be 0.75 x nail diameter.
- 3. Nails shall be common or ICC approved plywood nails unless noted otherwise. Common nails shall have the following minimum shank diameters: 8d nail - .131" diameter
- 10d nail .148" diameter 16d nail - .162" diameter

F. Joist Hangers and Framing Connectors

1. By Simpson Strong Tie Co. ICC-ES # 209, #393, #413, #1211

G. Fasteners

- Expansion Anchors to be Hilti Kwik-Bolt TZ ICC-ES-ESR #1917.
- 2. Epoxy shall be two part epoxy. Use Simpson SET-XP Epoxy ICC-ESR #2508. 3. Powder driven "shot pins" to be Hilti NK72 or approved equal. For use on interior, non-bearing sill plates on slab only.

K. Dry Pack: 1 Part Portland cement to 2 ½ parts sand; or use non-shrink metallic grout (submit proposal to structural engineer for review).

- L. Cover to Bars: Cover to reinforcing bars shall be as follows, unless otherwise shown or noted: Cover to reinforcing bars shall be increased by 50% (or epoxy coated reinforcing used) in corrosive environments or other severe exposure conditions. 1. When concrete is placed against ground
- 2. When concrete is placed against forms, but after form removal will be in contact with ground 3. Inside face of walls not exposed to the elements. 4. All others
- M. Curing: Keep concrete slab on grade wet for 7 days, or cover with approved curing compound in strict accordance with manufacturer's installation recommendations.
- N. Vibrate all concrete in place with a mechanical vibrator used by experienced personnel.
- O. Slab Finishes: Interior slab finish shall be troweled smooth. Exterior slab finish shall be with a light broom perpendicular to travel. Unless otherwise noted.
- P. Slab tolerance to be planar to within $\frac{1}{6}$ in 10 feet when checked with a 10' long rod in any direction.
- Q. Sill Plate Anchor Bolts: Provide minimum $\frac{5}{8}$ diameter x 10 long anchor bolts spaced at 48" on center and within 12" of all sill plate ends or breaks. Provide additional anchor bolts as required by foundation plan and shear wall schedule. Anchor bolts shall be embedded 7" minimum into concrete.

4. Masonry

- A. General: All masonry work shall be done in accordance with the Masonry Standards Joint Committee (MSJC) Building Code requirements for masonry structures (TMS 402-13 / ACI 530-13 / ASCE 5-13), and the CBC 2019 edition, chapter 21.
- B. f'm = 1500 psi, (TMS 602 S-16 Table 2) Periodic Special Inspection is required.
- C. Concrete masonry units shall be of a quality at least equal to the requirements as set forth in ASTM C-90, medium weight units. Use open ended units with fully mortared joints. F'c = 1900 psi
- D. Mortar shall conform to ASTM C-270 type M or S with a compressive strength of 1250 psi at 7 days and 2500 psi at 28 days. Use mix ratios of 3 parts sand and 1 part type 2 Portland cement. Do not use masonry cement.
- E. Grout (fine grout) shall conform to ASTM C-476 with a compressive strength of 1250 psi at 7 days and 2000 psi at 28 days. Slump limits shall be a minimum of 8" and a maximum of 11". All masonry cells are to be solid grouted unless otherwise noted. Provide "Grout Aid" admixture in all grout. Follow manufacturer's recommendations for quantities and procedures.
- F. Reinforcing shall conform to ASTM A-615 Grade 40 or 60 for #4 and smaller and ASTM A-615 Grade 60 for #5 and larger. Epoxy coated reinforcing shall conform to ASTM A-775. See Sheet S1.3 for all required laps, bends \$ splices.
- G. All Masonry shall be laid true, level, plumb, and neatly in accordance with the plans. Use running bond, unless otherwise noted.
- H. No chipped, cracked, soiled, or otherwise imperfect block shall be used in the work.
- I. Consolidate all grout using an electric mechanical vibrator of suitable size for masonry work. Vibrate 3 minutes after grout is deposited.
- J. Maximum grout lifts shall be 5'-4" maximum. Cleanouts are not required.

5. Structural Steel and Miscellaneous Metals

- A. General: All structural steel work shall be done in accordance with the American Institude of Steel Construction Steel Construction Manual (AISC 360), Seismic Design Manual (AISC 341), and the CBC 2019 edition, chapter 22.
- B. Materials: All material specifications are to be retained by contractor and to be available to structural engineer at his request.
- 1. W Shapes: ASTM A-992 (50ksi) ASTM A-53 Grade B (35ksi) 2. Pipe Columns: 3. Hollow Structural Sections (HHS): ASTM A500 Grade B (46ksi)
- ASTM A-36 (36ksi) 4. Channels, Angles: 5. High Strength Bolts: ASTM A-325 (120ksi)
- 6. Anchor Bolts: ASTM A-307 or A-36 7. Welding Rod: Heavily coated, conforming to American Welding Society "specifications for Arc welding electrodes" of E70xxx classification. Use low hydrogen electrodes for welding reinforcing bars.
- C. Galvanizing: Galvanize all miscellaneous iron angles, clips and other elements exposed to weather or in processing areas.
- D. Shop Drawings: Submit a minimum of 2 sets of shop drawings to structural engineer for review (more sets shall be submitted if required by architect or general contractor).
- E. Shop Primer: Standard brand of rust inhibitive primer conforming to Federal Specification TT-P-86E, Type III. Galyanized metal primer shall conform to Federal Specification TT-P-64LD, Type II. Do not paint areas to be welded or embedded into masonry or concrete.
- F. Erection: All field welding shall be in accordance with American Welding Society Structural Welding Code, AWS D1.1, and all applicable revisions. All welds shall be made only by welders, tackers, and welding operators who have been previously qualified by test as prescribed by the AWS Structural Welding Code, AWS D1.1, and applicable revisions. Weld electrodes shall be as specified by the American Institute of Steel Construction, "Specification for the Design, Fabrication, and Erection of Steel for Buildings": Erection shall be in accordance with the American Institute of Steel Construction "Code of Standard Practice". Adequate temporary guying and bracing shall be installed during erection where needed to secure the framing against wind \$ seismic forces, erection equipment, and erection operations. Do not use gas cutting torches for correcting fabrication errors in the structural framing. Report any errors to the structural engineer immediately. All bolts to be tightened to "snug tight" conditions, unless noted otherwise.
- G. Special Inspection is required for all field welding and high strength bolt installation.
- H. Fabrication: Fabricate structural steel in accordance with AISC specifications and as indicated on the shop drawings as reviewed by the architect, structural engineer, and general contractor. Fabrication shop is to be licensed and approved by the building official. If fabrication shop is not licensed and approved, all welding performed in the shop is to be Special inspected by a licensed Special inspector.

1. General

- A. The contractor shall inform the Structural Engineer 48 hours in advance of reaching the following stages of construction: 1. Footing excavations completed.
- 2. Footing reinforcing bars in place.
- 3. Concrete placing operations.
- 4. Wood framing completed but not closed in.
- 5. Ply nailing completed but not covered.

6. All structural work completed.

- B. Details shown on the structural drawings are typical, similar details apply to similar conditions. Contractor shall verify existing conditions. Any existing conditions requiring construction different from that shown shall be reported to the architect immediately.
- C. All drawings shall be read in conjunction with the specifications, architectural, mechanical, electrical, and all other contract drawings as applicable.
- D. Dimensions shown shall take precedence over scale on plans, sections, and details. Notes and details on the drawings shall take precedence over general notes and typical details.
- E. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the method of construction. The contractor shall provide all measures necessary to protect the structure, workmen, or other persons during construction. OSHA regulations are to be strictly adhered to in providing the protective measures as stated above to include, but not limited to, all shoring, bracing, and underpinning.

Discrepancies shall be brought to the attention of the architect immediately.

- F. Unless otherwise noted on the structural drawings, use typical details on sheet: \$1.2 \ \$51.3 as applicable.
- G. Dimensions are to be checked and verified by the contractor. Discrepancies are to be reported to the architect immediately. Discrepancies are to be resolved before proceeding with work.
- H. All Special inspections are to be performed by a licensed Special Inspector approved by the building official and the structural engineer. The Special Inspector is an Agent of the Owner and is paid by the Owner, per CBC 2019, chapter 17 "Structural Tests and Special Inspections"

2. Foundation Design

Special Inspector.

- A. General: All foundation work shall be done in accordance with the CBC 2019 edition, chapter 18.
- B. The Soils Report for this project was prepared by: Pacific Materials Laboratory File Number: 16-11153-2 Dated: 12/22/16, Updated 10/25/17
- C. Foundation design is based on an allowable soil bearing pressure of: N/A psf (Caissons)
- D. Provide 90% minimum soil compaction under all slabs and structural foundation work, unless noted otherwise in the soils report.
- E. Earthwork shall be performed in accordance with the soils report and certified by a
- F. Prior to the contractor requesting a building department foundation inspection, the soils engineer (contractor, if soils report is not available) shall advise the building official in
- 1) The building pad was prepared in accordance with the approved plans and the soils
- report (if available).
- 2) The utility trenches have been properly backfilled and compacted. The foundation excavations, forming, and reinforcements comply with the approved plans and the soils report (if available).
- G. Foundation excavations shall not disturb adjacent existing structures.

Residential: (Light Frame Construction, Two Stories or Less)

the structural engineer a minimum of 2 days prior to concrete pour.

3. Concrete

- A. General: All concrete work shall be done in accordance with the ACI Building Code (ACI 318-14), the ACI manuals of concrete practice, and the CBC 2019 edition,
- B. Mix Design: Concrete shall have the following 28 day compressive strengths (unless specifically noted otherwise on plans):

Typical:		
Slabs on Grade, Footings3000	psi	*
Grade Beams, Caissons, Structural Slabs, Basement Walls		
Retaining Walls3000	psi	*
*Indicates Special Inspection is required.		

- Slabs on Grade, Footings. ..2500 psi (Special Inspection is Not Required) Mix Designs shall be prepared by the concrete supplier or testing agency and reviewed by
- C. Cement: Shall Conform to ASTM C-150, Type 2, Low Alkali. Minimum 5 ½ sacks per
- D. Aggregate: Fine aggregate to conform to ASTM C-33. Coarse aggregate to comform to ASTM C-33. Maximum size to be 1" (maximum of 1 ½" will be acceptable for footings only). Aggregate gradation shall conform to ASTM C-33. Pea Gravel shall not be used.
- E. Reinforcing Materials: #4 and smaller shall conform to ASTM A-706 or A-615 Grade 40 or 60. #5 and larger shall conform to ASTM A-706 or A-615 Grade 60. Steel welded wire reinforcement shall conform to ASTM A-1064. Epoxy coated reinforcing shall conform to ASTM A-775 or A-884. See Sheet S1.3 for all required laps, bends \$ splices. #6 bars and larger are to be shop fabricated. Make all bends cold.
- F. Concrete consistency: Slump limits shall be a minimum of 2 ½" and a maximum of 4". Slabs on grade may be placed with a maximum slump of 5"
- G. Mix Designs shall be submitted as specified by the architect. If not specified, submit a minimum of two copies to the structural engineer a min. of 2 days prior to pour.
- H. Admixtures: Obtain structural engineers approval for all admixtures not noted: Flyash shall conform to ASTM C-618, type F; air entraining admixtures shall conform to ASTM C-260; water reducing admixtures shall conform to ASTM C-494 or ASTM C-1017.

1. Check with all trades to insure proper placement of all inserts, sleeves, openings,

conduits, etc. prior to pouring concrete. All penetrations through grade beams and all penetrations larger than 6" in diameter are to be approved by the structural engineer.

J. All sleeves not specifically shown on the drawings shall be located by the trades involved and shall be reviewed by the structural engineer.

Building Code: CBC 2019 Edition (Based on IBC 2018 Edition)

Structural Observation Program (CBC 2019; 1704.6)

- A. The owner shall employ the Engineer of Record licensed in the State of California who is responsible for the structural design, to do structural observation.
- B. Engineer of Record: Keyin L. Vandervort, Registration/License Number: S 3734
- C. Designated Engineer to do Structural Observation: Keyin L. Vandervort, Registration/License Number: S 3734. EOR shall be contacted at least 48 hours in advance to schedule Structural Observations.
- D. The Engineer responsible for the Structural Observation, the Contractor, and appropriate Subcontractors shall hold a pre-construction meeting to review the details of the structural system to be structurally observed.
- E. Foundation: Structural elements to be observed prior to placement of concrete in the foundation. Footing excayations width and depth, reinforcing placement, holdown anchors and anchor bolts at shear walls, and utility line penetrations through structural elements.
- F. Floor Framing: Structural elements to be observed at each floor level. Shear wall construction, shear transfer, floor sheathing nailing.
- G. Roof Framina: Structural elements to be observed prior to roof covering. Shear wall construction, shear transfer, roof sheathing nailing, prefabricated roof truss installation (where applicable)
- H. FINAL OBSERVATION: Structural elements to be observed at the final observation visit. All drag straps and holdown straps installed, utility rough-ins, and all previous corrections completed.

Project Design Gravity Loads

Roof Loads

Dead Loads: 32 psf 20 Live Loads: psf (4:12 Slope)

<u>Floor Loads</u>

N/A psf (Structural Slab) Dead Loads: Live Loads: **40** psf (Residential)

Partition Loads N/A psf

Project Design Lateral Loads Risk Category II

<u> Wind Design Data</u> (Directional Procedure, Simplified Method) (ASCE 7-16 27.4)

Basic Wind Speed (Vult): 110 mph Exposure: **B** Height Above Ground Level (z): 15'

Maximum Horizontal Design Pressure: 21.89 psf

<u>Seismic Design Data</u>

Equivalent Lateral Force Procedure (ASCE 7, 12.8) Site Latitude: 34.418 Site Longitude: -119.752

Site Class: **D** R: **6.5** (Plywood Shear Walls)

IE: 1.0 Ss: **2,332**

S1: **0.823** Sps: 1,866 Cs: **0,287** Seismic Design Category: E

ρ: **1,3** $V = \rho C S M D L = 0.373 M D L$

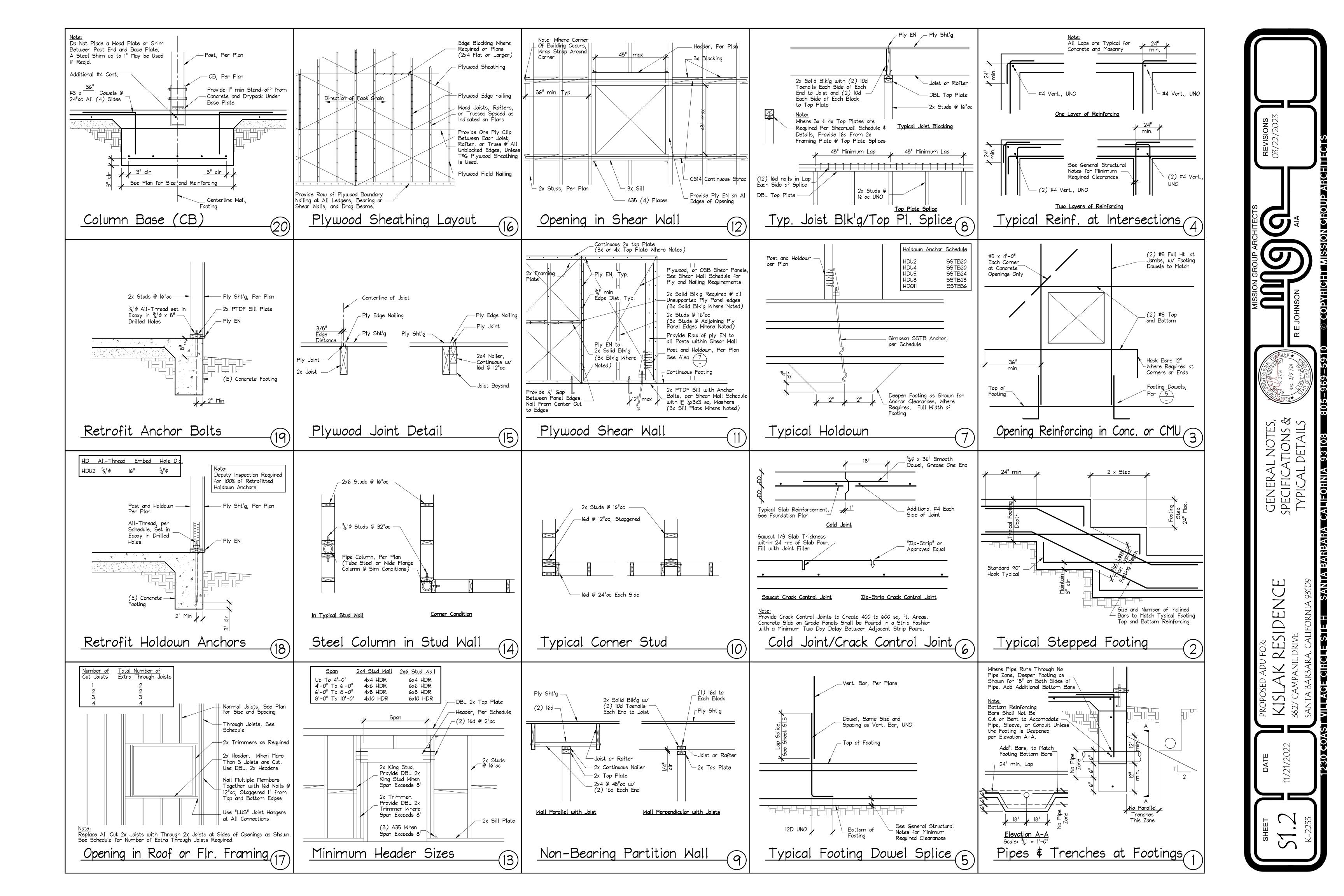
Tests and Inspections (X) The following items require Special Inspection per CBC 2019 edition, chapter 17.

See plans and details for specific locations. STATEMENT OF SPECIAL INSPECTIONS

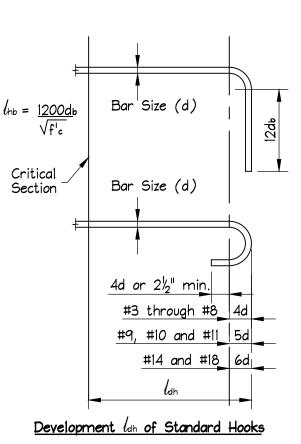
	QUIRING SPECIAL INSPECTION AND STRUCTURAL	SPECIA	L INSPECT.	STRUCT.	SOILS
	TION DURING: (TO BE SELECTED & CHECKED BY GN PROFESSIONAL OF RECORD PER CBC 2019)	CONT.	PERIODIC	OBSERV	
	PLACEMENT OF COMPACTED FILL, GRADING, AND EXCAVATIONS				X
GRADING &	HELICAL PIER INSTALLATION				
FOUNDATION	CAISSON EXCAVATION		Х		Х
	OTHER				
	MIX DESIGN			Х	
CONCRETE	REINFORCING PLACEMENT		Х	Х	1
	COMPRESSION TESTING				1
	PNUEMATICALLY PLACED CONCRETE				1
	STRUCTURAL - CONCRETE f' _c ≥ 3,000 psi		Х	Х	1
	FOUNDATION / SLAB ON GRADE. CONC. f'c = 2,500 psi				1
	NON-SHRINK GROUT				1
	BOLTS INSTALLED IN CONCRETE				1
	OTHER				1
	INSTALLATION OF ADHESIVE ANCHORS, RODS & DOWELS				1
DRILLED	EXPANSION ANCHORS				1
ANCHORS	TITEN HD ANCHORS				1
	OTHER				
	SHOP WELDING, NON-CERTIFIED & APPROVED SHOPS		Х		
	FIELD WELDING				1
STRUCTURAL	STRUCTURAL STEEL ERECTION			Х	
STEEL &	HIGH STRENGTH BOLT CONNECTIONS				1
WELDING	REINFORCING WELDING				
	METAL DECKING WELDING				1
	OTHER				1
	GROUT MIX DESIGN				1
	REINFORCING PLACEMENT IN FOOTING				1
	REINFORCING PLACEMENT IN WALL				1
MA CONDY	SAMPLE & TEST MORTAR				1
MASONRY	SAMPLE & TEST GROUT				1
	MASONRY UNIT PLACING & GROUTING				1
	PRISM TESTS				1
	BRICK PUSH TESTS				1
	OTHER				1
	STRUCTURAL FRAMING MEMBERS & CONNECTIONS			Х	1
	DIAPHRAGM NAILING			Х	1
WOOD	SHEAR WALLS, HOLDOWNS & SHEAR TRANSFER			Х	1
CONSTR.	DRAG BEAMS, STRAPS & CONNECTIONS			Х	1
	FINAL FRAMING			Х	1
	OTHER				1
MISC.	SPRAY APPLIED FIREPROOFING				1
TESTS	OTHER				1
	1		•	•	

YPIC GESPE

> DE S RE



<u>Description of Building Elements</u> <u>Roof</u>	Number and Type of Fastener	<u>Spacing and Location</u>	<u>Description of Building Elements</u> <u>Wall</u>	Number and Type of Fastener	<u>Spacing and Location</u>
. Blocking between ceiling joists, rafters or trusses to top plate or other framing below	3-8d common (2½"x0.131"); or 3-10d box (3" x 0.128"); or 3-3"x0.131" nails 3-3" 14 gage staples, ¾6" crown	Each end, toenail	19. 1" brace to each stud and plate	2-8d common (2½"x0.131"); or 2-10d box (3"x 0.128"); or 2-3"x0.131" nails; or 2-3" 14 gage staples, ¾6" crown	Face nail
	2-8d common (2½"x0.131") 2-3"x0.131" nails	Each end, toenail	20. 1" x 6" sheathing to each bearing	2-8d common $(2\frac{1}{2}"x0.131")$; or 2-10d box $(3"x 0.128")$	Face nail
Blocking between rafters or truss not at the wall top plate, to rafter or truss	2-3" 14 gage staples 2-16d common (3½"x0.162)		21. 1" x 8" and wider sheathing to each bearing	3-8d common (2½"x0.131"); or 3-10d box (3"x 0.128")	Face nail
	2-3"x0.131" nails 2-3" 14 gage staples 16d common (3½"x0.162) @ 6"oc	End nail	<u>Floor</u> 22. Joist to sill, top plate, or girder	3-8d common (2½"x0.131"); or floor 3-10d box (3"x 0.128"); or 3-3"x0.131" nails; or 3-3" 14 gage staples, ¾6" crown	Toenail
Flat blocking to truss and web filler	2-3"x0.131" nails @ 6"oc 2-3" 14 gage staples @ 6"oc 3-8d common (2½" x 0.131"); or 3-10d box (3"x0.128"); or	Face nail	23. Rim joist, band joist, or blocking to top plate, sill or other framing member	8d common (2½"x0.131"); or 10d box (3"x 0.128"); or 3"x0.131" nails; or 3" 14 gage staples, 7/6" crown	6"oc, toenail
. Ceiling joists to top plate	3-3"x0.131" nails 3-3" 14 gage staples, 7/6" crown	Each joist, toenail	24. 1" x 6" subfloor or less to each joist	2-8d common (2½"x0.131"); or	Face nail
. Ceiling joist not attached to parallel rafter, laps	3-16d common $(3\frac{1}{2}^{"}x0.162")$; or 4-10d box $(3^{"}x0.128")$; or		25. 2" subfloor to joist or girder	2-10d box (3"x 0.128") 2-16d common (3½"x0.162")	Face nail
over partitions (no thrust) (see Section 2308.7.3.1, Table 2308.7.3.1)	4-3"x0.131" nails 4-3" 14 gage staples, 7/6" crown	Face nail	26. 2" planks (plank \$ beam - floor \$ roof)	2-16d common $(3\frac{1}{2}^{"}x0.162")$	Each bearing, face nail
Ceiling joist attached to parallel rafter (heel	Per Table 2308.7.3.1	Face nail		20d common (4"x0.192")	32"oc, face nail at top and bottom staggered on opposit sides
joint) (see Section 2308.7.3.1, Table 2308.7.3.1) Collar tie to rafter	3-10d common (3"x0.148"); or 4-10d box (3"x0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 7/6" crown	Face nail	27. Built-up girders & beams, 2" lumber layers	10d box (3"x 0.128"); or 3"x0.131" nails; or 3" 14 gage staples, 7/6" crown And:	24"oc, face nail at top and bottom staggered on opposit sides
. Rafter or roof truss to top plate (See Secion 2308.7.5, Table 2308.7.5)	3-10d common (3"x0.148"); or 3-16d box (3½"x0.135"); or 4-10d box (3"x 0.128"); or 4-3"x0.131" nails	Toenaiļ ^c		2-20d common (4"x0.192") 3-10d box (3"x 0.128"); or 3-3"x0.131" nails; or 3-3" 14 gage staples, 7/6" crown	Ends and at each splice, fac
	4-3" 14 gage staples, 7/6" crown 2-16d common (3½"x0.162"); or 3-10d box (3"x 0.128"); or		28. Ledger strip supporting joists or rafters	3-16d box (3½"x0.135"); or 4-10d box (3"x 0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, ¾6" crown	Each joist or rafter, face nail
. Roof rafters to ridge valley or hip rafters; or roof rafter to 2-inch ridge beam	3-3"x0.131" nails 3-3" 14 gage staples, 76" crown; or 3-10d common (3"x0.148"); or 3-16d box (3½"x0.135"); or	End nail	29. Joist to band joist or rim joist	3-16d box (3½"x0.135"); or 4-10d box (3"x 0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 76" crown	End nail
	4-10d box (3"x 0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, %" crown	Toenail	30. Bridging or blocking to joist, rafter or truss	2-8d common (2½"x0.131"); or 2-10d box (3"x 0.128"); or 2-3"x0.131" nails 2-3" 14 gage staples, 1/6" crown	Each end, toenail
<u>Wall</u>	16d common (3½"x0.162");	24"oc face nail	Wood structural panels (WSP), subfloor, roof and	2-3 14 gage staples, 716 crown	Edges Intermediate support (inches) (inches)
Stud to stud (not at braced wall panels)	10d box (3"x 0.128"); or 3-3"x0.131" nails	16"oc face nail	interior wall sheathing to framing and particleboard wall sheathing to framing ^a	6d common or deformed (2"x0.113") (subfloor and wall)	6 12
	3-3" 14 gage staples, 7/6" crown 16" common (3½"x0.162"); or	16"oc face nail		8d box or deformed (2½"x0.131)(roof)	6 12
Stud to stud and abutting studs at intersecting wall corners (at braced wall panels)	16d box $(3\frac{1}{2}^{"}x0.135")$; or	12"oc face nail	31. 3/8" - 1/2"	$2\frac{3}{8}$ "x0.113" nail (subfloor and wall)	6 12
	3"x0.131" nails 3-3" 14 gage staples, 7/6" crown	12"oc face nail		$1\frac{3}{4}$ " 16 gage staple, $\frac{7}{16}$ " crown (subfloor and wall)	4 8
Built-up header (2" to 2") header)	16" common (3½"x0.162"); or	16"oc each edge, face nail		23%"x0.113" nail (roof) 134" 16 gage staple, 76" crown (roof)	4 8
	16d box (3½"x0.135");	12"oc each edge, face nail		8d common (2½"x0.131); or	3 6
Continuous header to stud	4-8d common (2½"x0.131"); or 4-10d box (3"x0.128")	Toenai!	32. ¹ % ₂ " - ³ / ₄ "	6d deformed (2"x0.113") 23%"x0.113" nail; or	6 12
. Top plate to top plate	16" common (3½"x0.162"); or 10d box (3"x 0.128"); or	16"oc face nail	33. 7/8" - 1/4"	2" 16 gage staple, 76" crown 10d common (3"x0.148"); or	4 8
	3-3"x0.131" nails 3-3" 14 gage staples, 7/6" crown	12"oc face nail	Other exterior wall sheathing	8d deformed (2½"x0.131)	6 12
. Top plate to top plate, at end joints	8-16d box (3½"x0.135"); or 12-10d box (3"x 0.128"); or 12-3"x0.131" nails	Each side of end joint, face nail (minimum 24" lap splice length each side of end	34. ½" fiberboard sheathing ^b	$1\frac{1}{2}$ " galvanized roofing nail ($\frac{1}{16}$ " head diameter); or $1\frac{1}{4}$ " 16 gage staple with $\frac{1}{16}$ " or 1" crown	3 6
. Top place to top place, at ena Joints	12-3" 14 gage staples, 7/6" crown 16" common (3½"x0.162"); or	joint) 16"oc face nail	35. ²⁵ ⁄ ₃₂ " fiberboard sheathing ^b	$1\frac{1}{2}$ " galvanized roofing nail ($\frac{7}{16}$ " head diameter); or $1\frac{1}{4}$ " 16 gage staple with $\frac{7}{16}$ " or 1" crown	3 6
. Bottom plate to joist, rim joist, band joist or blocking (not at braced wall panels)	16d box (3½"x0.135"); or 3"x0.131" nails	12"oc face nail	<u>Mood structural panels combinations subfloor</u> <u>underlayment to framing</u> 36. 34" and less	8d common $(2\frac{1}{2}^{n} \times 0.131)$; or 6d deformed $(2^{n} \times 0.113^{n})$	6 12
	12-3" 14 gage staples, 7/6" crown 2-16d common (3½"x0.162"); or		37. %" - 1"	8d common (2½"x0.131); or 8d deformed (2½"x0.131)	6 12
. Bottom plate to joist, rim joist, band joist or blocking at braced wall panels	3-16d box (3"x 0.135"); or 4-3"x0.131" nails 4-3" 14 gage staples, 7/6" crown	16"oc face nail	38. 1/8" - 1/4"	10d common (3"x0.148); or 8d deformed (2½"x0.131)	6 12
	4-8d common (2½" x 0.131"); or 4-10d box (3"x 0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 76" crown; or	Toenail	Panel siding to framing 39. ½" or less	6d corrosion-resistant siding (1%"x0.106"); or 6d corrosion-resistant casing (2"x0.099")	6 12
5. Stud to top or bottom plate	2-16d common (3½"x0.162"); or 3-10d box (3"x 0.128"); or 3-3"x0.131" nails; or 3-3" 14 gage staples, 76" crown	End nail	40. %"	8d corrosion-resistant siding (23%"x0.128"); or 6d corrosion-resistant casing (2½"x0.113)	6 12
7. Top or bottom plate to stud	2-16d common (3½"x0.162"); or 3-10d box (3"x 0.128"); or 3-3"x0.131" nails; or 3-3" 14 gage staples, 76" crown	End nail	Mood structural panels (MSP), subfloor, roof and interior wall sheathing to framing and particleboard wall sheathing to framing ^a 41. 1/4"	4d casing ($1\frac{1}{2}$ "x0.080"); or 4d finish ($1\frac{1}{2}$ "x0.072")	6 12
8. Top plates, laps at corners and intersections	2-16d common (3½"x0.162"); or 3-10d box (3"x 0.128"); or 3-3"x0.131" nails; or 3-3" 14 gage staples, ½" crown	Face nail	42. ¾"	6d casing (2"x0.199"); or 6d finish (Panel supports at 24 inches)	6 12

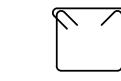


Detailing Dimension

Detailing Dimension 4D or $2\frac{1}{2}^{"}$ min.

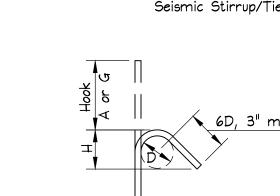






Alternate 2 Piece Seismic Stirrup/Tie

135° Bend



Recommended End Hooks

	Hook Develop	ment Lengths
ze	$f'_c = 2500 psi$	$f^{1}c = 3000$ psi
3	10"	9"
4	12"	11"
5	16"	14"
6	19"	17"
7	21"	19"
გ	25"	22"

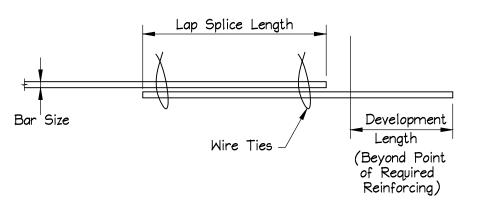
Notes:
1. Tabulated values are based on Grade 40 or 60 reinforcing bars and normal-weight concrete.
2. Tension development lengths of standard hooks are based on ACI 318-14, Sections 25.4.3. Lengths are in inches.
3. For epoxy coated hooks, multiply by 1.2

90° Hook

Bar Size	D	18 <i>0°</i> +	90° Hook			
Size	(in.)	A or G	7	A or G		
#3	21/2"	5"	3"	6"		
#4	3"	6"	4"	8"		
#5	3¾"	7"	5"	10"		
#6	4½"	8"	6"	12"		
#7	5¼"	10"	7"	14"		
#8	6"	11"	8"	16"		

<u>Seisn</u>	<u>nic</u>	Stirr	up/T	<u>ies</u>
			1	

Bar	D	135° Seismic Hook			
Size	(in.)	A or G	Н		
#3	1½"	4¼"	3"		
#4	2"	4½"	3"		
#5	21/2"	5½"	3¾"		
#6	4½"	8"	4½"		
#7	5¼"	9"	5¼"		
#8	6"	101/2"	6"		



* Use Class B Splice, Unless Noted Otherwise

Tension Development \$ Lap Splice Lengths - Masonry

GIOGIT POYO, OPITION OF LONG TO LONG TO THE CONTRACT								
Bar	D	Development \$ Lap Splice Lengths						
Size	(in.)	Edge	Center	40db or 12" min.				
#2	40	17"	12"	12"				
	60	26"	14"	12"				
#3	40	26"	14"	15"				
	60	38"	21"	15"				
# 1	40	34"	18"	20"				
#4	60	51"	27"	20"				
#5	60	63"	34"	25"				
#6	60	99"	53"	30"				
#7	60	115"	62"	35 ["]				
#8	60	152"	81"	40"				

Edge dimension (cover) > 2" min.
 Center dimension (cover) > 3.75"
 Multiply by 1.5 for Epoxy Coated Reinforcing

Compression Development and Lap Splice Lengths - Concrete

Bar	Grade	Developme	ent Length	Lap S	oplice
Size	Grade	2500psi	3000psi	2500psi	3000psi
#3	40	8"	8"	16"	12"
	60	10"	9"	16	12
ш.,	40	9"	8"	20"	15"
#4	60	12"	11"	20	l lo
#5	60	14"	14"	26"	19"
#6	60	16"	17"	31"	23"
#7	60	18"	19"	36"	27"
#8	60	20"	22"	40"	30"

Notes:

1. Tabulated values are based on Grade 40 or 60 reinforcing bars and

normal-weight concrete.

2. Compression development lengths and compression lap splice lengths are based on ACI 318-14, Sections 25.4.9 and 25.5.5, respectively. Lengths are in inches.

Tension Development \$ Lap Splice Lengths - Concrete

Bar Size	Grade	f'c	Devel. Length	Class A Splice	Class B Splice*
#2	40	2500	12"	12"	15"
	40	3000	12"	12"	13"
	60	2500	16"	16"	20"
	60	3000	14"	14"	19"
	40	2500	17"	17"	22"
ا بير ا	40	3000	15"	15"	19"
#3	60	2500	25"	25"	32"
		3000	22"	22"	28"
	40 60	2500	22"	22"	29"
#4		3000	20"	20"	25"
#4		2500	32"	32"	42"
		3000	29"	29"	37"
#5	60	2500	40"	40"	52"
		3000	36"	36"	47"
#6	60	2500	48"	48"	62 ["]
	60	3000	43"	43"	56"
#7	60	2500	70"	70"	90"
	60	3000	63"	63 ["]	81"
#8	60	2500	79"	79"	103"
#0	<i>6</i> 0	3000	72"	72"	93"
Mataa					

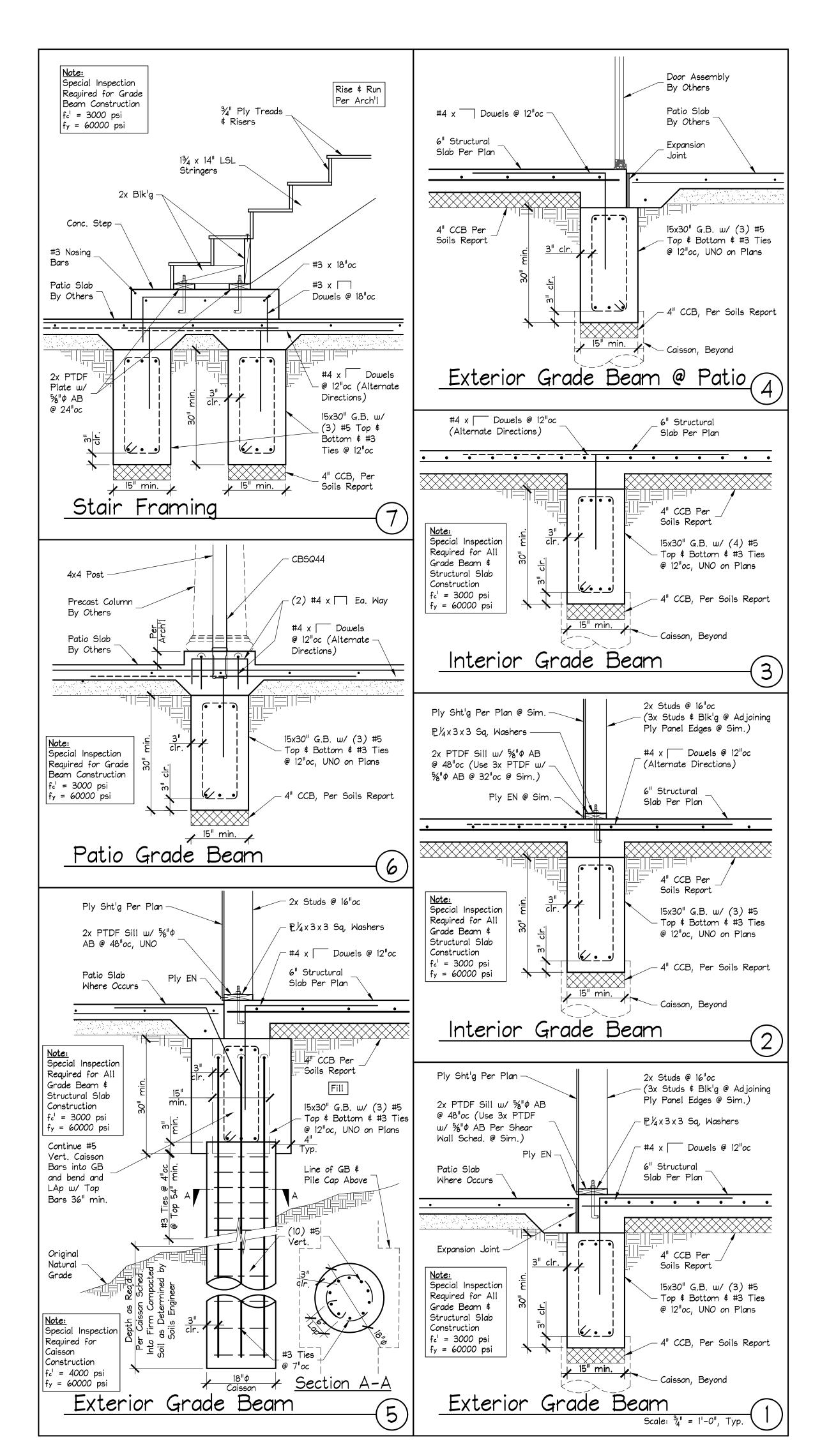
- Notes:

 Tabulated values are based on Grade 40 or 60 reinforcing bars and normal-weight concrete.

 Tension development lengths and tension lap splice lengths are based on ACI 318-14, Sections 25.4.2.2 and 25.5, respectively. Tabulated values for beams or columns are based on transverse reinforcement and concrete cover meeting minimum Code requirements. Lengths are in
- 3. Lap Class A values are the required tension development lengths, 6 ; lap splice lengths are multiples of tension development lengths; Class A splice = 1.06 and Class B splice = 1.36
- (ACE 318-14, Section 25.5.1).

 4. For lightweight aggregate concrete, multiply the tabulated values by 1.3.
- 5. For epoxy-coated bars, multiply the tabulated values by 1.20.

Reinforcing Laps, Bends \$ Splices



TYPICAL FOUNDATION NOTES

- 1. Prepare Site per Soils Report By: Braun \$ Associates, Inc. Dated: December 22, 2016, Updated: March 31, 2018 File # 16-11153-2
- 2. Contractor to Verify all Dimensions and Coordinate with Architectural Plans.
- 3. All Anchor Bolts, Slab Dowels, etc. are to be Tied in Place and Holdown Anchors Set in Place by Template Prior to Requesting a Building Department Foundation
- 4. For Shear Wall Anchor Bolting, See Shear Wall Schedule.
- 5. Prior to the Contractor Requesting a Building Department Foundation Inspection, the Soils Engineer Shall Advise the
- a. The Building Pad was Prepared in Accordance with the Soils Report
- b. The Utility Trenches have been Properly Backfilled and Compacted

Building Official in Writing that:

- c. The Foundation Excavations Comply with the Intent of the Soils Report.
- 6. Holdowns shall be Retightened just Prior to Covering the Wall Framing.
- 7. All Connectors and/or Fasteners in Contact with Pressure Treated Lumber shall be Stainless Steel (304 or 316) or Hot Dipped Galvanized (ASTM A-123 \$ A-153) G85 minimum.
- Shear Wall Designation

 10' Minimum Shear Wall Length SHEAR WALL SCHEDULE ____Extents of Shear Wall Sill Anchorage Blk'g, Rim or Drag to Top Plate Framing Allowable Sill * Studs\(Blk'\) @ Plate Ply Panel Edges Ply and Nailing Inspection Shear Load Concrete Wood Solid Member TJI Blk'g Plate ⅓₂" Ply C-DX 10d @ 6"oc EN 310 #/ft N/A 16d @ 6"oc A35 @ 16"oc 2x 2x 5/₈"φx12" AB @ 48"oc 16d @ 6"oc 10d @ 12"oc FN 15/32" Ply C-DX ½"Φx6" SDS 10d @ 4"oc EN 5/₈"φx12" AB @ 32"oc 460 #/ft N/A 2x 3x A35 @ 12"oc 16d @ 4"oc 3crews @ 8"00 10d @ 12"oc FN 15/32" Ply Struct. ½"Φx6" SDS 5/₈"φx12" AB @ 24"oc A35 @ 8"oc N/A N/A 10d @ 3"oc EN Зх Зх 665 #/ft Screws @ 6"oc 10d @ 12"oc FN
 - 1. Use Common Nails Only, Do Not Use Sinkers! 10d Min Shank Dia. = .148"
 - 2. Use ½"x3"x3" Square Washers at All Anchor Bolts

FOUNDATION PLAN

- 3. Where Nails Are Spaced Closer Than 6"oc, the nails Shall be Staggered. 4. Site Built Shear Walls to Maintain a Minimum 2:1 Height to Width Ratio, U.N.O.
- 5. Install Simpson Strong Wall Per Manufacturer's Recommendations.
- 6. Simpson Strong Wall Concrete Templates Shall be Used to Correctly Position AB's and HD Anchors for Simpson Strong Wall Products.
- 7. For Shear Wall Construction Details, see Detail (11)

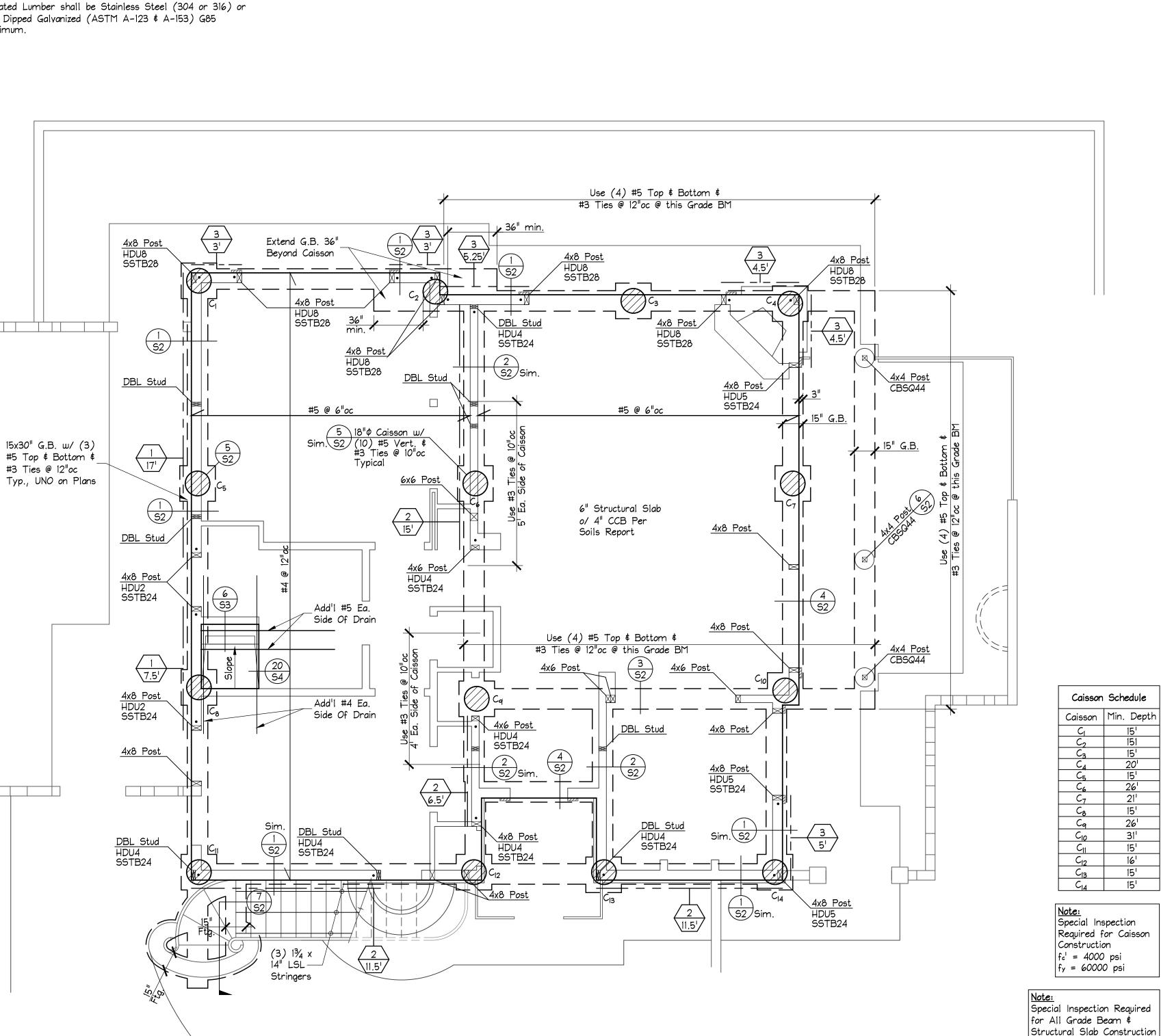
- 8. Use (2) 20d Common Nails to Each Stud In Lieu of (2) 16d where 3x Sill Occurs
- 9. Where Panels Are Applied on Both Faces of a Shear Wall and Nail Spacing is Less Than 6"oc on Either Side, Panel Joints Shall be Offset to Fall on Different Framing Members. Alternatively, the Width of the Nailed Face of Framing Member Shall be 3" Nominal or Greater @ Adjoining Ply Panel Edges. Nails at All Adjoining Panel Edges Shall be Staggered.

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* WSWH Shear Walls: ICC-ES ESR-2652 ** SSW Shear Walls: ICC-ES ESR-1679



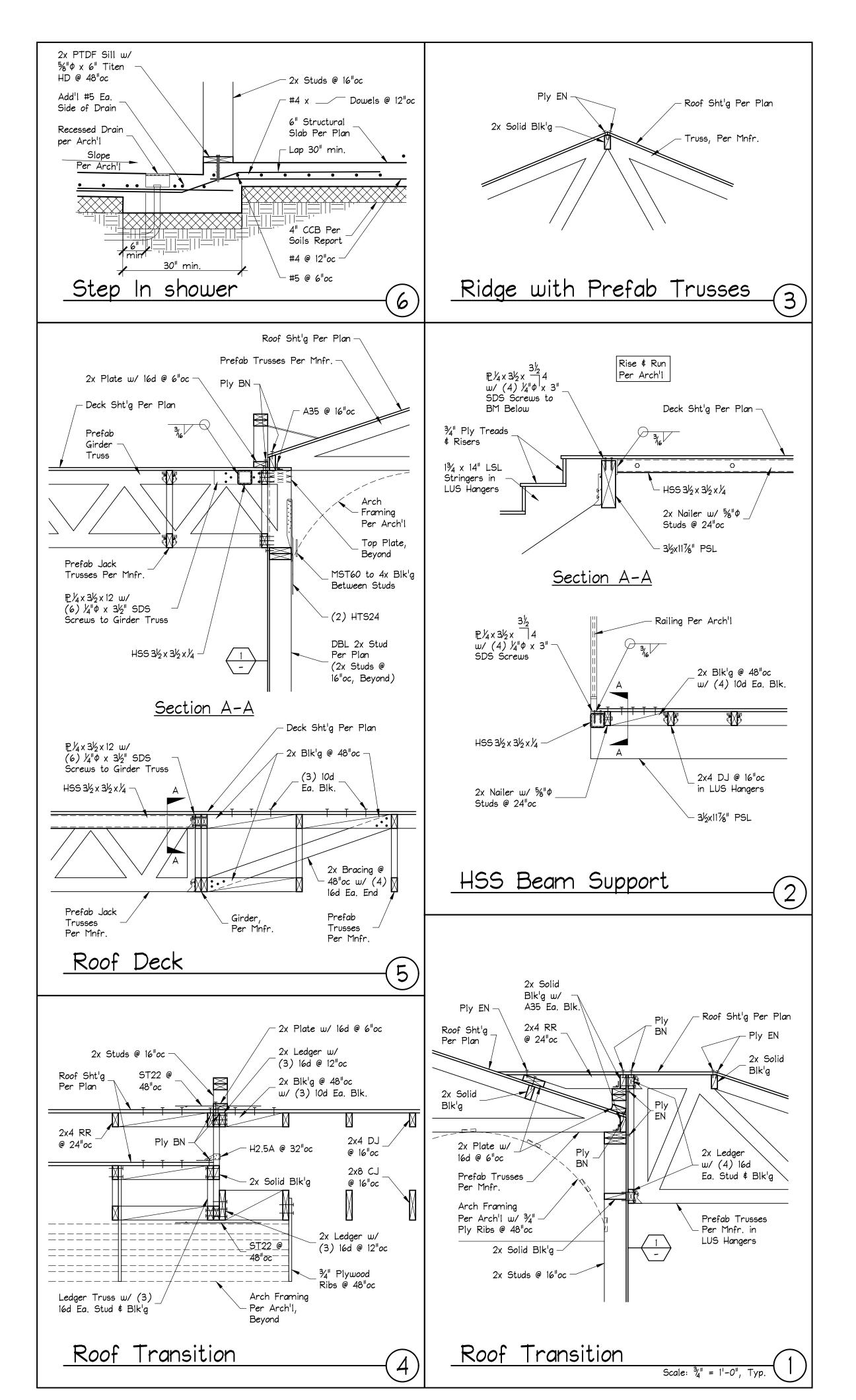
1/4'' = 1'-0''

EOR to be Notified 48 Hours in Advance to Schedule Structural Observations as Noted in the Structural Observation Program on Sheet S1.1 Required Special Inspections as Noted on S1.1 to be

 $f_c^1 = 3000 \text{ psi}$

 $f_y = 60000$ psi

Performed by an Approved and Licensed Special Inspector

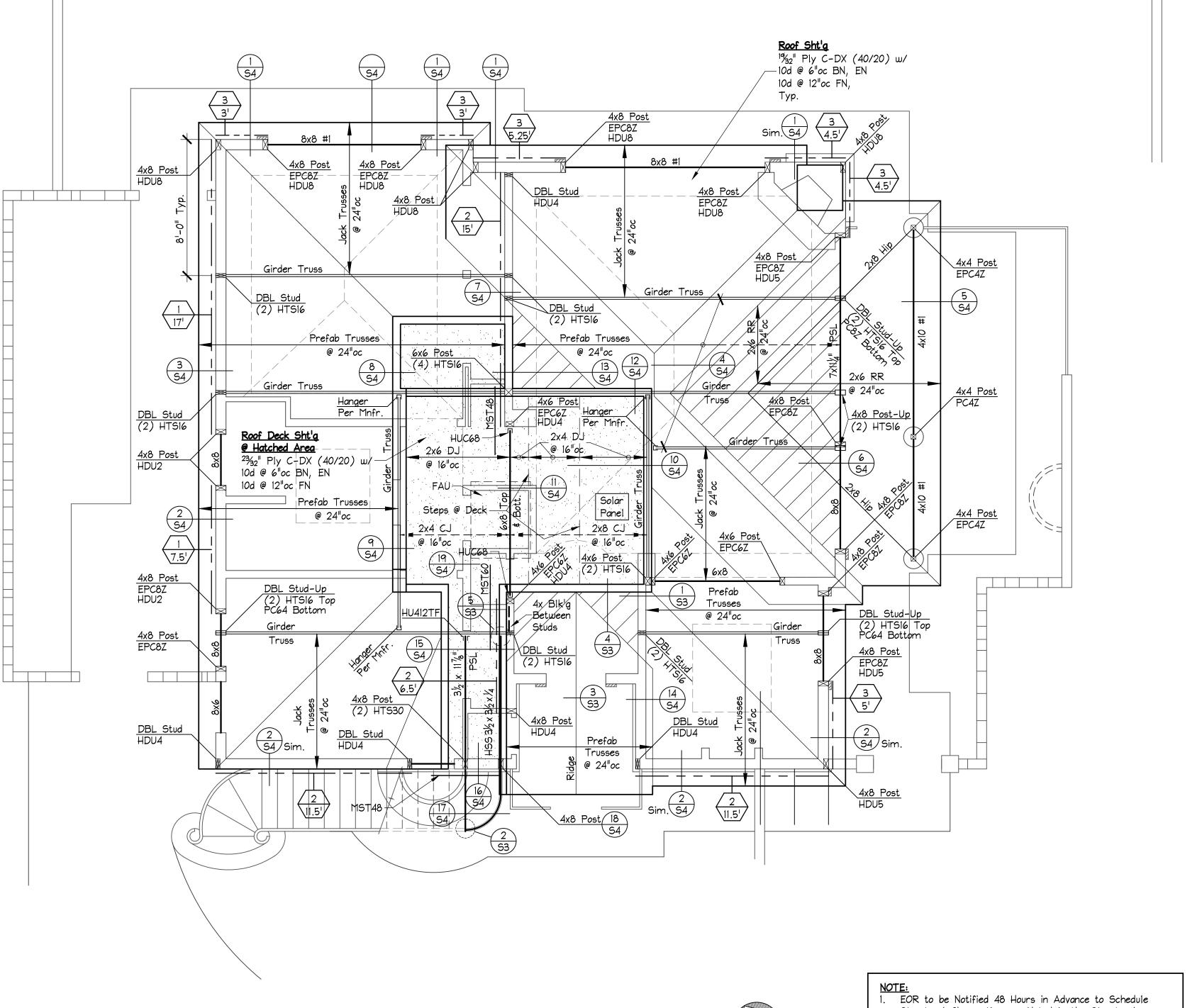


10'	-Shear Wall Designation -Minimum Shear Wall Lo -Extents of Shear Wall			SHEAR WALL	SCHEDULE					
		Sill Anchorage	;	Blk'g, Rim or Dro	ng to Top Plate		Fram	ning	Allowable	Special
SYM	Ply and Nailing	Concrete	Wood	Solid Member	TJI Bik'g	Top Plate	Sill * Plate	Studs‡ Blk'g @ Ply Panel Edges	Shear Load	Inspection
	¹⁵ / ₃₂ " Ply C-DX 10d @ 6"oc EN 10d @ 12"oc FN	%"Φx12" AB @ 48"οc	16d @ 6"oc	A35 @ 16 ["] oc	16d @ 6"oc	2x	2x	2x	310 #/ft	N/A
2	"5 ₃₂ " Ply C-DX 10d @ 4"oc EN 10d @ 12"oc FN	½" Фх12" AB @ 32"oc	½"\$x6" SDS Screws @ 8"oc	A35 @ 12"oc	16d @ 4"oc	2x	3x	3х	460 #/ft	N/A
3	¹⁵ 32" Ply Struct. 1 10d @ 3"oc EN 10d @ 12"oc FN	½"Φx12" AB @ 24"οc	½"\$x6" SDS Screws @ 6"oc	A35 @ 8"oc	N/A	3x	3x	3x	665 #/ft	N/A

- 1. Use Common Nails Only, Do Not Use Sinkers! 10d Min Shank Dia. = .148"
- 2. Use ¼"x3"x3" Square Washers at All Anchor Bolts
- 3. Where Nails Are Spaced Closer Than 6"oc, the nails Shall be Staggered. 4. Site Built Shear Walls to Maintain a Minimum 2:1 Height to Width Ratio, U.N.O.
- 5. Install Simpson Strong Wall Per Manufacturer's Recommendations.
- 6. Simpson Strong Wall Concrete Templates Shall be Used to Correctly Position AB's and HD Anchors for Simpson Strong Wall Products.
- 7. For Shear Wall Construction Details, see Detail 11

ROOF FRAMING PLAN

- 8. Use (2) 20d Common Nails to Each Stud In Lieu of (2) 16d where 3x Sill Occurs
- 9. Where Panels Are Applied on Both Faces of a Shear Wall and Nail Spacing is Less Than 6"oc on Either Side, Panel Joints Shall be Offset to Fall on Different Framing Members. Alternatively, the Width of the Nailed Face of
- Framing Member Shall be 3" Nominal or Greater @ Adjoining Ply Panel Edges. Nails at All Adjoining Panel Edges Shall be Staggered.
- * WSWH Shear Walls: ICC-ES ESR-2652
 ** SSW Shear Walls: ICC-ES ESR-1679



= 1'-0"

NOTE:

1. EOR to be Notified 48 Hours in Advance to Schedule
Structural Observations as Noted in the Structural
Observation Program on Sheet S1.1

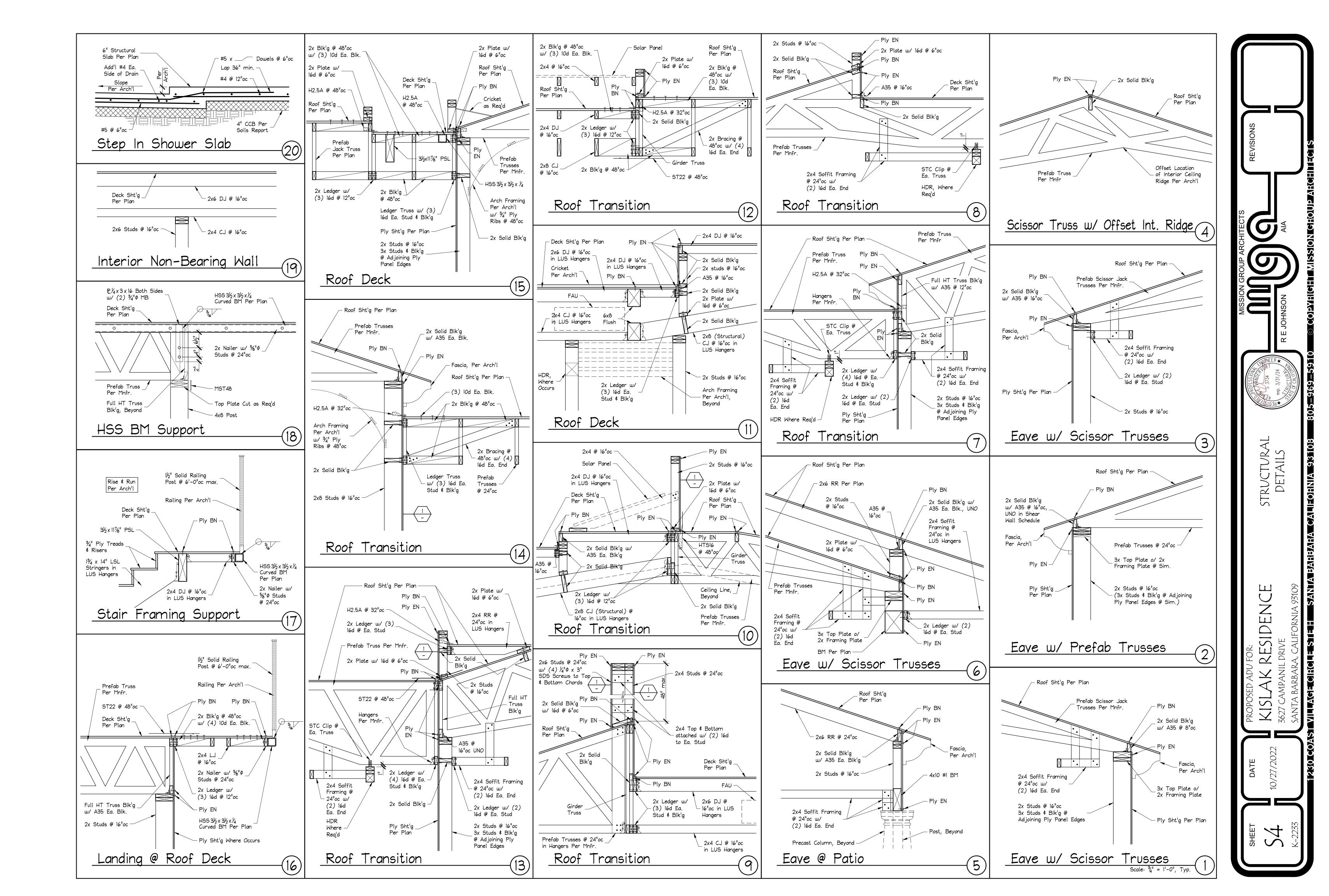
2. Required Special Inspections as Noted on S1.1 to be
Performed by an Approved and Licensed Special Inspector

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ROOF PLAN

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COMMUNITY DEVELOPMENT DEPARTMENT

630 GARDEN ST. SANTA BARBARA, CA 93101 (805) 564-5578 | SantaBarbaraCA.gov

FINAL APPROVAL CHECKLIST

SUPPLEMENTAL APPLICATION





WHAT IS FINAL APPROVAL?

Final approval is the last level of design review before applying for a Building Permit (BLD) application. Final approval generally occurs at a separate hearing, after project design approval, and includes a complete set of working drawings with all details, color samples, door hardware, and exterior lighting fixtures for review. Applicants may also request project design approval and final approval on the same hearing date, if sufficient details are provided.

HOW DOES THE PROCESS WORK?

Once a project receives project design approval, it shall constitute the substantive design approval of the project. If substantial changes to the plans are proposed after project design approval, a new project design approval will be required. Design review comments on final approval should only address whether the design substantially conforms to the project design approval, and comments on details and landscaping.

WHEN IS A COMPLETED CHECKLIST REQUIRED?

A completed **Final Approval Submittal Checklist** is required when you submit for final approval. To resubmit an application, upload documents, like plans and letters, into the record in the City's Accela Citizen Access Portal (ACA) system, along with the <u>Resubmittal Form</u>. All forms must be completed, signed, and submitted as a PDF attachment to your electronic submittal.

FINAL APPROVAL CHECKLIST

Provide required details and sheet references with your submittal for final approval. Fill in the blank or indicate N/A if "not applicable". Final approval does not permit the omission of any required information.

PROJ	JECT ADDRESS: 3427 C	AMPAHIL	Pr	PLN RECORD ID: 1024-	00274
ALL	BUILDING ELEVATIONS	Sheet #			Sheet #
2	Exterior Details	A60-10-6		Paint or Stain Color (trim, etc.)	40,4 EAG. 0
W	Exterior Finishes	A0,48 A4.0	W	Materials (roofing, plaster, etc.)	11 11
V	Parapet Heights	AU108AS-0		Exterior Lighting (incl. cut sheets)	NA
	Roof/Attic/Understory Vents	NA		Specification Sheets, as applicable	H/A
CON	ISTRUCTION DETAILS	Sheet #			Sheet #
V	Retaining Wall	04.0		Ironwork	A.6.2
	Window/Door detail	Aleil		Stairs	A6,28
	Roof Details (eaves)	24.0		Handrails	A6,2
	Decks	43.0, A5 £ 6,8		Skylights	NA
	Fences/Arbors/Trellis	NA		Awnings	NA
	Trash/Recycling Enclosures	NA		Gutters and Down Spouts	A3,0
ELEC	CTRICAL/MECHANICAL/PLUM	MBING EQUIPMI	ENT		Sheet #
	Transformer Vault				NA
	Utility Service Meter				EZ
	Screening Elements				Alil
	Generators/Electrical/Mechanical/	HVAC (including cut	t shee	ets & dBA at property lines)	A1.)
	Fire Valves (Verify Fire Sprinkler C	Ordinance per SBM0	S8.0	04 requirements)	H/A
	Cross Connection Control Devices	(backflow device)			H/A
CON	SULTANT/ENGINEER SHEET	S Sheet #		, 1 - 1 - 1	Sheet #
	Electrical	E1-E3		Structural	51.1-754
U	Mechanical	ML1-M2.)		Plumbing	PILEPIZ

ROOFTOP ARCHITECTURAL DET	TAILS			Sheet #			
HVAC Equipment (exhaust fai	HVAC Equipment (exhaust fans, condensing units, air conditioning units, etc.)						
Dimensions of equipment and	Alil						
☐ Mission tile roofing installation	Mission tile roofing installation specifications						
☐ Specification Sheets, if applica	able			N/A			
Parapet Height				A9.0			
Screens				N/A			
Chimney Caps				A6,0			
Flashing				A6,0			
Gutters/ Scuppers				A6.0			
Solar panel location or potenti	al future solar pa	nel inst	allation (if applicable)	A3,0			
High fire roof coverings, valley	s, gutters			A600			
Paint and Stain Color Names Material Type, Brand and Inve	and Numbers			Sheet #			
LANDSCAPE PLAN	Sheet #			Sheet #			
☐ Irrigation Plan	N/A		High Fire/Defensible Space	NA			
☐ Plant Species/Number/Sizes	HA		Water Conservation Standards	NA			
☐ Planters, Pots, Furniture	NA		Site Walls (materials and color)	C4.08 Act. 0			
Paving Materials	120		Backflow Device	N/A			
Erosion Control Measures	03,0		Rooftop Garden/Landscaped Roof	H/A			
Storm Water Management Pro	gram (SWMP)			Sheet #			
Location of filtration devices				02,0			
Cross-section details				CitiO			
Drainage flow from all impervi	ous areas			1.50			
Amounts of new, replaced, or	가는 >/ :						
Hydrology/Storm Water Repor	1.50						