

F.A.R. Calculator

Instructions: Enter the information in the white boxes below. The spreadsheet will calculate the proposed FAR (floor area ratio), the 100% max FAR (per the Zoning Ordinance for "Required FAR"), and the 80% max FAR (per the Zoning Ordinance for "Required FAR"). Additionally, it will determine whether a FAR Modification is required. "Guideline FAR" calculations are as outlined in the "Applicability" section of the Single Family Residence Design Guidelines, page 23-C.

The **Net Lot Area** does not include any Public Road Easements or Public Road Right-of-Way areas. The proposed **TOTAL Net FAR Floor Area** shall include the net floor area of all stories of all building, but may or may not include basement/cellar floor area. For further clarification on these definitions please refer to SBMC 328-15.083 & 30.300. This form has not yet been updated for current Title 30 zone designations, see SBMC 330.05.010 for comparison.

ENTER Project Address:	3627 CAMPANIL DRIVE
Is there a basement or cellar existing or proposed?	No
ENTER Proposed TOTAL Net FAR Floor Area (in sq. ft.):	4,022
ENTER Zone ONLY from drop-down list:	A-1 or RS-1A
ENTER Net Lot Area (in sq. ft.):	57,499
Is the height of existing or proposed buildings 17 feet or greater?	No
Are existing or proposed buildings two stories or greater?	No
The FAR Requirements are:	GUIDELINE**
ENTER Average Slope of Lot:	19.00%
Does the height of existing or proposed buildings exceed 25 feet?	No
Is the site in the Hillside Design District?	Yes
Does the project include 500 or more cu. yds. of grading outside the main building footprint?	No
An FAR MOD is not required per SBMC 328.15 or 30.20.030	

FLOOR AREA RATIO (FAR):	0.070
Lot Size Range:	>= 20,000 sq. ft.
MAX FAR Calculation (in sq. ft.):	4,430 + (0.013 x lot size in sq. ft.)
100% MAX FAR:	0.090
100% MAX FAR (in sq. ft.):	5,177
85% of MAX FAR (in sq. ft.):	4,401
80% of MAX FAR (in sq. ft.):	4,142
The 4022 square foot proposed total is 78% of the MAX FAR.*	

* NOTE: Percentage total is rounded up.
 **NOTE: If your project is located on a site with multiple or overlay zones, please contact Planning Staff to confirm whether the FAR limitations are "Required" or "Guideline".

Acreage Conversion Calculator	
ENTER Acreage to Convert to square footage:	1.32
Net Lot Area (in sq. ft.):	57499.2

CONSULTANTS :

- ARCHITECT:** MISSION GROUP ARCHITECTS
 RICHARD E. JOHNSON ARCHITECT, AIA
 1230-1 COAST VILLAGE CIRCLE
 SANTA BARBARA, CA. 93108
 (805) 969-5910
 rejmgsab@gmail.com
- STRUCTURAL ENGINEER:** KEVIN L. VANDERVORT STRUCTURAL ENGINEERING
 250 STORKE ROAD SUITE 12
 GOLETA, CA. 93117
 (805) 562-8462
 kevin@klvse.com
- CIVIL ENGINEER:** MIKE GONES C.E. RCE #38168
 1219 1/2 LAGUNA STREET
 SANTA BARBARA, CA. 93101
 (805) 966-2259
 mikegonesce@outlook.com
- STORM WATER MANAGEMENT:** MIKE GONES C.E. RCE #38168
 1219 1/2 LAGUNA STREET
 SANTA BARBARA, CA. 93101
 (805) 966-2259
 mikegonesce@outlook.com
- ELECTRICAL ENGINEERING:** JMPE ELECTRICAL ENGINEERING
 JOHN MALONEY CA. REG. E13083
 627 OLIVE STREET
 SANTA BARBARA, CA. 93105
 (805) 569-9216
 MALONEY@JMPE.NET
- MECHANICAL ENGINEERING:** MECHANICAL ENGINEERING CONSULTANTS, INC.
 ENERGY/TITLE-24: PAUL TZOUVARAS
 315 E. CANON PERDIDO STREET SUITE "B"
 SANTA BARBARA, CA. 93101
 (805) 957-4632
 PAUL@MECENG.COM
- SOILS INVESTIGATIONS:** PACIFIC MATERIALS LABORATORY
 RON PIKE
 35-A SOUTH LA PATERA LANE
 GOLETA, CA. 93116
 (805) 964-6901
 PML@PML.SBCOMMAIL.COM
 SEE SHEET A-0.4 FOR SOILS RECOMENDATIONS
 LAB #: 115040-2
 FILE #: 16-11153-2

SHEET INDEX	DATE	REVISION
A-0	PROJECT INFORMATION	REV. 9-14-24
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A-0.4	PHOTOS & COLOR BOARD	REV. 9-14-24
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S3	ROOF FRAMING PLAN	
S4	STRUCTURAL DETAILS	

PROJECT INFORMATION:

OWNERS: DR. PAULA KISLAK
 545 HODGES LANE
 SANTA BARBARA, CALIFORNIA 93108
 PK99@COX.NET

PROJECT ADDRESS: 3627 CAMPANIL DRIVE
 SANTA BARBARA, CALIFORNIA 93109

PROPERTY USE: EXISTING: 1 SINGLE FAMILY RESIDENCE
 PROPOSED: SINGLE FAMILY w/ ADU

APN: 047-101-003

LOT SIZE: 61,561 SF
 1.32 ACRES

ZONING: RS-1A (SBMC TITLE 30)

GENERAL PLAN: LOW DENSITY RESIDENTIAL (MAX. 1 DU/AC.)

SPECIAL DESIGN DISTRICT: HILLSIDE

SETBACKS: FRONT: 35'-0" INTERIOR: 15'-0" (HOA 25'-0") ADU: 4'-0"

AVG. PARCEL SLOPE: 21% APPROX. PER CITY RECORDS

FEMA FLOOD ZONE: ZONE X

HEIGHT LIMIT: 30'-0" FROM LOWEST NATURAL OR FINISHED GRADE
 16'-0" FOR ACCESSORY DWELLING UNIT

HIGH FIRE: YES - COASTAL INLAND

FIRE SPRINKLERS: YES

OCCUPANCY: GROUP R-3 (RESIDENCE)

TYPE OF CONSTRUCTION: V-B

PARKING REQUIREMENTS: EXISTING PARKING - PRIMARY RES.: COVERED 3 / 1 UNCOVERED
 PROPOSED PARKING - PRIMARY RES.: COVERED 3 / 0 UNCOVERED
 REQUIRED PARKING - PRIMARY RES.: 2 COVERED STALLS
 EXISTING PARKING - ADU: N/A
 PROPOSED PARKING - ADU: PROPOSED 0 COVERED / 1 UNCOVERED
 REQUIRED PARKING - ADU: 1 PURSUANT TO SBMC 30.185.040 SINCE PROJECT DOES NOT COMPLY WITH EXEMPTIONS

STORM WATER SUMMARY:

WATER QUALITY PER TIER 2 (STORAGE NOT REQUIRED) REQUIRED BY CITY OF SANTA BARBARA WILL BOTH BE ACCOMPLISHED BY INSTALLATION OF CISTERN STORAGE SYSTEM. THIS SYSTEM ASSURES NO INCREASE IN RUNOFF FROM THE PROPOSED DEVELOPMENT FOR STORM RUNOFF UP TO THE 25 YEAR STORM. OVERFLOW FOR STORMS GREATER THAN THIS WILL HONOR THE EXISTING HISTORICAL RUNOFF PATTERNS OF USING THE EXISTING NATURAL WATERSHED.

PROPOSED NEW IMPERVIOUS AREA: 1539 SF
 PROPOSED REPLACED/REDEVELOPED IMPERVIOUS AREA: 0 SF
 PROPOSED REMOVED IMPERVIOUS AREA: 264 SF
 EXISTING IMPERVIOUS AREA TO REMAIN: 7031 SF
 SELF-TREATING WALKS: 367 SF

TOTAL NEW IMPERVIOUS AREA: 1906 SF
 SEE CIVIL PLANS ON SHEET C2.1 FOR CALCULATIONS & DETAILS

SCOPE OF WORK:

THIS PROPOSAL IS FOR CONSTRUCTION OF A 1200 SF ACCESSORY DWELLING UNIT IN THE CITY OF SANTA BARBARA PURSUANT TO SBMC 30.185.040. ADDITIONALLY, A 184 SF ROOF DECK AND ACCESS STAIRS ARE PROPOSED, ALONG WITH 182 LINEAR FT. OF SITE/RETAINING WALLS ARE PROPOSED. THERE IS AN EXISTING 1 STORY PRIMARY RESIDENCE OF 2,195 SF WITH ATTACHED GARAGE OF 627 SF ON THE PROPERTY. THE PROJECT WILL REQUIRE 153 CY OF GRADING, 1 PARKING SPACE IS PROPOSED ON EXISTING AC DRIVEWAY. THE EXISTING SEWER LATERAL IS PROPOSED TO BE REPLACED IN ITS ENTIRETY.

REV. 1 to BLD2022-03023 - SCOPE OF WORK:

THIS PROPOSAL IS FOR BUILDING COLORS CHANGE FROM PERMITTED COLORS THAT CURRENTLY MATCH EXISTING PRIMARY RESIDENCE TO NEW COLORS THAT WILL MATCH PENDING NEW PRIMARY RESIDENCE FINISH COLORS WHICH IS CURRENTLY UNDER REVIEW (PLN2023-00507) PER SBMC EXCEPTION 30.185.040.H.11.A. ADDITIONALLY, CHANGE PROPOSED WATER HEATER TO A TANKLESS MODEL.

BUILDING TABULATIONS:

	NET	GROSS
3726 CAMPANIL DRIVE		
EXISTING PRIMARY RES. FLOOR AREA:	2,195 SF NET	2,300 SF GROSS
EXISTING PRIMARY RES. GARAGE AREA:	627 SF NET	677 SF GROSS
EXISTING TOTAL FLOOR AREA:	2,822 SF NET	2,977 SF GROSS

ACCESSORY BUILDING CUMULATIVE ALLOWANCE:

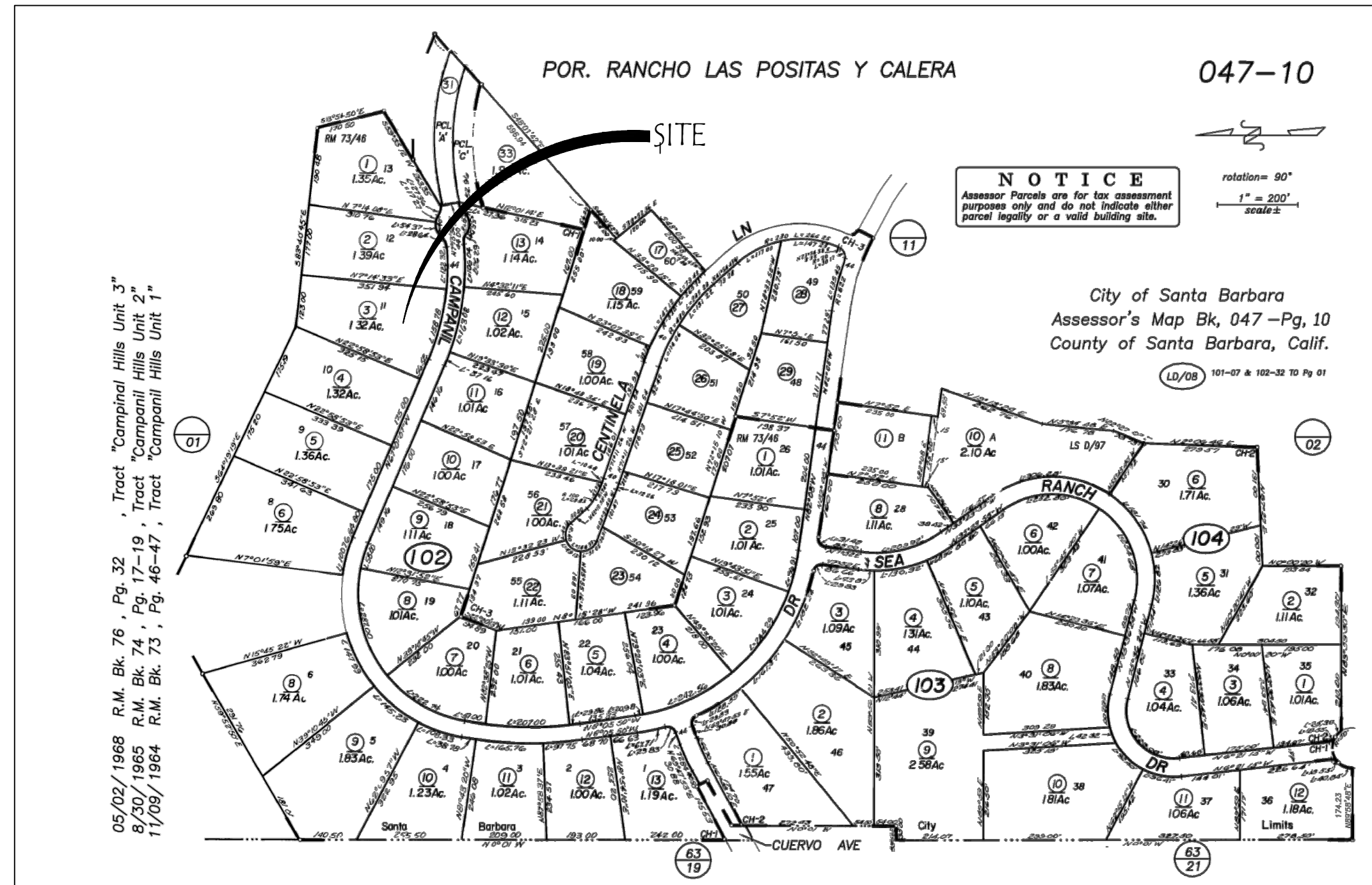
PROPOSED ADU FLOOR AREA:	1,200 SF NET	1,299 SF GROSS
EXISTING GARAGE:	627 SF NET	677 SF GROSS
ACCESSORY STRUCTURES:	NONE	NONE
TOTALS: (1,950 SF ALLOWED)	1,827 SF NET	1,976 SF GROSS
		PROJECT COMPLIES WITH SBMC 30.140.020

GRADING QUANTITIES:

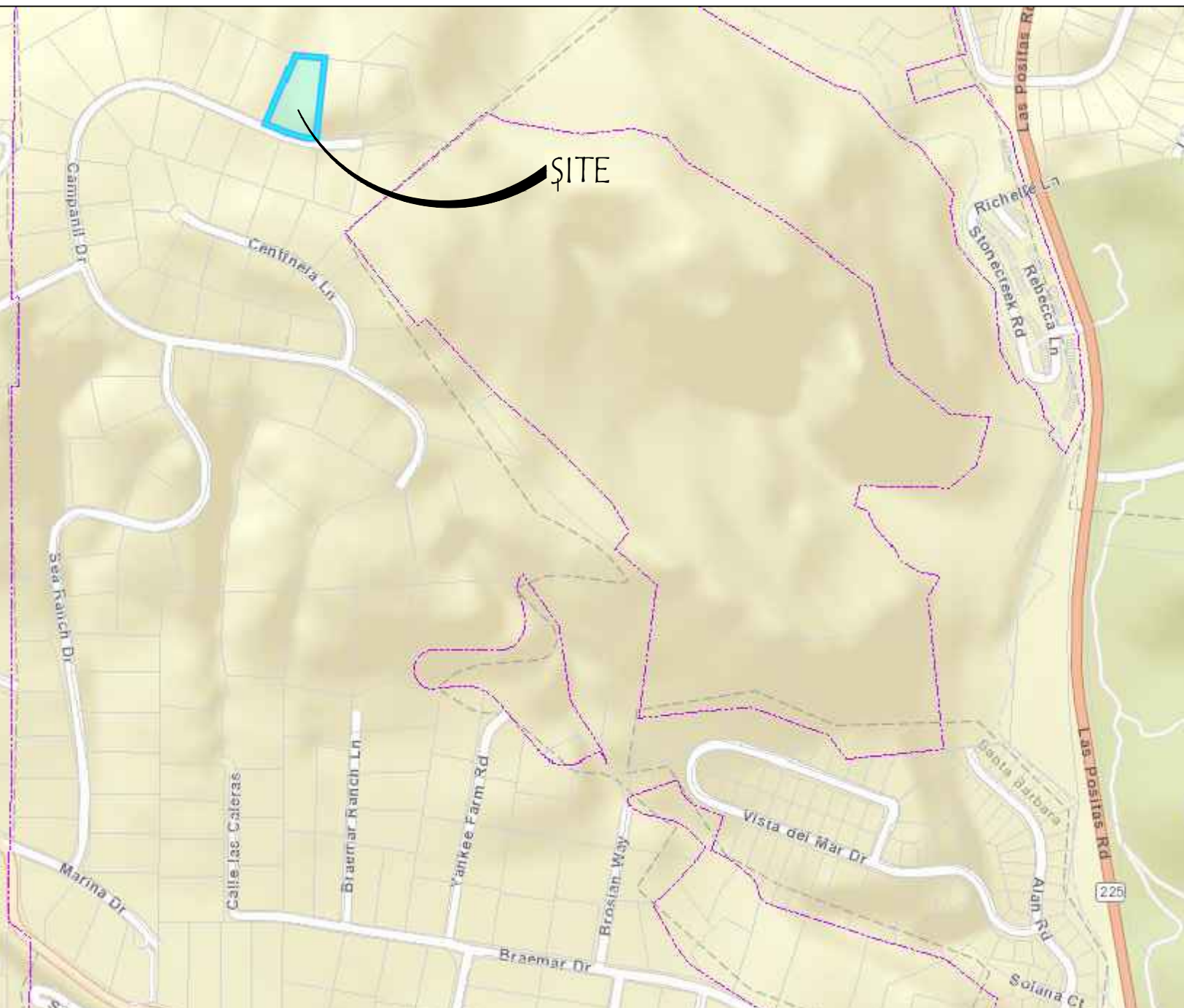
TOTALS
CUT: 100 CY
FILL: 53 CY
EXPORT: 0 CY
IMPORT: 0 CY
TOTALS: 153 CY (SEE CIVIL SHEETS)

PUBLIC WORKS NOTE:

FOR ANY WORK IN THE PUBLIC RIGHT OF WAY OR CITY SEWER EASEMENT A SEPARATE PERMIT THROUGH THE PUBLIC WORKS DEPT. WILL BE REQUIRED.



ASSESSOR PARCEL MAP



VICINITY MAP

CODE COMPLIANCE:

- ALL WORK SHALL COMPLY WITH ALL OF THE FOLLOWING:
 - 2019 CALIFORNIA RESIDENTIAL CODE
 - 2019 CALIFORNIA MECHANICAL CODE
 - 2019 CALIFORNIA ELECTRICAL CODE
 - 2019 CALIFORNIA FIRE CODE
 - 2019 CALIFORNIA ENERGY CODE (T-24)
 - 2019 CALIFORNIA PLUMBING CODE
 - 2019 CALIFORNIA GREEN BUILDING STANDARDS CODE
 - 2019 CALIFORNIA BUILDING CODE
 - CITY OF SANTA BARBARA MUNICIPAL CODE AND ADOPTED ORDINANCES #5919
 - CRS SECTION R327 MATERIALS & CONSTRUCTION METHOD FOR EXT. WILDFIRE EXPOSURE.

GENERAL NOTES:

- PRIOR TO START OF CONSTRUCTION, THE CONTRACTOR SHALL SCHEDULE A PRECONSTRUCTION CONFERENCE WITH ALL PARTIES INVOLVED AT THE PROJECT SITE TO REVIEW THE SPECIAL INSPECTION REQUIREMENTS, PROCEDURES, AND INDIVIDUAL SPECIAL INSPECTORS THAT WILL BE ASSIGNED TO THE PROJECT, AS WELL AS REQUIREMENTS FOR STRUCTURAL OBSERVATION. CONTRACTOR SHALL CONTACT THE CITY OF SANTA BARBARA BUILDING DIVISION TO CONFIRM AN ACCEPTABLE MEETING DATE AND TIME.
- NO NATURAL GAS INFRASTRUCTURE TO SERVE ACCESSORY DWELLING UNIT PER SECTION 22.110 OF THE CITY OF SANTA BARBARA MUNICIPAL CODE.
- THE NFRC THERMAL PERFORMANCE LABELS SHALL REMAIN ON THE WINDOWS AND/OR DOORS UNTIL FINAL INSPECTION.
- SOLAR PANELS AT ROOF DECK WILL BE UNDER A SEPARATE PERMIT.

PROPOSED ACCESSORY DWELLING UNIT FOR: KISLAK RESIDENCE @ 3627 CAMPANIL DRIVE

SANTA BARBARA CALIFORNIA 93109

REVISIONS
 REV. 1 9/14/2024
 EXT. COLORS
 WTR. HTR.

MISSION GROUP ARCHITECTS
MISSION GROUP ARCHITECTS
 RICHARD E. JOHNSON, AIA

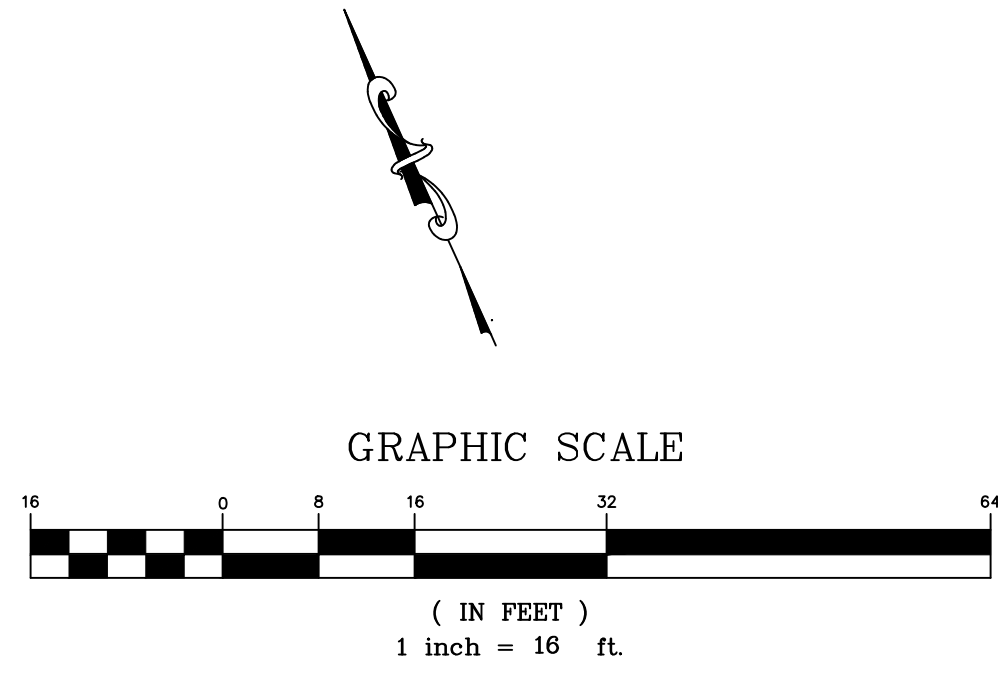
PROJECT INFORMATION
KISLAK RESIDENCE
 3627 CAMPANIL DRIVE
 SANTA BARBARA, CALIFORNIA 93109

PROPOSED ADU FOR:
KISLAK RESIDENCE
 3627 CAMPANIL DRIVE
 SANTA BARBARA, CALIFORNIA 93109

DATE
 09/14/2024

SHEET
A-0

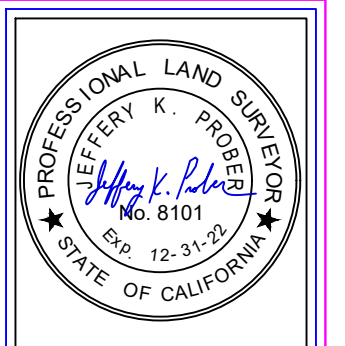
1230 COAST VILLAGE CIRCLE - STE H - SANTA BARBARA, CALIFORNIA 93108 - 805-969-5910



LEGEND

AC	ASPHALT CONCRETE
AD	AREA DRAIN
BLD	BUILDING
CIT	CITRUS TREE
COL	COLUMN
CP	CONTROL POINT (survey)
DL	DRIFLINE (oak tree)
DO	DRAIN OUTLET (pipe)
DW	DRIVEWAY
E	EASTERLY
EM	ELECTRIC METER
EP	EDGE OF PAVEMENT
FD	FOUND (survey monument)
FF	FINISH FLOOR
FL	FLOWLINE
FN	FENCE
FOB	FACE OF BUILDING
G	GRADE
GAR	GARAGE
GM	GAS METER
H	HEDGE
LP	LAMP POLE
N	NORTHERLY
PEP	PEPPER TREE
PL	PARCEL LINE
P.U.E.	PUBLIC UTILITY EASEMENT
S	SOUTHERLY
SM	SMALL
SMH	SEWER MANHOLE
T	TOP of *
TC	TOP OF CURB
TCN	TOP OF CONCRETE
TEL	TELEPHONE EQUIP.
TH	THRESHOLD
TOE	TOE OF SLOPE
TS	TOP OF SLOPE or
TS	TOP OF STEP(S)
TV P E	T.V., TELEPHONE, & ELECTRIC (underground utility lines)
TW	TOP OF WALL
W	WESTERLY
WL	WALL
WM	WATER METER
WV	WATER VALVE
WWL	WING WALL

Logo\LogoSml1743104.jpg
 645 Flora Vista Drive, SB, CA 95109
 (805) 452-9600 Fax: 944-2415 plsb@com.net
 www.ProberLandSurveying.com



3627 Campanil Drive
 Santa Barbara, CA
 Topographic Survey

Surveyor's Notes:
 1. BOUNDARY DATA: CAMPANIL HILLS TR. BK 73 PG. 46 OF MAPS (R1)
 2. HORIZONTAL DATUM: NAD83; COORD. SYSTEM: SPC CA 05 4FT. EPOCH 2022.75
 3. VERTICAL DATUM: NAVD83; INITIAL STA. SMARTNET 3229
 4. ALL DISTANCES SHOWN ARE UNLESS OTHERWISE SPECIFIED IN FEET AND INCHES
 5. A CURRENT TITLE REPORT IS REQUIRED TO VERIFY AND PLOT ALL EASEMENTS AFFECTING SUBJECT LOT.
 6. BASIS OF BOUNDARY (RECORD BOUNDARY ROTATED TO GRID); FRONT LOT CORNERS (P. 50 - P. 56 SHOWN HEREON)
 7. ORTHOMOSAIC IMAGE: PHANTOM 4 PRO DRONE - PIXID MAPPER

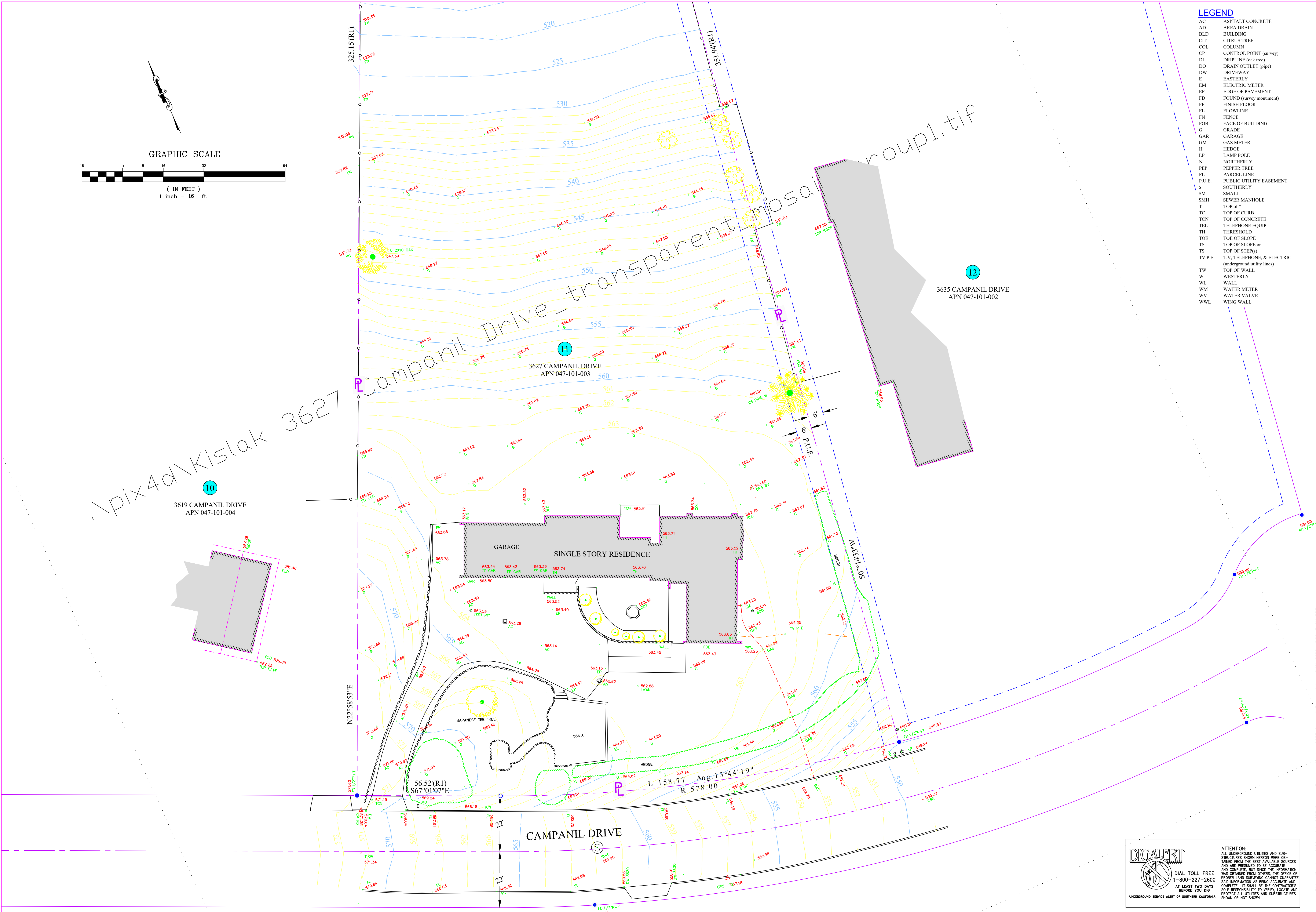
Drawn By	JKP
Field Work Performed	3-30-2022
Scale	1 inch = 16 ft.

A0.1

DIAL ALERT
 ALL UNDERGROUND UTILITIES AND SUB-STRUCTURES SHOWN HEREON WERE OBTAINED FROM THE BEST AVAILABLE SOURCES AND ARE PRECISED TO BE ACCURATE AND COMPLETE. BUT SINCE THE INFORMATION WAS OBTAINED FROM OTHERS, THE OFFICE OF PROBER LAND SURVEYING CANNOT GUARANTEE SAID INFORMATION AS BEING ACCURATE AND COMPLETE. IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO VERIFY, LOCATE AND PROTECT ALL UTILITIES AND SUBSTRUCTURES SHOWN OR NOT SHOWN.

DIAL TOLL FREE
 1-800-227-2600
 AT LEAST TWO DAYS BEFORE YOU DIG

UNDERGROUND SERVICE ALERT OF SOUTHERN CALIFORNIA



G:\Land\Project_2022\Kislak_3627_Campanil_Drive\Map\Kislak_3627_Campanil_Drive_v4.dwg 3/31/2022 9:39:47 PM PJT



SOUTH SIDE EXISTING RESIDENCE TO BE DEMOLISHED



NORTH SIDE EXISTING RESIDENCE TO BE DEMOLISHED

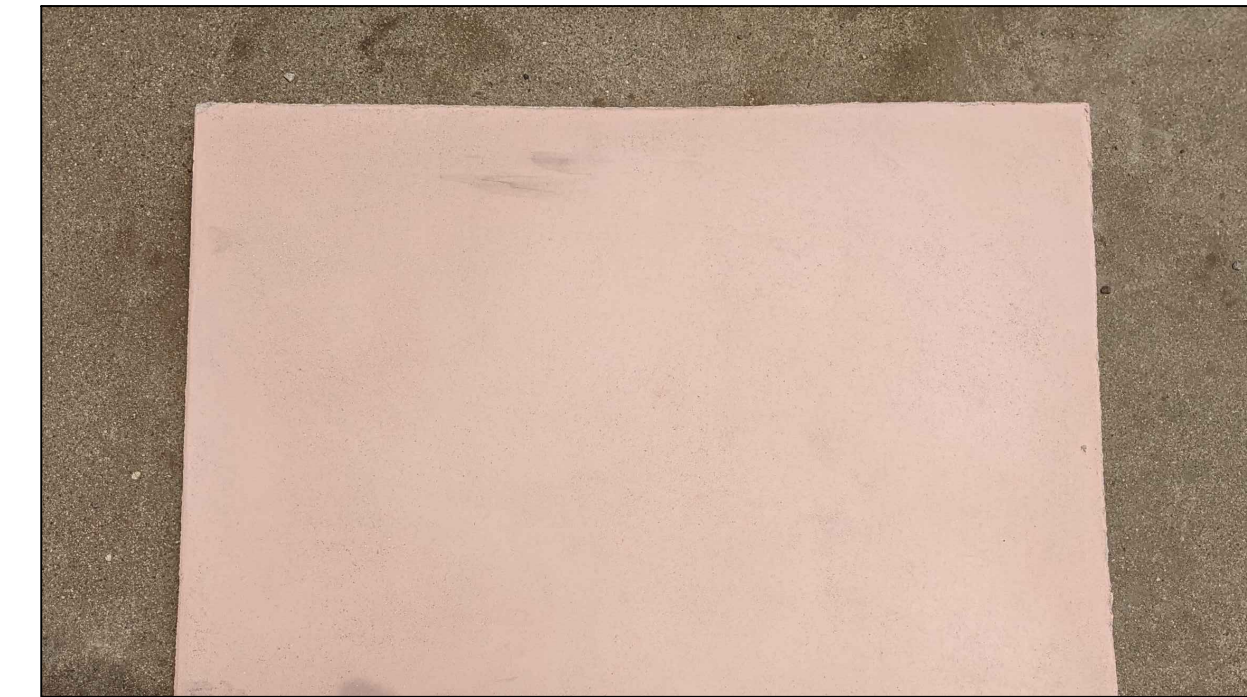


CONCEPTUAL RENDERING OF PENDING PROPOSED RESIDENCE (PLN2023-00507)

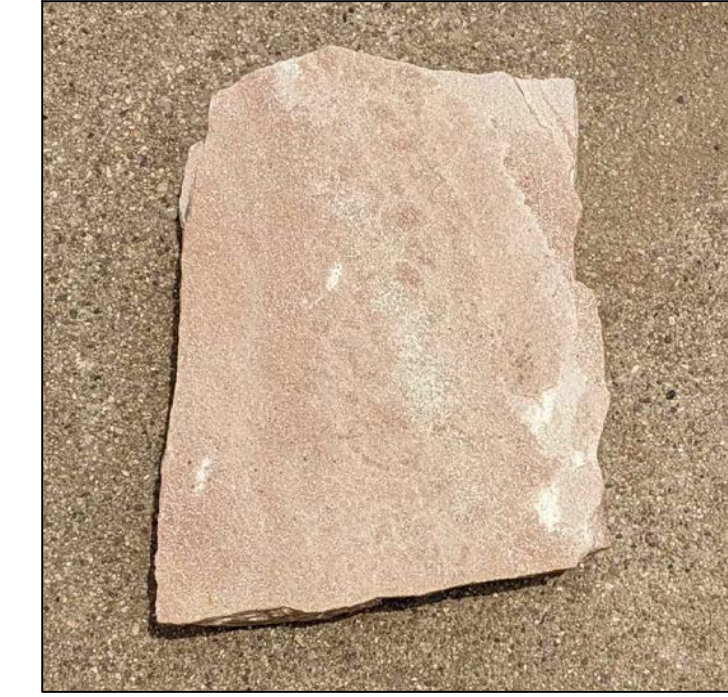
MATERIALS LEGEND:

NOTE: VERIFY ALL COLORS AND MATERIALS WITH OWNER PRIOR TO ORDERING

- ① ROOFING: TWO PIECE MISSION TILE BY REDLANDS CLAY TILE
UL CLASS "A" FIRE RATING
ICC: ESR-4395
COLOR - 3 TILE CUSTOM BLEND
60% LIGHT TILE, 30% & 30% DARK TILES
- ② EXTERIOR PLASTER: 7/8" SMOOTH TROWEL EXTERIOR PLASTER
OVER MTL. LATH. & TYVEK HOMEWRAP TYPICAL
COLOR: CUSTOM COLOR AS SHOWN (SOFT PINK)
- ③ STONE VENEER: BEIGE TRAVERTINE VENEER OVER SCRATCH COAT/GAL. LATH
OVER TYVEK WATERPROOF BARRIER OVER PLYWD.
FLOORING TO BE CHEROKEE SANDSTONE
- ④ CARVED STONE TRIM: BEIGE TRAVERTINE VENEER (SEE ABOVE)
- ⑤ DOORS & WDWS.: STEEL WINDOWS & DOORS BY MIRAMAR MFG. INC
ALL GLASS TO MEET CRITERIA SET FORTH IN ENERGY CALC.
DUAL GLAZED W/ MAX. U-FACTOR OF .35
& MAX. SHGC OF .35
COLOR: BLACK
- ⑥ GUTTERS & FLASHING: 20 GA. MIN. COPPER GUTTERS AND FLASHING
NEW GUTTERS TO HAVE DEBRIS GUARD
- ⑦ CUSTOM WROUGHT IRON: DESIGN TO BE DETERMINED
DESIGN SHALL COMPLY WITH [CRC SECTION R312]
SEE DETAIL 2/A6.2 FOR GENERAL NOTES
- ⑧ STACKED ALLANBLOCK: AB EUROPA COLLECTION



2 EXT. PLASTER



3 PAVING



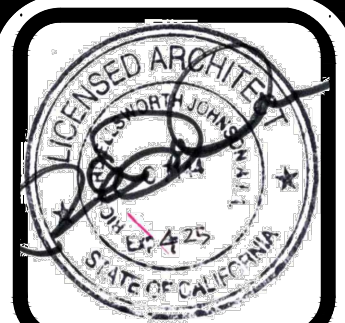
1 ROOF TILE



3 VENEER & TRIM



OVERALL PROPOSED MATERIALS BOARD



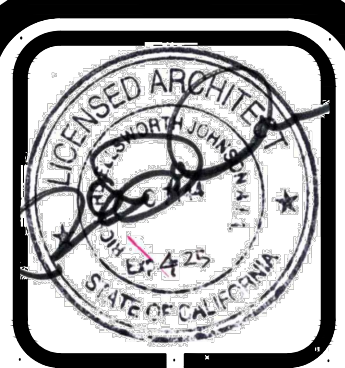
MISSION GROUP ARCHITECTS
R. E. JOHNSON
AIA

PHOTOS AND COLOR BOARD

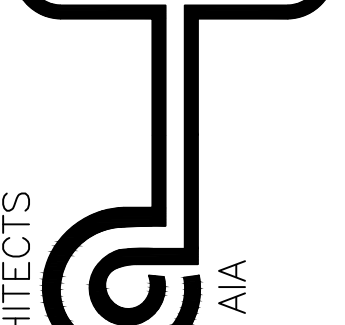
PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

DATE
09/14/2024

SHEET
AO.4



REVISIONS



MISSION GROUP ARCHITECTS
R. E. JOHNSON
AIA

SOILS RECOMMENDATIONS

PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

DATE
07/31/2024

SHEET
AO.4a

1230 COAST VILLAGE CIRCLE, STE 108 - SANTA BARBARA, CALIFORNIA 93108 - 805-969-5910

1230 COAST VILLAGE CIRCLE, STE 108 - SANTA BARBARA, CALIFORNIA 93108 - 805-969-5910

Pacific Materials Laboratory
of Santa Barbara, Inc.
35-A South La Patena Lane
P.O. Box 96, Goleta, CA 93116
Phone: (805) 964-6901
FAX No.: (805) 964-2239
E-mail: pml@pml.sbcocmail.com

April 6, 2023
Lab No: 140385-2
File No: 23-11153-2

Paula Kislak
545 Hodges Lane
Santa Barbara, CA 93108

SUBJECT: Grading and Foundation Plan Review
Proposed Accessory Dwelling Unit (ADU)
3627 Campanil Drive
Santa Barbara, California

REFERENCE: This Laboratory's Preliminary Geotechnical Investigation Report
Lab No. 115040-2, Dated December 22, 2017
This Laboratory's Preliminary Geotechnical Investigation Update Report
Lab No. 137805-2, Dated May 3, 2022

Dear Ms. Kislak:

In accordance with your request, I have reviewed the grading and foundation plan for the proposed ADU. Based upon my review, it is my opinion the grading and foundation plan is in substantial conformance with the recommendations of our geotechnical report referenced above.

This review is not comprehensive and does not include a review of the architect's and/or civil and structural engineers' design and calculations.

The observation of excavations during the construction phase represents an opportunity by our firm either to confirm soil conditions estimated by the exploratory borings, or to discover soil conditions that have not been addressed. When such undisclosed conditions are encountered, opinions and recommendations addressing these conditions will be rendered at that time.

The reference report is considered preliminary and no person should consider the recommendations or soil conditions described therein as conclusive. The recommendations and conclusions contained in the reference report are considered preliminary until all excavations have been observed during the construction phase, after which a final report will be issued at your request stating that the grading and foundation works accomplished and installed are appropriate for the soil conditions encountered.

If you have any questions concerning this report, please do not hesitate to call. Thank you for the opportunity of providing this service.

Respectfully submitted,
PACIFIC MATERIALS LABORATORY, INC.
Ronald J. Pike
Ronald J. Pike
Geotechnical Engineer, G. E. 2291

RJP:lcc
cc: Paula Kislak, Email: pk99@cox.net
"We Test The Earth"

December 22, 2016 -7- Lab No: 115040-2
File No: 16-11153-2

6. The bottom of the pile excavation shall be cleaned of debris, using a pancake auger for the last pass, in order to provide end-bearing contact.

7. This Laboratory shall be requested to inspect all excavations prior to steel and concrete placement.

8. Concrete pool decks will move differentially with respect to the pool structure. This may be due to the difference in support elevations. A flexible deck performs best and hides the differential movement. An example of a flexible deck is individual stone pavers with grass growing between the joints. It is recommended that a flexible surrounding deck be incorporated into the design.

9. The walls of the swimming pool shall be designed to resist a lateral earth load of 100 pcf.

10. The walls of the pool shall be designed as freestanding walls deriving no lateral support from the adjacent soil.

11. The owner or his agent shall request the Project Geotechnical Engineer to also observe all excavations prior to placement of compacted soil, or rebar and shotcrete.

12. We request the foundation plan be submitted to our office for a general review to verify substantial compliance to the recommendations contained in this report.

ADJACENT LOADS

Where footings are placed at varying elevations, the effect of adjacent loads may be calculated using the widely published Formulas for Stresses in Semi-infinite Elastic Foundations or the Boussinesq figures and equations for both vertical and horizontal surcharge loads.

APPURTENANCES

The expansive soil encountered at the site is identified as the most challenging aspect of the foundation design. The Rincon Formation is prone to instabilities, such as surficial slope failures, mudflows, creep, expansion, and shrinkage. Perfect performance of appurtenant improvements, as well as the old and new footings of the house, is an unacceptable level of expectation on the part of the property owner or future owners. Associated features, such as patios, walkways, trellis columns, pool equipment enclosure, and driveways will be subject to movement due to the expansion and shrinkage of the clay surface soils. The foundation design is intended to support the structure in a safe manner and with an acceptable risk of movement that must be accepted by the owner.

An economical way of reducing the anticipated movement of the appurtenant improvements, such as walkways and patios, is by moisture control of the supporting soils and by carefully choosing the type of building materials used to construct these facilities.

December 22, 2016 Pacific Materials Laboratory of Santa Barbara, Inc. Lab No: 115040-2
File No: 16-11153-2

features. Moisture control can be approached by installing surface storm drain collection systems, controlling surface water, and the proper placement of planting areas around the foundation system. Water entering the subsurface soils can be reduced by the placement of a false bottom below planters or an impervious membrane, such as visqueen beneath decorative rock, patios, or paths. Controlling surface water and directing it away from slopes is critical. However, even with correct drainage, the moisture content of the soil will change from summer to winter and, therefore, the soils will shrink and swell, moving any item supported over the soil. Drainage can serve to reduce the rate of movement.

With respect to construction materials for the appurtenant improvements, redwood decks are more flexible than concrete patios and do not reflect as much damage from soil movement. If concrete flatwork is required, there are at least two alternative approaches. This method, however, holds one end of the flatwork fixed while the other is free to move, thus cracking the concrete. For either method, the owner can expect movement and, therefore, the exterior flatwork may experience more movement than the foundation. By leaving the two disconnected, the exterior flatwork is allowed to float. The problem, however, is that the flatwork may tend to float away from the house, creating an uneven gap distance between the foundation and the edge of the concrete slab or creating a trip edge at joint entrances.

The other alternative is to dowel the flatwork into the exterior foundation with steel rebar to prevent the differential movement and to prevent the gap and trip edge from occurring. This method, however, holds one end of the flatwork fixed while the other is free to move, thus cracking the concrete. For either method, the owner can expect movement and, therefore, the exterior flatwork may experience more movement than the foundation. By leaving the two disconnected, the exterior flatwork is allowed to float. The problem, however, is that the flatwork may tend to float away from the house, creating an uneven gap distance between the foundation and the edge of the concrete slab or creating a trip edge at joint entrances.

SETTLEMENT

It is the intent of the recommendations contained in this report to achieve angular distortions of approximately 1/480. A total settlement of approximately 1 inch or less is anticipated in the undisturbed, native soil and approximately 1% to 1.5% of the fill height is the anticipated total settlement at areas where compacted fill soil is placed in accordance with the grading recommendations provided in this soil engineering report. Movement from expansive soil has already been discussed in this report under the heading APPURTENANCES. The soil bearing values and estimated settlements contained in this report are preliminary and may need to be modified after the foundation and grading plans are substantially complete.

¹ Angular distortion is the ratio of the vertical differential settlement divided by the horizontal distance over which the vertical differential is measured.

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CONSTRUCTION OBSERVATION

The owner or his agent shall request the Project Geotechnical Engineer to observe all excavations prior to placement of compacted soil, gravel backfill, or rebar and concrete.

PLAN REVIEW

We request the grading and foundation plans be submitted to our office for a general review to verify substantial compliance to the recommendations contained in this report.

CLOSURE

The recommendations contained herein are for the sole use of our client and are based upon this Laboratory's understanding of the project which has been described herein. If the project scope, location, or conceptual design is subsequently altered, this Laboratory shall be requested to modify, as necessary, the recommendations contained herein as is appropriate for the new development concept. If the recommendations of this report are not implemented within one year, we recommend an update and review of the contents of this report be performed by this Laboratory.

The recommendations contained herein are based upon the assumption that Pacific Materials Laboratory shall be requested to perform the testing and observation services which will be required during the grading and foundation operations in order to verify that the actual soil conditions encountered and the construction procedures are consistent with the recommendations contained herein. If this service is performed by others, only the technical correctness of the actual analytical tests described here is attested to by this Laboratory.

Thank you for the opportunity of providing this service. If you have any questions regarding this matter, please do not hesitate to call.

Respectfully submitted,
PACIFIC MATERIALS LABORATORY, INC.
Ronald J. Pike
Ronald J. Pike
Geotechnical Engineer, G. E. 2291

RJP:vjh
cc: Addressee (3)
Clause Construction, Attn: Dan Clause, Email: dan@clauseconstruction.com
JT Engineering & Associates, Inc., Attn: Jason Tani, Email: jtengr@gmail.com
Thomas V. Smith Architect, Email: thomas.v.smith@netscape.com

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provided in this report for the proposed additions to the house because it is assumed the owners will not want to go to the expense of using a pile foundation throughout the old house and the new addition, but rather tolerate the cracks in the surfaces of walls, floors, and ceilings both in the existing and in the proposed structure. If this is not correct and pile recommendations for the house foundation are wanted, please notify our office.

Based upon this understanding, we present the following preliminary recommendations:

FOUNDATIONS

The supporting soils were found to be expansive based on the current CBC. Such soil conditions define the soil as being expansive and require the foundation be designed in compliance with Section 1808.6.2. Based on this understanding, we recommend the following:

1. A mat foundation shall be designed to conform to Section 1808.6.2, using the Wire Reinforcement Institute (WRI) Method. The Project Structural Engineer shall perform the calculation to design the foundation for the addition using the WRI Manual. If the dimensions of the slab, footings, and tie beams are less than the minimum dimensions specified in this section, the larger dimensions shall govern. The same is true for the steel reinforcement.
2. The foundation recommendations which follow are recommended minimums. The actual foundation may exceed these minimums depending on the results of the design using the WRI Method.
3. All continuous exterior footings of the structure shall extend a minimum of 36 inches and all continuous interior footings shall extend a minimum distance of 24 inches below the compacted pad grade.
4. Due to the expansion potential of the soil, the Project Structural Engineer shall utilize Section 1808.6.2 of the CBC for the design of the concrete slab-on-grade floor, foundation, and the interior tie beams using the WRI Method. The effective Plasticity Index (P.I. x C_c x C_u) shall be assumed to be 46. As a minimum, a network of tie beams shall be placed in two directions, perpendicular to each other and at a spacing of approximately 15 feet on center each way. The tie beams shall be a minimum of 20 inches deep below the top of the concrete slab-on-grade floor and shall be a minimum of 12 inches wide, with horizontal rebar placed near the top and near the bottom of the tie beam. The tie beam shall be doweled into the concrete slab-on-grade floor with a minimum of No. 3 rebar at 18 inches on center.
5. All footings shall contain a minimum of two No. 4 horizontal rebar placed one in the base and one in the stem of the footing. The Project Civil or Structural Engineer shall specify the foundation steel details on either the WRI method and/or as required to resist the imposed loads.

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6. Interior isolated spread footings may not be utilized unless connected on 4 sides, where possible, to tie beams. These footings shall extend a minimum of 24 inches below pad grade on the interior and 36 inches below the lowest adjacent ground surface on the exterior.

7. As a minimum, concrete slabs on grade shall be a full 6 inches thick and shall contain No. 3 rebar spaced 18 inches on center each way. The steel reinforcement shall be placed near the center of the slab. The slab shall be underlain with a minimum 4-inch coarse washed concrete sand layer. A 10-mil or heavier impervious membrane shall cover a sand layer. These concrete slab-on-grade requirements shall be modified as needed by the designers for surcharge loads, wheel loads, concentrated loads, or for moisture control. The floor covering supplier or manufacturer should be contacted for their specifications for design features which will result in a successful bond between the concrete slab and floor covering. Floor flatness and shrinkage crack control must be addressed by a competent contractor experienced in the skill of concrete placement. The owners or their agents shall inform those designing, building, and installing the concrete slab on grade and footing of the performance and aesthetics expected.

8. Concrete slabs on grade shall be doweled into all adjacent footings using No. 3 rebar spaced 18 inches on center.
9. Based upon compliance with the above recommendations, an allowable soil bearing value for compacted soil 2,000 psf for 24-inch-deep footings and 2,500 psf for 36-inch-deep footings with a one-third increase when considering wind or seismic forces may be assumed.
10. Floor elevations located lower than the surrounding exterior grades are recommended to be protected from moisture intrusion. Please consult the building designer for details, such as waterproofing and French drains.
11. The Geotechnical Engineer shall be requested to observe the footing excavations after the compaction tests are completed and prior to placement of the rebar reinforcing.

While these recommendations are intended to provide satisfactory foundation performance for a new structure, it should be noted the concrete foundation will be in contact with an expansive soil. The new foundation will be connected to the existing foundation, which is different and inadequate to prevent damage from expansive soil. Consequently, you may expect cracks to appear as a result of the movement. Also, changes over time may alter the foundation's ability to remain level (i.e. broken water lines, droughts, non-symmetrical irrigation practices and altered drainage grades). In addition, associated features, such as porches, patios, driveways, sidewalks, and curbs, will be subject to differential movement. Porches and patios of concrete slab-on-grade construction are recommended to be attached to the perimeter foundation of the proposed structures such that an equivalent elevation is maintained at the joints. All concrete features shall be reinforced with continuous horizontal

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steel and doweled cold joints. The joint between the old and the new structures may also experience differential movement. Where possible, construction joints are recommended for surface crack control. Over the life of the structure, maintenance or replacement of sections of the associated features and appurtenances damaged by the expansive soils may be required.

RESISTANCE TO LATERAL LOADS

Lateral loads may be resisted by frictional resistance along the foundation base and passive earth pressures along the foundation sides. An allowable friction coefficient of 0.35 may be used. The passive pressures of 350 pcf of footing may be used. A triangular distribution should be used. The frictional resistance and the passive pressure may be combined without reduction. The resistance may be increased by one-third for wind or seismic loading.

SWIMMING POOL

The location selected for the proposed swimming pool was found to be on a previously placed fill, approximately 40-feet thick. The fill is underlain by a 10-foot layer of black Rincon clay. This is the original topsoil layer. The Rincon shale was encountered below the black clay at a depth of 50 feet. These soil conditions make the placement of a swimming pool at that location a potential risk for damage to the pool. The potential for unstable slopes and expansive soil to damage the pool is high and there is no economical mitigation. Supporting the pool on piles doesn't make sense because the piles would need to be at least 60 feet deep and that is too long to prevent lateral creep movement. All slopes in Santa Barbara are landslide prone when heavy rains occur. The height and soil type associated with the north slope is such that an incident of instability is possible. It is possible to construct a swimming pool at the proposed location provided it is understood the pool may be damaged by potential slope instability. The recommendations below provide a pool foundation that mitigates the effects of expansive soil only.

1. The foundation system for the proposed replacement pool shall be reinforced concrete grade beams supported by reinforced concrete cast-in-place piles.
2. The reinforced concrete piles shall be a minimum of 18 inches in diameter and shall extend a minimum distance of 15 feet below the floor of the pool.
3. The vertical load capacity of the piles may be based on a skin friction value of 500 psf for the total length of the pile embedded below the floor of the pool.
4. The piles, grade beams, walls, and floor of the pool shall be designed by a Civil or Structural Engineer.
5. A 4-inch-tall collapsible cardboard box (CCB) forming material, such as SureVoid[®] or equivalent, shall be placed below the grade beams and below concrete structural slabs to prevent the uplift swell pressures of the expansive soil from acting on the bottom of the structure.

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INTRODUCTION

This report presents the results of a preliminary geotechnical investigation performed at 3627 Campanil Drive, in the County of Santa Barbara, California, existing at the site is a single-family residence. It is proposed to build a swimming pool in the backyard. It is also proposed to build additions around the perimeter of the existing footprint at selected locations. The site is level as the result of a previous grading operation. The level pad transitions to descending slopes on the north, south, and east sides.

SCOPE OF WORK

It is the purpose of this investigation to classify the soil disclosed by the exploratory borings by observation and tests on selected samples. In addition, this study includes laboratory tests to evaluate soil strength, the effect of moisture variation on the soil-bearing capacity, compressibility, liquefaction, and expansiveness. Based upon this information, we will provide preliminary foundation recommendations for the proposed additions and swimming pool.

The scope of this investigation does not include the analysis of the corrosive potential of the soil, previous site construction, or analysis of geologic structures and their associated features, such as faults, fractures, bedding planes, strike and dip angles, ancient landslides, potential for earth movement in undisturbed or natural soil formations sloped or level, or other sources of potential instability which relate to the geologic conditions, as these items should be addressed by a qualified Engineering Geologist.

This exploration was conducted in accordance with presently accepted geotechnical engineering procedures currently applied in the local community in order to provide the appropriate geotechnical design characteristics of the foundations soils and of the proposed fill soils in order to properly evaluate the proposed structure with respect to differential settlement based upon the anticipated soil characteristics at the time of construction.

LIMITATIONS

This Laboratory's basic assumption is that the soil borings presented herein are representative of the entire footprint of the proposed development, however, no warranty is implied. If, during the course of construction, soil conditions are encountered which vary from those presented herein, please contact this Laboratory immediately so appropriate field modifications may be expeditiously proposed.

It is your responsibility to contact our office, providing at least 48 hours of notice for grading or footing excavation observations and testing. The observation of excavations during the construction phase represents an opportunity by our firm to either confirm soil conditions estimated by the exploratory borings or to discover soil conditions which have not been

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addressed. When such undisclosed conditions are encountered, opinions and recommendations addressing these conditions will be rendered at that time.

This report is considered preliminary and no person should consider the recommendations or soil conditions described herein as conclusive. The recommendations and conclusions of this report are considered preliminary until all excavations have been observed during the construction phase, after which a final report will be issued stating that the grading and foundation works accomplished and installed are appropriate for the soil conditions encountered.

FIELD INVESTIGATION

The subsurface soil conditions were explored by four truck-mounted auger borings, which were drilled to depths of up to 50 feet. The locations of the borings were selected as appropriate and representative. Representative relatively "undisturbed" tube soil samples were obtained during the drilling operation by the Modified California sampler method. Laboratory tests and analysis of representative soil samples, obtained during the drilling operation, were performed to estimate the engineering properties and determine the soil classification. The locations of the borings are shown on Plate 1; these locations are approximate and have not been located by surveyed measurements. The boring log data is presented in Appendix A, "Field Investigation", while the results of the laboratory tests are provided in Appendix B, "Laboratory Tests".

SOIL CONDITIONS

1. No groundwater was encountered in the exploratory borings that extended to depths of up to 50 feet. It should be recognized that water table elevations, even seasonal perched water tables, might fluctuate with time, being dependent upon seasonal precipitation, irrigation, land use, and climatic conditions, as well as other factors. Therefore, water level observations at the time of the field investigation may vary from those encountered during the construction phase of the project. The evaluation of such factors is beyond the scope of this report.
2. The soil profile consists of the dark brown clay of the Rincon Formation, underlain by the Rincon Shale.
3. The black clay topsoil of the Rincon Formation was encountered at a depth of 40 feet in Boring No. 1, indicating approximately 40 feet of fill was placed to create the level pad. The Rincon Shale was encountered at a depth of 50 feet.
4. The supporting soil was found to have a very high potential for expansion.
5. The results of the Consolidation Tests indicate the soil is subject to swell with increased moisture content.

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6. The soil profile at this site is judged to be stiff soil corresponding to a Site Class D as defined by Table 1613.5.2 of the 2013 California Building Code (CBC). This estimate is based on the borings, which encountered the geologic formation known as the Rincon Formation, which is widely regarded as a Type D soil profile since the Standard Penetration Resistance typically results in blow counts having a range of between 15 to 50.
7. The potential for liquefaction is considered to be very low.

PRELIMINARY CONCLUSIONS AND RECOMMENDATIONS

It is the opinion of this Laboratory the proposed construction is feasible from a soil-engineering perspective provided the recommendations contained in this soil engineering report are incorporated into the design and implemented during construction, and provided the owner and future owners understand certain foundation performance limitations.

The clay soil supporting the existing structure is expansive. As the moisture content changes from season to season, the soils supporting the foundation are expected to shrink and expand. The existing single-story house appears to be experiencing these phenomena, based on my observation of cracks in the surfaces of the structure.

It is the understanding of this Laboratory the proposed additions will be one-story wood frame structures. The foundation recommendations for the additions, which follow, conform to the California Building Code (CBC) requirements for the construction of foundations placed on expansive soil.

The foundation design method is provided in the CBC, Section 1808.6.2. This method does not result in a foundation that can prevent the upward heaving movement of an expanding clay, nor the settlement of a shrinking clay. The foundation design is based on the construction of a rigid concrete slab that will move up and down with the unstable clay, but only tilt and not distort while moving, in the attempt to keep the superstructure from distorting while also moving. The problem, for which there may not be an economical solution, is that a foundation for the addition, built to the foundation recommendations contained in this document, will be attached to a different and existing old foundation (not designed to mitigate the effects of expansive soil) and, therefore, differential movement between the two types of foundations is to be expected. The only way to mitigate differential movement is to remove the existing old foundation and replace it with a new foundation of the same construction as the foundation proposed for the addition. This option is typically not selected and, when not selected, the owners and future owners must be informed that differential movement between the existing house and the addition cannot otherwise be prevented and, therefore, must be accepted and tolerated if the two types of foundations are connected. The movement may spread cracks throughout the building.

The complete removal of the existing foundation and the construction of a new foundation using piles, grade beams, and a structural slab would mitigate the potential foundation problems derived from expansive soil. No pile recommendations are

Pacific Materials Laboratory of Santa Barbara, Inc.



1) STREET VIEW TO SOUTH DIRECTLY ACROSS STREET FROM PROPERTY



2) STREET VIEW TO NORTH TOWARDS PROPERTY

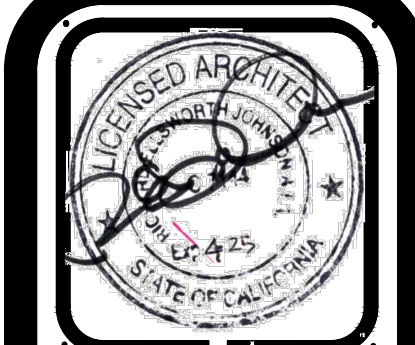
3) VIEW TO NORTH DOWN (E) DRIVEWAY



4) STREET VIEW TO NORTH TOWARDS PROPERTY



5) VIEW OF SOUTH SIDE OF EXISTING RESIDENCE LOOKING EAST



MISSION GROUP ARCHITECTS
R. E. JOHNSON
AIA

PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

DATE
6/30/2023

SHEET
A0.5

1230 COAST VILLAGE CIRCLE STE H SANTA BARBARA CALIFORNIA 93108 805-969-6910 © COPYRIGHT MISSION GROUP ARCHITECTS



6) NORTH SIDE OF EXISTING RESIDENCE LOOKING EAST



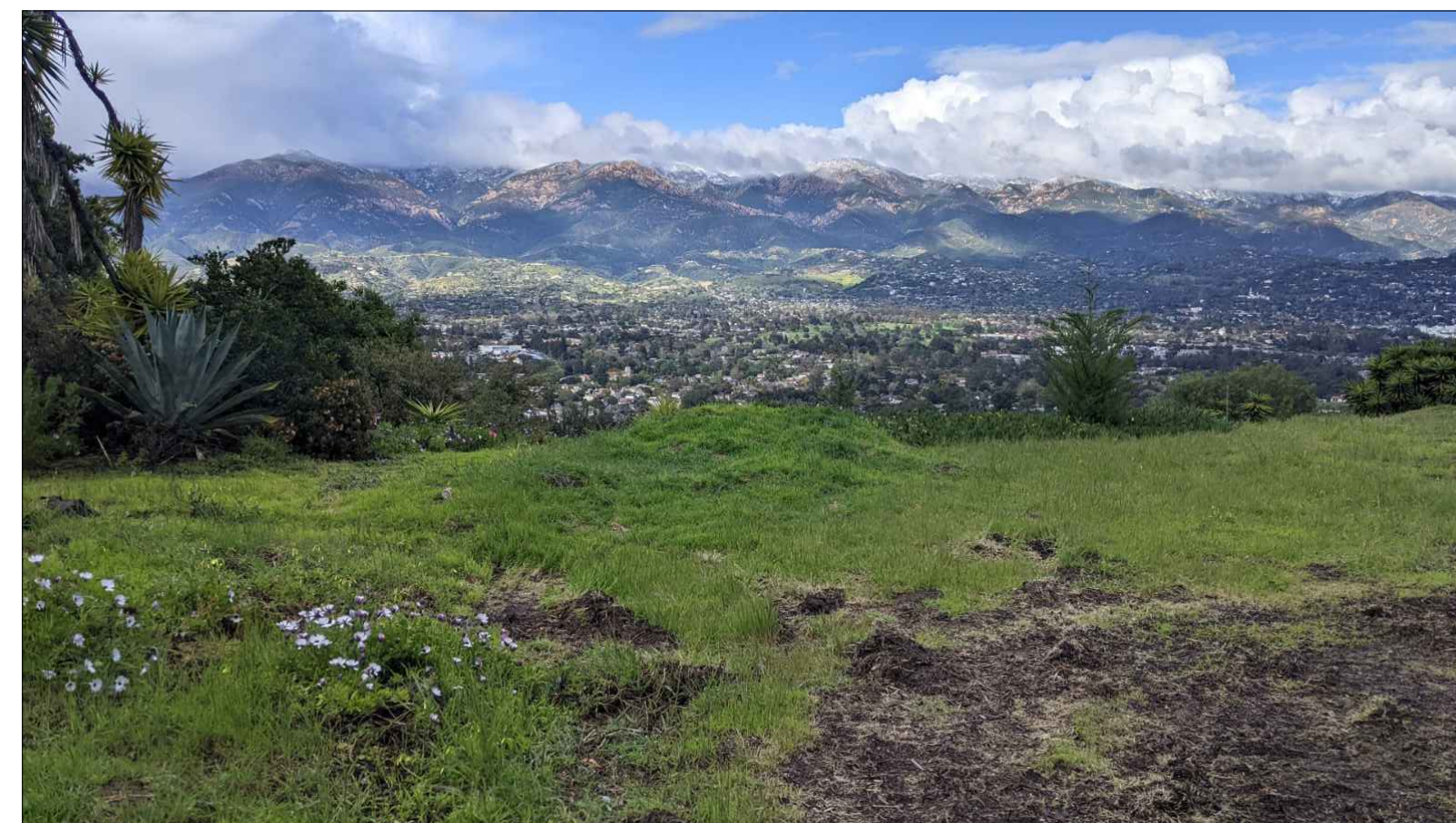
7) WEST SIDE OF EXISTING RESIDENCE LOOKING SOUTH



8) EAST SIDE OF EXISTING RESIDENCE LOOKING SOUTH



9) EAST SIDE OF EXISTING RESIDENCE LOOKING NORTH



10) WEST SIDE OF EXISTING PROPERTY LOOKING NORTH (ADU SITE)



11) NORTH SIDE OF EXISTING RESIDENCE LOOKING WEST (ADU SITE)

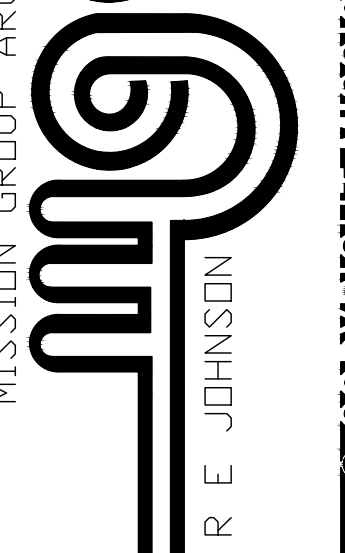


12) AERIAL VIEW OF SITE AND VICINITY



REVISIONS

MISSION GROUP ARCHITECTS



R. E. JOHNSON
AIA

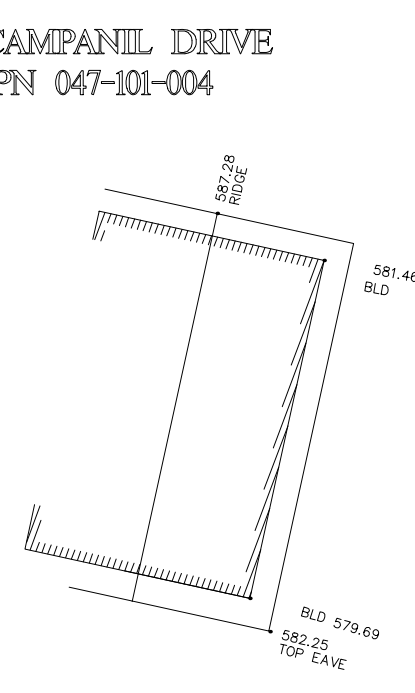
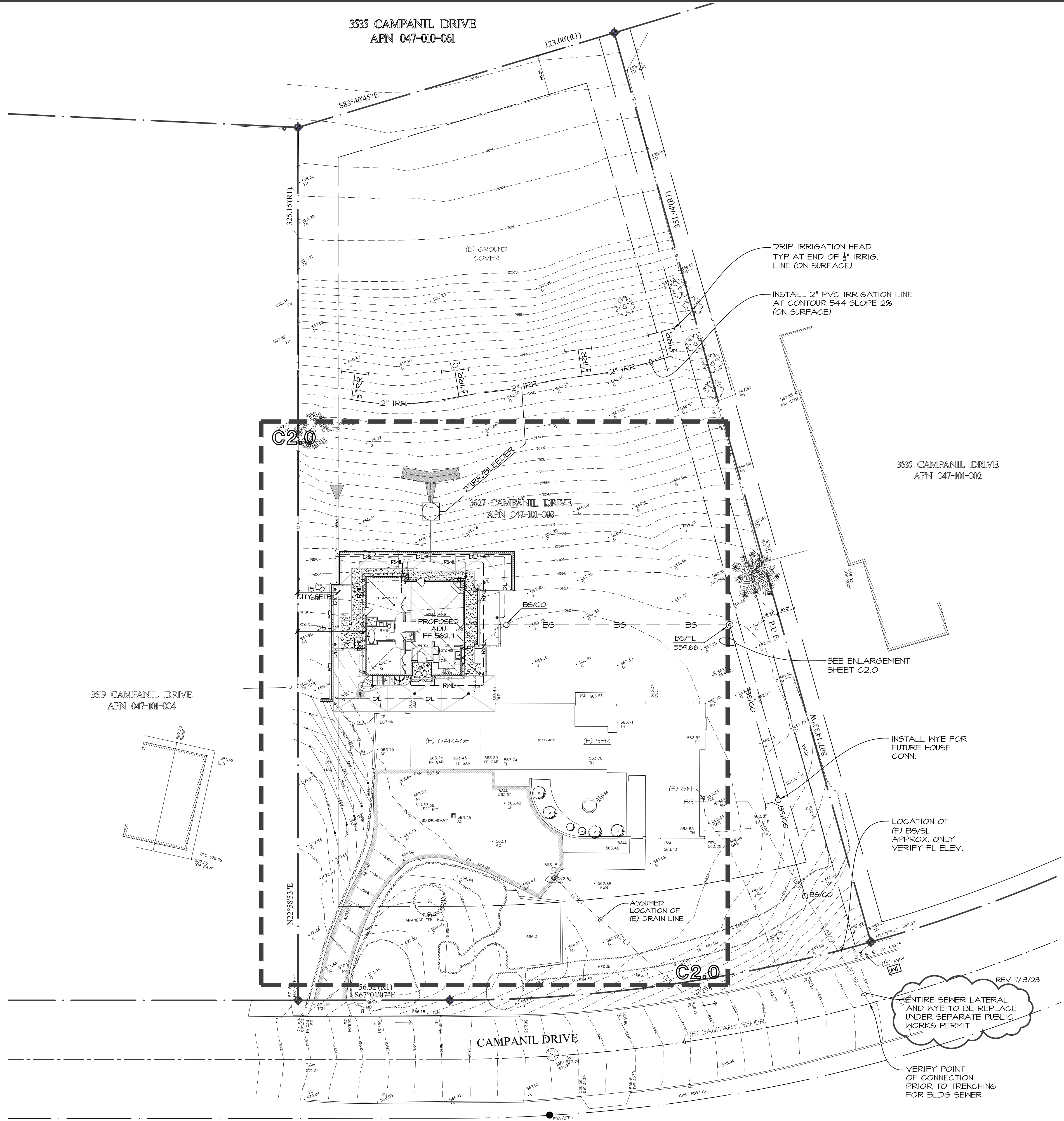
SITE PHOTOS

PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

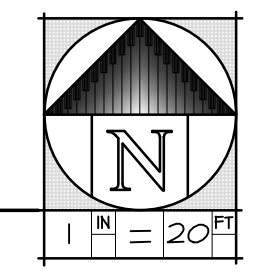
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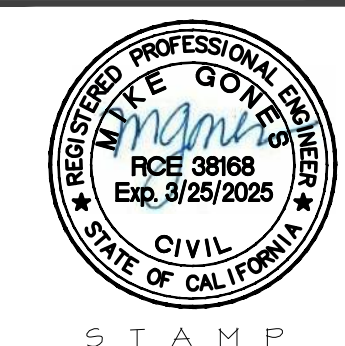


KEY MAP/TOPO PLAN



SURVEYOR'S NOTES

1. BOUNDARY DATA: CAMPANIL HILLS TR. BK. 73, PG. 46 OF MAPS (R1)
2. HORIZONTAL DATUM: NAD83; COORD. SYSTEM: SPC CA 05 4FT, EPOCH 2022.75
3. VERTICAL DATUM: NAVD83; INITIAL STA. SMARTNET 3229
4. PARCEL SIZE: 1.32 AC. GROSS & NET (R1)
5. A CURRENT TITLE REPORT IS REQUIRED TO VERIFY AND PLOT ALL EASEMENTS AFFECTING SUBJECT LOT.
6. BASIS OF BOUNDARY (RECORD BOUNDARY ROTATED TO GRID); FRONT LOT CORNERS (PL. 50 > PL. 56 SHOWN HEREON) S71°19'57"E 214.51' MEASURED; S72°49'10"E 214.40' CALC. FROM (R1); ROTATION = -1°29'13"
7. ORTHOMOSAIC IMAGE: PHANTOM 4 PRO DRONE + PIX4D MAPPER



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CIVIL ENGINEER

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KISLAK ADU

3627 CAMPANIL DRIVE
SANTA BARBARA, CA 93109

Drawings by EG

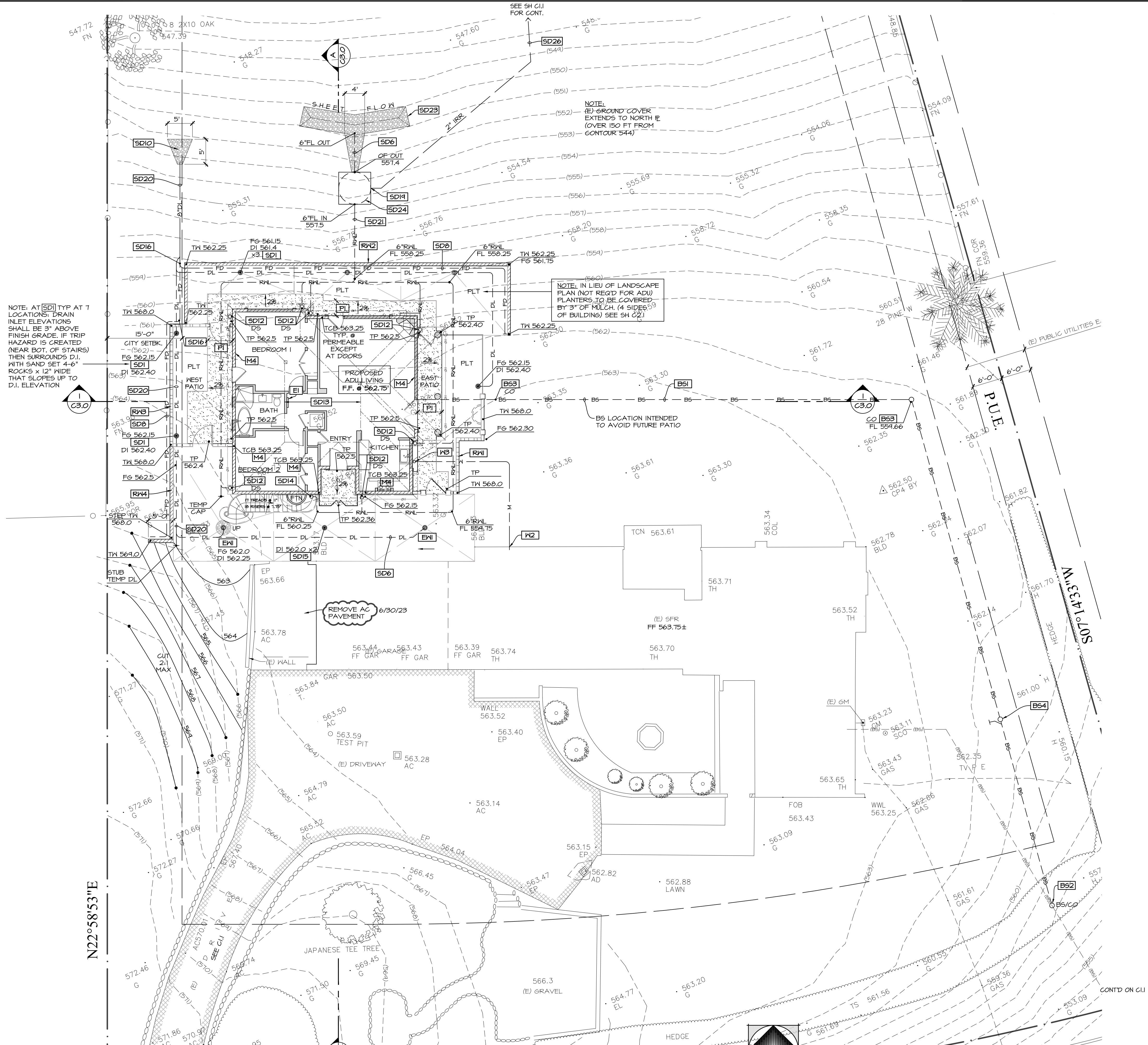
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REVISIONS:

REV	7/13/2023
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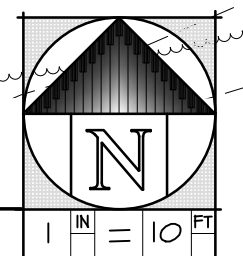
JOB No: 22712

C1.1



GRADING, DRAINAGE AND UTILITIES PLAN FOR ADU

- SEE ARCHITECTURAL AND LANDSCAPE DRAWINGS FOR ADDITIONAL UNDERGROUND CONDUIT, PIPES, ETC. FOR SITE LIGHTING, IRRIGATION.
- SEE ARCHITECTURAL DRAWINGS FOR EXACT DOWNSPOUT LOCATION.
- SEE ARCHITECT FOR DIMENSIONS, SETBACKS, BLDG LOCATION/ORIENTATION, ETC.
- ALL (N) UTILITIES SHALL BE PLACED UNDERGROUND.
- ALL IMPERMEABLE SURFACE (ROOF, PATIOS, DRIVEWAY) SHALL BE CONNECTED TO THE RAIN WATER LINE (RWL) SYSTEM FOR TREATMENT & STORAGE. AREA DRAINS/YARD DRAINS CONNECT TO STORM DRAIN LINE (DL) SYSTEM.



SYMBOLS	
AC	ASPHALT PAVEMENT
AD	AREA DRAIN
BC	BEGIN CURB
BD	BUILDING DRAIN
BE	BUILDING ENVELOPE
BOF	BOTTOM OF FOOTING
BOX	BOTTOM OF KEY
BOL	BOLLARD
BS	BUILDING SEWER
BW	BACK WATER VALVE
CI	CAST IRON
CSK	COUNTERSINK
CO	CLEAN OUT/GARB OUTLET
CP	CONTROL POINT
DE	DEVELOPMENT ENVELOPE
DI	DRAIN INLET
DO	DRAIN OUTLET
DS	DOWN SPOUT
DTM	DRAIN THRU WALL
(E)	EXISTING
EJ	EXPANSION JOINT
EL	EASEMENT LINE
EP	EDGE OF PAVEMENT
FD	FRENCH DRAIN
FF	FINISH FLOOR/TOP OF SLAB
FG	FINISH GRADE
FH	FIRE HYDRANT
FL	FLOW LINE
FS	FLAGSTONE
FL	FIRELINE
G	GRADE
GB	GRADE BREAK
GM	GAS METER
GV	GATE VALVE
HB	HOSE BIP
HL	HOSE LINE
HP	HIGH POINT
IP	IRON PIPE
IRR	IRRIGATION
IV	IRRIGATION VALVE
LA	LANDSCAPE ARCHITECT
LD	LEACH DRAIN
LL	LEACH LINE
LS	LICENSED SURVEYOR
MH	MAN HOLE
(N)	NEW
NC	NOT IN CONTRACT
OF	OVERFLOW
OH	OVERHANG
OHM	OVERHEAD WIRE
PL	PROPERTY LINE
PLT	PLANTER
PP	POWER POLE
PR	PRESSURE REGULATOR
RM	REMOVE
RHM	RIGHT OF WAY
RWL	RAIN WATER LINE
SB	SPLASH BLOCK
SMH	SEWER MANHOLE
SL	SEWER LATERAL
SS	SANITARY SEWER
SC	SANICUT
SM	SIDEWALK
TC	TOP OF CONCRETE
TG	TOP OF CURB
TD	TOP OF DECK
TF	TOP OF FLAGSTONE
TGR	TOP OF GUARDRAIL
TR	TOP OF PAVEMENT
TR	TREE
TOS	TOP OF SLAB
TS	TOP OF SLOPE
TSW	TOP OF SIDEWALK
TH	TOP OF WALL
UNO	UNLESS NOTED OTHERWISE
VLT	VAULT
WV	WATER VALVE
WM	WATER METER
(W)	ELEVATION
(W)	IN ELEVATION
(W)	CONTOUR
(W)	IN CONTOUR
(W)	DIRECTION OF SURFACE DRAINAGE
(W)	SOIL BORING/TEST PIT
(W)	UTILITY POLE
(W)	PROPERTY CORNER

# CONSTRUCTION NOTES	
B51	(N) 4" ABS BLDG. SEWER AT 2% SLOPE, APPROX. LOCATION.
B52	CONNECT TO (E) BS.
B53	(N) SEWER CLEAN OUT.
B54	INSTALL WYE, CO, STUB FOR FUTURE M.H. CONNECTION.
C1	INSTALL CATV PULL BOX.
C2	INSTALL 2" U.G. CATV CONDUIT FROM PULL BOX TO POLE.
E1	INSTALL (N) 100 AMP SUBPANEL.
E2	INSTALL 3" CONDUIT TO (E) MAIN PANEL.
EW1	FINISH GRADE THIS AREA TO SLOPE TO DRAIN INLETS.
M4	SEE STRUCT. FOR 6" CURB ABOVE FF.
P1	CONSTRUCT 5" CONC. SLAB W/4" @ 18" OCEN PER DET. 12/C4.0.
P2	
P3	
P7	
RW1	CONSTRUCT PRIVACY WALL PER DET. 10/C4.0.
RW2	INSTALL RETAINING WALL PER DET. 6/C4.0.
RW3	CONSTRUCT RET. WALL PER DET. 5/C4.0.
RW4	CONSTRUCT RET. WALL PER DET. 20/C4.0.
SD1	INSTALL PLANTER DRAIN PER DET. 8/C4.0 W=4".
SD2	INSTALL PLANTER DRAIN PER DET. 8/C4.0 W=6".
SD5	INSTALL PATIO DRAIN PER DET. 3/C4.0. W=4".
SD6	INSTALL 6" DL AT 2%.
SD7	INSTALL 4" DL @ 2%.
SD8	INSTALL 4" FD AT 1% MIN. SLOPE.
SD10	INSTALL ENERGY DISSIPATOR PER DET. 14/C4.0. L=5' W=5'.
SD11	INSTALL PATIO DRAIN PER DET. 3/C4.0 W=3". SLOPE CONC TO CENTER.
SD12	CONNECT DS TO RWL PER DET. 11/C4.0.
SD13	ROOF TOP DECK DRAINAGE TO OUTLET ONTO ROOF GUTTER SYSTEM.
SD14	PATIO DRAIN AT TOP EXT. STEPS PER ARCHITECT.
SD15	INSTALL TEMPORARY 50 OR ROUND YARD DRAIN PER DET. 4/C4.0 W=12" TO SERVE UNTIL MAIN HOUSE FLATWORK COMPLETE.
SD16	CONNECT FD TO DL.
SD18	FLAT ROOF DRAIN INLET, PIPE TO DS PER ARCH. DRWG.
SD19	PROVIDE 2" PVC OUTLET PIPE WITH MANUAL VALVE NEAR BOTTOM OF TANK FOR GRAVITY IRRIGATION OF GROUND COVER ON HILLSIDE BELOW ADU. SLOPE PIPE AT 2%.
SD20	INSTALL 8" DL @ 2% MIN. SLOPE.
SD21	INSTALL 6" RWL @ 2% MIN. SLOPE.
SD22	INSTALL 4" RWL @ 2% MIN. SLOPE.
SD23	INSTALL OVERFLOW FLOW SPREADER V. DITCH DET. 16/C4.0.
SD24	INSTALL 1250 GAL. UNDERGROUND RAIN WATER STORAGE CISTERN FLOOR AT ELEV. 546.5. ACCESS LID/RISER, SEE SH. C4.1 FOR CISTERN CONTROLS. CISTERN TO HAVE PUMP TO IRRIGATION BELOW CONTOUR 544 CISTERN TO SIT ON 6" CLASS II BASE @ 45% REL. COMP. THAT EXTENDS 12" BEYOND TANK PROVIDE 4" FRENCH DRAIN AROUND TANK BASE.
SD25	INSTALL 6" OVERFLOW LINE @ 2% SLOPE OUTLET.
SD26	INSTALL 2" PVC IRRIGATION DISTRIBUTION PIPE @ 2% SLOPE.
T1	INSTALL TELEPHONE PULL BOX.
T2	INSTALL 2" U.G. TELEPHONE CONDUIT FROM PULL BOX TO POLE.
W1	(E) DOMESTIC WATER METER. OWNER TO EVALUATE CONDITION OF SERVICE PIPE TO HOUSE AND REPLACE AS NECESSARY. (SHOWN ON C11)
W2	CONNECT 3" WL TO EXISTING HOUSE PLUMBING SYSTEM UNTIL NEW MAIN RESIDENCE IS BUILT. CONTRACTOR TO DETERMINE BEST POINT FOR TEMP. CONNECTION.
W3	PROVIDE SHUT OFF VALVE & PRESSURE REGULATOR.

SURVEYOR'S NOTES

- BOUNDARY DATA: CAMPANIL HILLS TR. BK. 73, PG. 46 OF MAPS (R1)
- HORIZONTAL DATUM: NAD83; COORD. SYSTEM: SPC CA 05 sFT, EPOCH 2022.75
- VERTICAL DATUM: NAVD83; INITIAL STA. SMARTNET 3229
- PARCEL SIZE: 1.32 AC. GROSS & NET (R1)
- A CURRENT TITLE REPORT IS REQUIRED TO VERIFY AND PLOT ALL EASEMENTS AFFECTING SUBJECT LOT.
- BASIS OF BOUNDARY (RECORD BOUNDARY ROTATED TO GRID): FRONT LOT CORNERS (P150 - P156 SHOWN HEREON) S71°19'57"E 214.51' MEASURED; S72°49'10"E 214.40' CALC. FROM (R1); ROTATION = -1°29'13"
- ORTHOMOSAIC IMAGE: PHANTOM 4 PRO DRONE + PIX4D MAPPER

STORM WATER SUMMARY

WATER QUALITY PER TIER 2 (STORAGE NOT REQ'D) REQUIRED BY CITY OF SANTA BARBARA WILL BOTH BE ACCOMPLISHED BY INSTALLATION OF CISTERN STORAGE SYSTEM. THIS SYSTEM ASSURES NO INCREASE IN RUNOFF FROM THE PROPOSED DEVELOPMENT FOR STORM RUNOFF UP TO THE 25 YR. STORM. OVERFLOW FOR STORMS GREATER THAN THIS WILL HONOR THE EXISTING HISTORICAL RUNOFF PATTERNS OF USING THE EXISTING NATURAL WATERSHED.

STORM WATER MANAGEMENT CERTIFICATE

PROJECT CIVIL ENGINEER SHALL SUBMIT STAMPED LETTER CERTIFYING THAT ALL POST CONSTRUCTION BMP'S HAVE BEEN INSTALLED PER PLAN. CONTRACTOR SHALL CALL ENGINEER FOR INSPECTION PRIOR TO COVERING WORK. CERTIFICATE WILL BE REQUIRED PRIOR TO CERTIFICATE OF OCCUPANCY.

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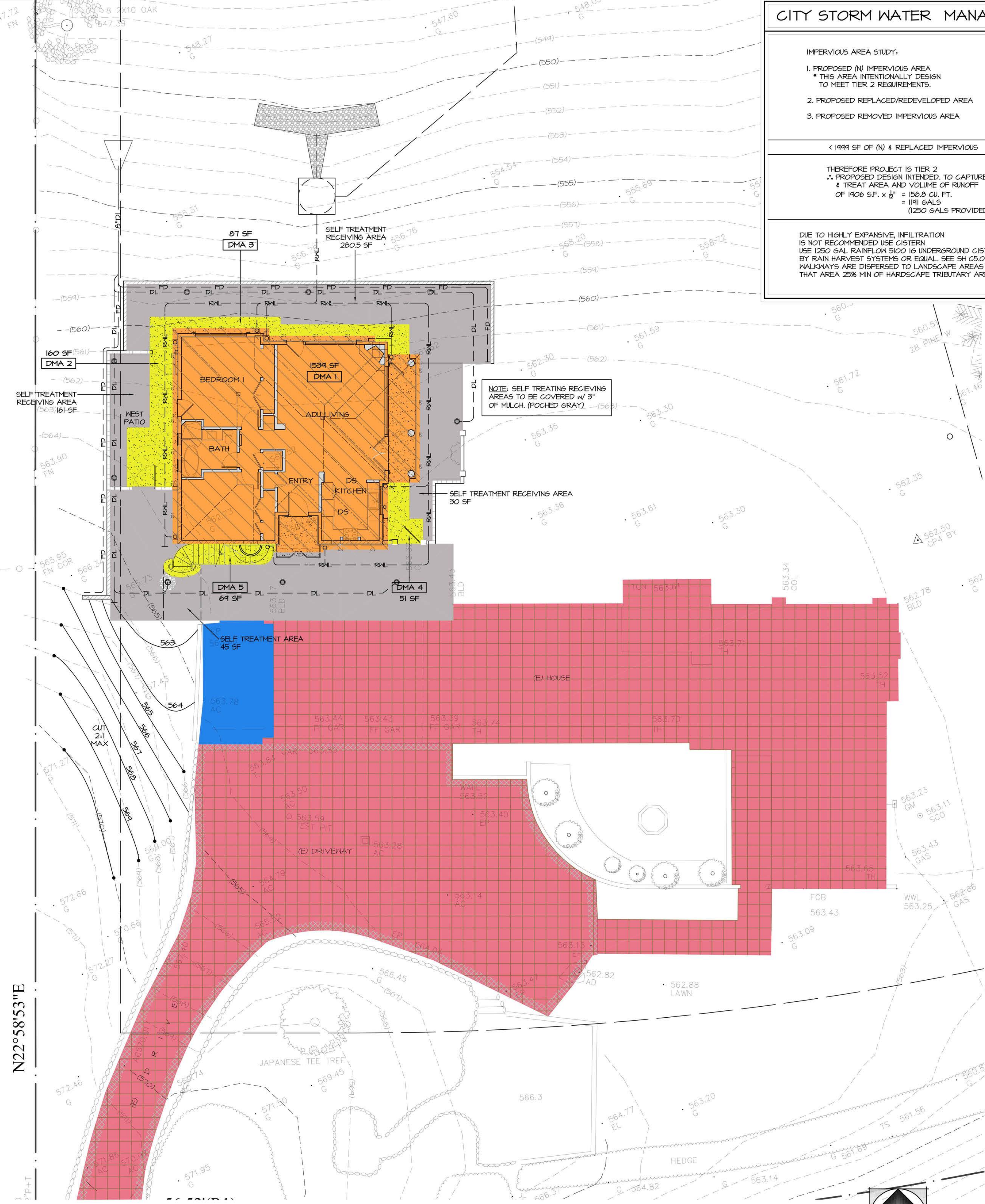
KISLAK ADU
 3627 CAMPANIL DRIVE
 SANTA BARBARA, CA 93109

Drawings by EG
 DATE: 6/30/2023

REVISIONS:

JOB No: 22712

C2.0



CITY STORM WATER MANAGEMENT

IMPERVIOUS AREA STUDY:

1. PROPOSED (N) IMPERVIOUS AREA THIS AREA INTENTIONALLY DESIGN TO MEET TIER 2 REQUIREMENTS. **1406***

2. PROPOSED REPLACED/REDEVELOPED AREA **0**

3. PROPOSED REMOVED IMPERVIOUS AREA **264**

< 1499 SF OF (N) & REPLACED IMPERVIOUS

THEREFORE PROJECT IS TIER 2
 ** PROPOSED DESIGN INTENDED TO CAPTURE & TREAT AREA AND VOLUME OF RUNOFF OF 1406 SF. x 1" = 150.8 CU. FT. = 1191 GALS. (1250 GALS PROVIDED)

DUE TO HIGHLY EXPANSIVE INFILTRATION IS NOT RECOMMENDED USE CISTERN USE 1250 GAL. RAINFLOW 5100 (6 UNDERGROUND CISTERN SYSTEM BY RAIN HARVEST SYSTEMS OR EQUAL. SEE SH C5.0 WALKWAYS ARE DISPERSED TO LANDSCAPE AREAS THAT AREA 25% MIN OF HARDSCAPE TRIBUTARY AREA.

LEGEND: AREA STUDY

PROPOSED NEW IMPERVIOUS AREA- AREA WHERE NEW IMPERVIOUS AREA (E.G. HARDSCAPE AND ROOF) IS PROPOSED WHERE THERE IS EXISTING PERVIOUS AREA (LANDSCAPING, PERMEABLE PAVEMENT, ETC.)
1539 SQ FT DMA 1

PROPOSED REPLACED/REDEVELOPED IMPERVIOUS AREA- AREA WHERE NEW IMPERVIOUS AREA (E.G. HARDSCAPE AND ROOF) IS PROPOSED WHERE THERE IS CURRENTLY EXISTING IMPERVIOUS AREA (E.G. HARDSCAPE AND ROOF)
0 SQ FT

PROPOSED REMOVED IMPERVIOUS AREA- AREA WHERE NEW PERVIOUS AREA IS PROPOSED (LANDSCAPING, PERMEABLE PAVEMENT, ETC.) WHERE THERE IS CURRENTLY EXISTING IMPERVIOUS AREA (E.G. HARDSCAPE AND ROOF)
264 SQ FT

(E) IMPERVIOUS TO REMAIN
1031 SQ FT (E) HOUSE & DRIVEWAY

SELF TREATING WALKS (DRAINS TO VEGETATION YARD/SELF TREATING AREAS)
367 SQ FT DMA 2-5
 AREA REQ'D = 25x367 = 92 SF
 AREA PROVIDED = 517 SF

NOTE: TOTAL NEW IMPERVIOUS AREA (INCLUDING SELF TREATING WALKS) = 1539 + 367 = 1906 SF
 PROPOSED IMPERVIOUS AREA IS CUMULATIVE FOR TWO YEARS AFTER CERTIFICATE OF OCCUPANCY TO PREVENT PIECE MEALING OF PROJECTS TO AVOID STORM WATER MANAGEMENT.

NOTE: AREAS SHOWN ARE IN COLOR PLEASE PRINT IN COLOR BEFORE REVIEW.

AREA TO BE COVERED IN 3" OF MULCH.

POST CONSTRUCTION BMP INSPECTION

The following mandatory inspection by the city Building Inspector or City GSP are required. Inspection shall be called in by Contractor 72 hours prior to needed inspection. The City will then route to the GSP Inspector or third Party Company.

A. General
 1. Pre construction meeting attendance.

B. Cistern
 1. Excavation
 2. After tanks set on certified class II base.
 3. After U.G. piping is complete
 4. After system is complete with pumps/controls etc.

C. Irrigation pipe
 1. After pipe & irrig. heads are installed.

STORM WATER HYDROLOGY STUDY

Q_{pre} = CIA (cfs)
 = [0.95x2.9x(7295/43560)] + [0.71x2.9x(50205/43560)]
 = 0.46 cfs + 2.37 cfs
 = 2.83 cfs

Q_{post} = CIA (cfs)
 = [0.95x2.9x(9201/43560)] + [0.71x2.9x(48299/43560)]
 = 0.58 + 2.28 cfs
 = 2.86 cfs

% increase = (2.86-2.83)/2.83 = 0.011 (1.1%)

Q = CIA (cfs) (rational formula)
 *C = 0.71 landscape *(per County Engineering Design Standards, runoff coefficient vs. rainfall figure 2 of Blue Book)
 C = 0.95 hardscape
 I = 2.9 in/hr 25 yr storm (per County Flood Control intensity duration curves, figure 1 of Blue Book)
 A = tributary area in acres

Volume Reduction Requirements (Storm Volume):
 Compare ΔVol₂₅ (25 yr storm runoff volume to be retained on site) to 1.0 in/24 hr storm.

Per City of SB Storm Guidance Manual, Appendix C p. C-3:
 ΔVol₂₅ = volume of runoff to be retained on site (cu. ft.)
 = 0.5 x ΔQ₂₅ x 2.67 x 720 (T_c in sec)
 = 0.5 x (2.86-2.83) x 2.67 x 720
 = 28.84 cu. ft.

For 1.0 in/24 hr storm
 Vol_{req} = (N) or replaced impervious area x 1.0 in/12 in ft
 = 1906 x 1.0/12 = 158.8 cu. ft.

Vol_{req} > ΔVol₂₅ ∴ required storage = 158.8 cu. ft. x 7.5 gal. ft³ = 1191 gal.

Available storage per sheet C.2 of plans: Underground Cisterns = 1 x 1250 gal. each = 1250 gal.

STORM WATER MANAGEMENT STUDY

THE FOLLOWING INFORMATION IS BASED ON COUNTY STORM WATER BMP GUIDANCE MANUAL # GRADING PLAN AND DRAINAGE BY MIKE GONES CIVIL ENGINEER, DATED MARCH 2023.

1. PROJECT IS HILLSIDE RESIDENTIAL WITH LESS THAN 1499 SF NEW IMPERVIOUS THEREFORE TIER 2.

2. SITE ASSESSMENT
 A. HILLSIDE DESIGN DISTRICT
 B. ARROYO BURRO WATERSHED
 C. TYPE SOIL D / CLAY
 D. SITE SLOPES MOSTLY NORTH > BLDG PAD AREA = 5% HILLSIDE = 25%
 E. NO GROUND WATER ANTICIPATED
 F. NO FLOOD HAZARD

3. POLLUTANTS OF CONCERN:
 TRASH - NOT AN ISSUE, PROPERTY LINE IS FENCED
 NUTRIENTS - N/A
 BACTERIA - NOT AN ISSUE CONNECTED TO PUBLIC SEWER
 SEDIMENT - NOT AN ISSUE OTHER THAN FROM DURING CONSTRUCTION (EROSION CONTROL PLAN WILL BE PREPARED)
 HYDROCARBON - DRIVEWAY RUNOFF (TREATED BY FILTRATION SYSTEM)
 METAL - NOT AN ISSUE FOR RESIDENTIAL PROJECT
 PESTICIDE - N/A

4. SITE DESIGN BMP OPTIONS: (E) RUNOFF PATTERNS TO BE HONORED. GRADING TO RESPECT (E) CONTOURS.

5. BASIC BMP'S - TIER 2, SEE STORM WATER RUNOFF BMP'S.

6. STORM WATER BMP OPTION SELECTED: CISTERN.

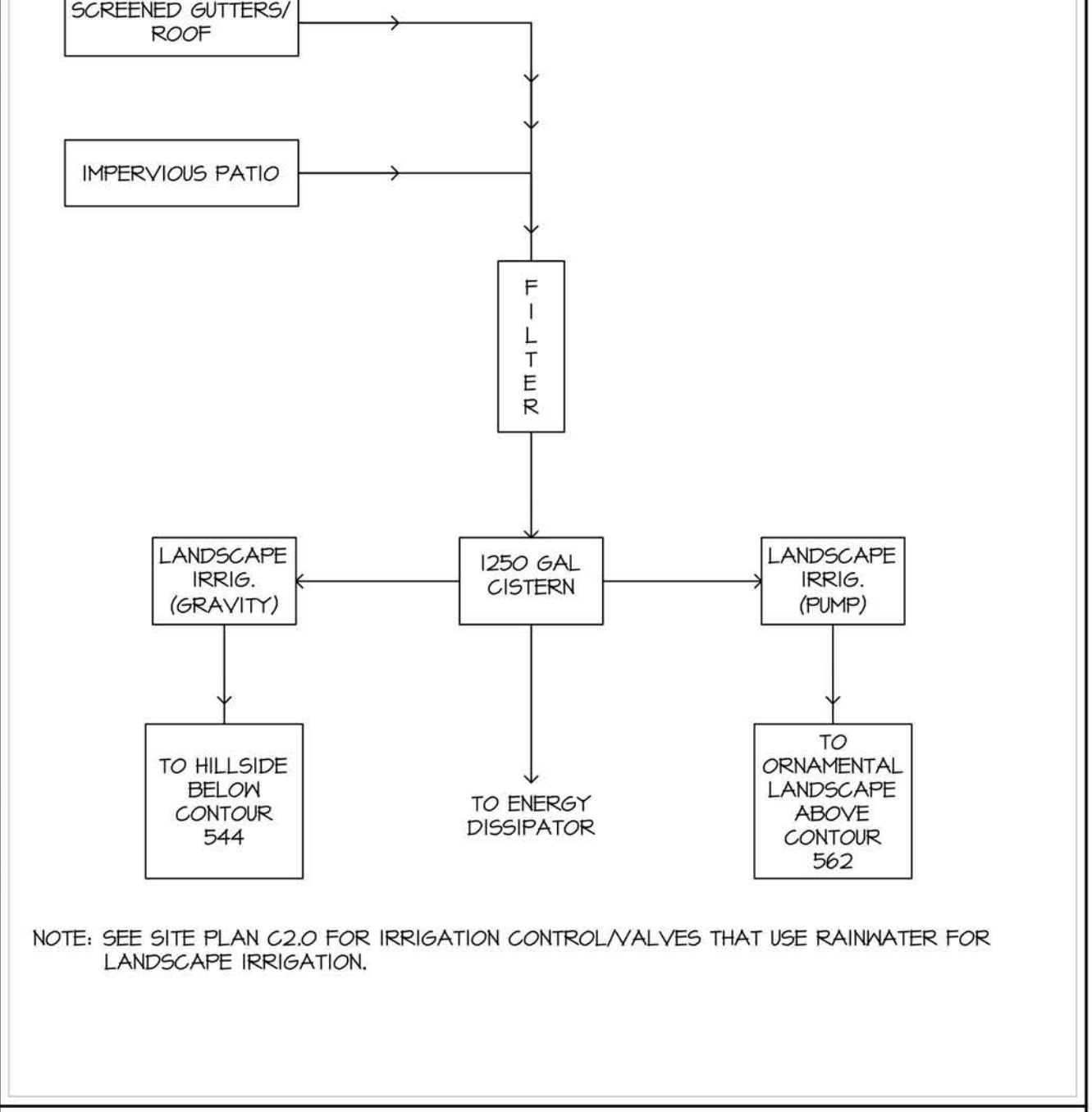
7. AREA STUDY FOR HYDROLOGY STUDY SEE SEPARATE STUDY THIS SHEET FOR CITY SWMP AREAS.

A. IMPERVIOUS AREAS (SQ. FT.)	(E)	PROPOSED	CHANGE
1. ROOF AREA	2444	4533	+ 1539
2. PATIO/WALK	1067	1434	+ 367
3. DRIVEWAY	3234	3234	0
4. SUB-TOTAL	7245	9201	+ 1906

B. PERVIOUS (SQ. FT.)	(E)	PROPOSED	PROPOSED
1. PATIO/WALK	0	0	0
2. DRIVEWAY	0	0	0
3. LANDSCAPE	50,205	48,299	- 1906
3. SUB-TOTAL	50,205	48,299	- 1906
TOTAL A4 + B3	51,500	51,500	0

NOTES: 1. INCREASE IN IMPERVIOUS < 2000 SF. ∴ TIER 2.
 2. PROPOSED IMPERVIOUS AREA = 1906 SF x 1/2 = 150.8 CU. FT. (V_{req}) x 7.5 GAL/CU. FT. = 1191 GAL. SEE HYDROLOGY STUDY THIS SHEET FOR MORE INFORMATION.

IMPERVIOUS AREA DATA:
 NEW IMPERVIOUS AREA = 1906 SF.
 REPLACED IMPERVIOUS AREA = 0 SF.
 REMOVED IMPERVIOUS AREA = 0 SF.



STORM WATER BMP MAINTENANCE AGREEMENT

THE PROPOSED STORM WATER BEST MANAGEMENT PRACTICES WILL BE MAINTAINED PURSUANT TO SBMC 22.87.050. PROPOSED BEST MANAGEMENT PRACTICES: UNDERGROUND CISTERNS CONNECTED TO LANDSCAPING AND PERMEABLE PATH SELF TREATING WALKWAYS.

Paula Kislak 6/30/23
 PROPERTY OWNER SIGNATURE DATE

STORM WATER MANAGEMENT CERTIFICATION

BEFORE BUILDING INSPECTOR WILL GRANT CERTIFICATE OF OCCUPANCY AND FINALIZE THE BUILDING PERMIT, THE BUILDING AND SAFETY DIVISION OR THEIR CONTRACTED GSP SERVICE PROVIDER MUST VERIFY THAT ALL POST CONSTRUCTION STORM WATER BMP'S WERE INSTALLED AS APPROVED AND THAT THEY COMPLY WITH THE CITY'S TIER 3 STORM WATER REQUIREMENTS.

STORM WATER DRAINAGE MANAGEMENT AREA STUDY

REGISTERED PROFESSIONAL ENGINEER
MIKE GONES
 CIVIL ENGINEER
 Exp. 3/25/2025
 STATE OF CALIFORNIA

STAMP

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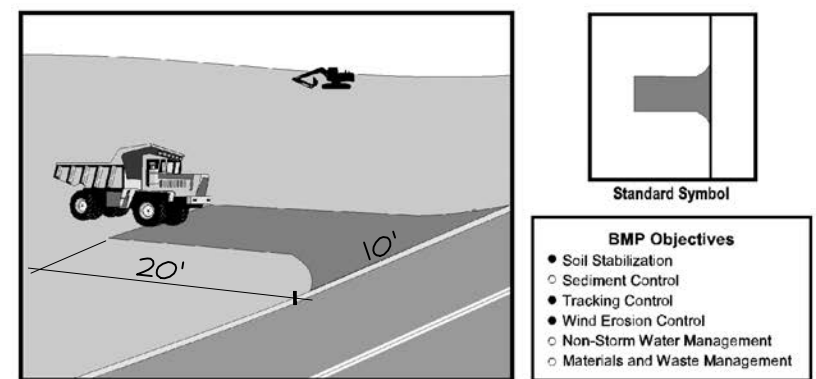
Drawings by EG
 DATE: 6/30/2023
 REVISIONS:

JOB No: 22T12

C2.1

SHEET 4 of 7

Stabilized Construction Entrance/Exit **TC-1**



Definition and Purpose
A stabilized construction entrance is defined by a point of entrance to a construction site that is stabilized to reduce the tracking of mud and dirt onto public roads by construction vehicles.

Appropriate Applications

- Use at construction sites:
 - When dirt or mud can be tracked onto public roads.
 - Adjacent to water bodies.
 - Where poor soils are encountered.
 - Where dust is a problem during dry weather conditions.
- This BMP may be implemented on a project-by-project basis in addition to other BMPs when deemed necessary and feasible by the Resident Engineer (RE).

Limitations

- Site conditions will dictate design and need.

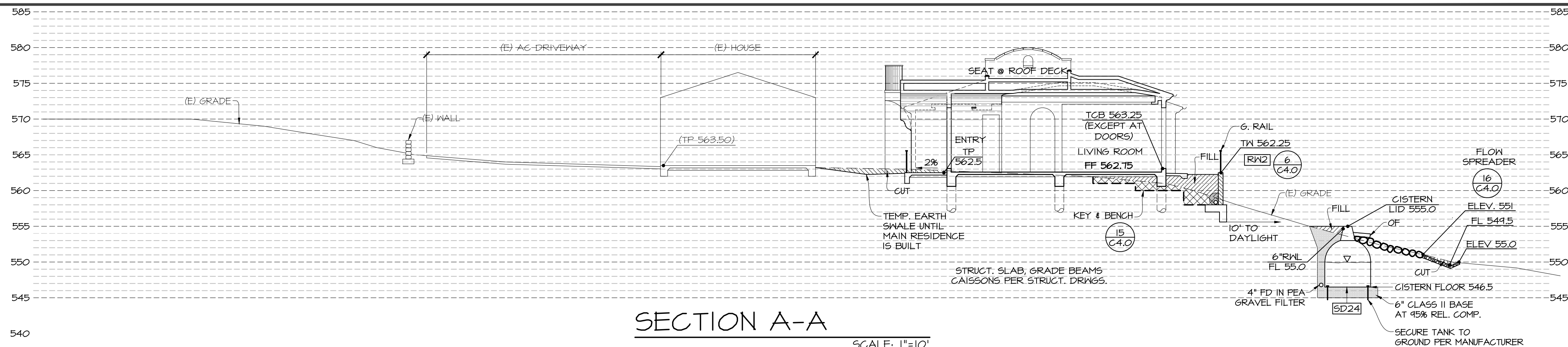
Standards and Specifications

- Limit the points of entrance/exit to the construction site.
- Limit speed of vehicles to control dust.
- Properly grade each construction entrance/exit to prevent runoff from leaving the construction site.
- Route runoff from stabilized entrances/exits through a sediment-trapping device before discharge.
- Design stabilized entrance/exit to support the heaviest vehicles and equipment that will use it.

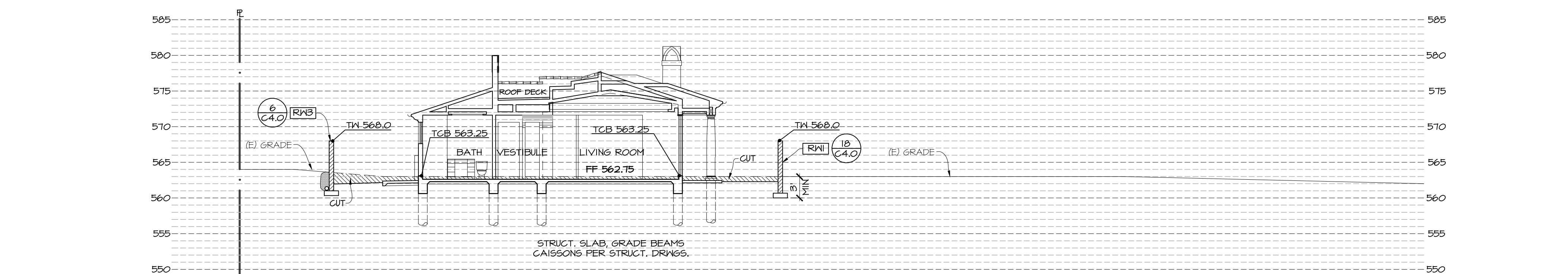
California Storm Water Quality Handbook
Construction Site Best Management Practices Manual
March 1, 2003
Section 1
Stabilized Construction Entrance/Exit TC-1
1 of 1

BEST MANAGEMENT PRACTICES FOR CONSTRUCTION ACTIVITIES:

- ERODED SEDIMENTS AND OTHER POLLUTANTS MUST BE RETAINED ON SITE AND MAY NOT BE TRANSPORTED FROM THE SITE VIA SHEET FLOW, SHALES, AREA DRAINS, NATURAL DRAINAGE COURSES OR RIND.
- STOCKPILES OF EARTH AND OTHER CONSTRUCTION RELATED MATERIALS MUST BE PROTECTED FROM BEING TRANSPORTED FROM THE SITE BY THE FORCES OF WIND OR WATER.
- FUELS, OILS, SOLVENTS AND OTHER TOXIC MATERIALS MUST BE STORED IN ACCORDANCE WITH THEIR LISTING AND ARE NOT TO CONTAMINATE THE SOIL AND SURFACE WATERS. ALL APPROVED STORAGE CONTAINERS ARE TO BE PROTECTED FROM THE WEATHER. SPILLS MAY NOT BE WASHED INTO THE DRAINAGE SYSTEM.
- EXCESS OR WASTE CONCRETE MAY NOT BE WASHED INTO THE PUBLIC WAY OR ANY OTHER DRAINAGE SYSTEM. PROVISIONS MUST BE MADE TO RETAIN CONCRETE WASTES ON SITE UNTIL THEY CAN BE DISPOSED OF AS A SOLID WASTE.
- TRASH AND CONSTRUCTION RELATED SOLID WASTES MUST BE DEPOSITED INTO A COVERED RECEPTACLE TO PREVENT CONTAMINATION OF RAINWATER AND DISPERSAL BY WIND.
- SEDIMENTS AND OTHER MATERIAL MAY NOT BE TRACED FROM THE SITE BY VEHICLE TRAFFIC. THE CONSTRUCTION ENTRANCE ROADWAYS MUST BE STABILIZED SO AS TO INHIBIT SEDIMENTS FROM BEING DEPOSITED INTO THE PUBLIC WAY. ACCIDENTAL DEPOSITIONS MUST BE SWEEPED UP IMMEDIATELY AND MAY NOT BE WASHED DOWN BY RAIN OR OTHER MEANS.
- ANY SLOPES WITH DISTURBED SOILS OR DEMANDED OF VEGETATION MUST BE STABILIZED SO AS TO INHIBIT EROSION BY WIND AND WATER.



SECTION A-A
SCALE: 1"=10'

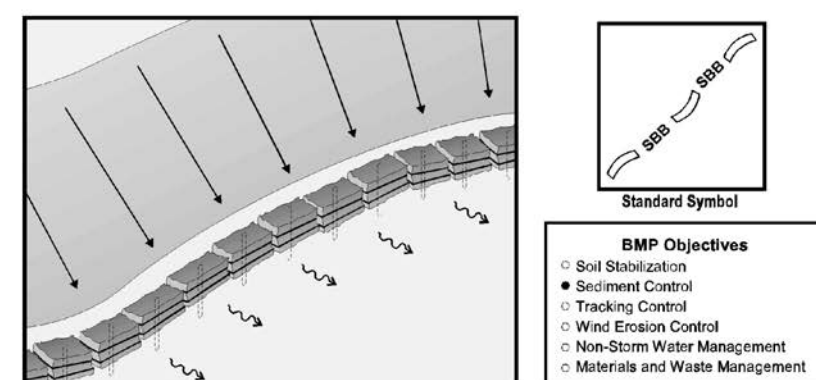


SECTION I-I
SCALE: 1"=10'

5 CONSTRUCTION ENTRANCE **N.T.S.**

1 BEST MANAGEMENT PRACTICES

Straw Bale Barrier **SC-9**



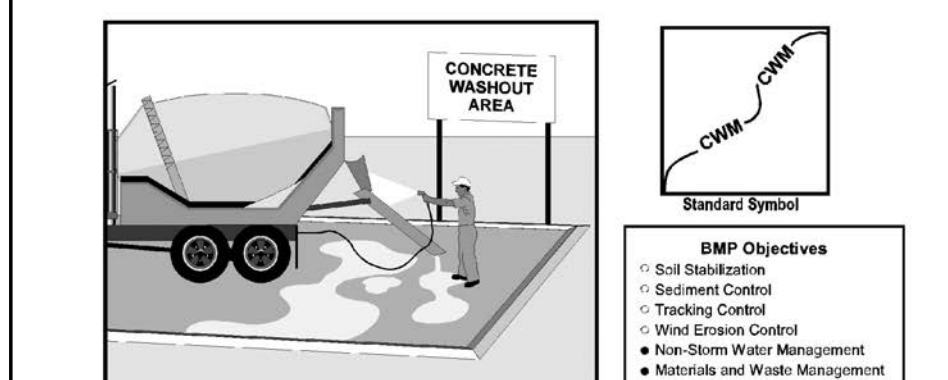
Definition and Purpose
A straw bale barrier is a temporary linear sediment barrier consisting of straw bales, arranged to intercept and slow sediment-laden sheet flow runoff. Straw bale barriers allow sediment to settle from runoff before water leaves the construction site.

Appropriate Applications

- This BMP may be implemented on a project-by-project basis in addition to other BMPs when deemed necessary and feasible by the Resident Engineer (RE).
- Along the perimeter of a site.
- Along streams and channels.
- Below the toe of exposed and erodible slopes.
- Down slope of exposed soil areas.
- Around stockpiles.
- Across minor swales or ditches with small catchments.
- Around above grade type temporary concrete washouts (See BMP WM-8, "Concrete Waste Management").
- Parallel to a roadway to keep sediment off paved areas.

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March 1, 2003
Section 4
Straw Bale Barrier SC-9
1 of 1

Concrete Waste Management **WM-8**



Definition and Purpose
These are procedures and practices that are designed to minimize or eliminate the discharge of concrete waste materials to the storm drain systems or watercourses.

Appropriate Applications

- Concrete waste management procedures and practices are implemented on construction projects where concrete is used as a construction material or where concrete dust and debris result from demolition activities.
- Where slurries containing portland cement concrete (PCC) or asphalt concrete (AC) are generated, such as from sawcutting, coring, grinding, grinding, and hydro-concrete demolition.
- Where concrete trucks and other concrete-related equipment are washed on site, when approved by the Resident Engineer (RE). See also NS-4, "Vehicle and Equipment Cleaning."
- Where mortar mixing stations exist.
- None identified.

Limitations

- None identified.

Standards and Specifications

Education

- Educate employees, subcontractors, and suppliers on the concrete waste management techniques described herein.
- The Contractor's Water Pollution Control Manager (WPCM) shall oversee and enforce concrete waste management procedures.

Concrete Slurry Wastes

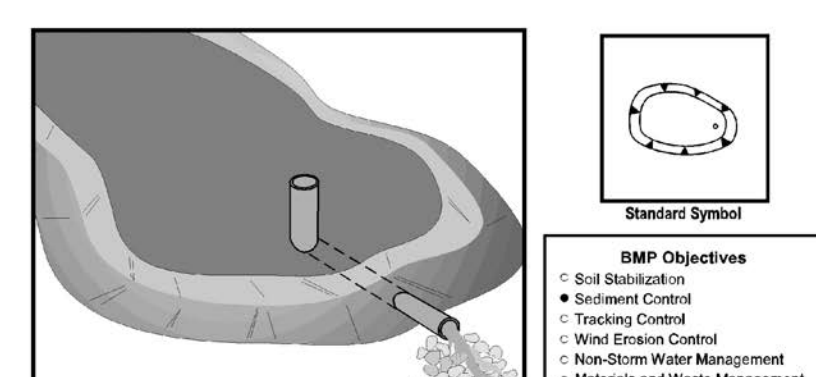
- PCC and AC waste shall not be allowed to enter storm drains or watercourses.

California Storm Water Quality Handbook
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Section 2
Concrete Waste Management WM-8
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6 STRAW BALE BARRIER **N.T.S.**

2 CONCRETE WASHOUT AREA **N.T.S.**

Sediment/Desilting Basin **SC-2**



Definition and Purpose
A sediment/desilting basin is a temporary basin formed by excavating and/or constructing an embankment so that sediment-laden runoff is temporarily detained under unexcavated conditions, allowing sediment to settle out before the runoff is discharged (refer to Figures 1 and 2).

Appropriate Applications

Sediment basins shall be designed in accordance with Section A of the State of California NPDES General Permit for Storm Water Discharges Associated with Construction Activities (General Permit). If there is insufficient area to construct a sediment basin in accordance with the General Permit requirements, then the alternate desilting design standards specified herein may be used. This BMP may be implemented on a project-by-project basis with other BMPs when deemed necessary and feasible by the RE.

Sediment/Desilting Basins shall be considered for use:

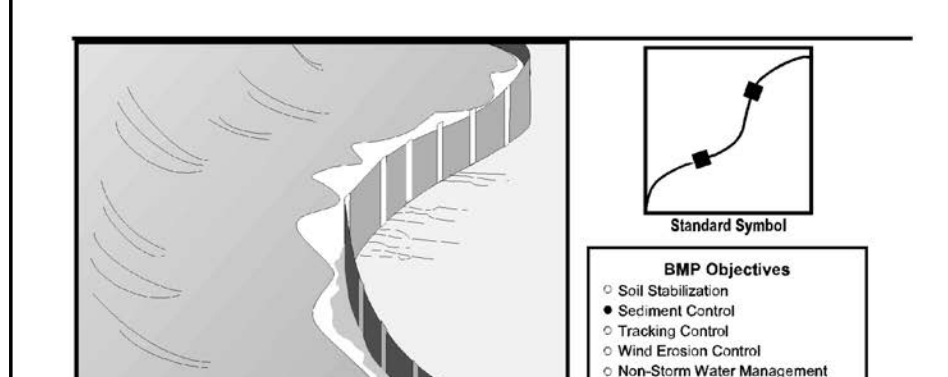
- On construction projects with disturbed areas during the rainy season.
- Where sediment-laden water may enter the drainage system or watercourses.
- At outlets of disturbed soil areas with areas between 2 ha and 4 ha (5 ac and 10 ac).

Limitations

- Alternative BMPs must be thoroughly investigated for erosion control before selecting temporary desilting basins.
- Requires large surface areas to permit settling of sediment.
- Not appropriate for drainage areas greater than 30 ha (75 ac).
- Not to be located in live streams.

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Sediment/Desilting Basin SC-2
1 of 1

Silt Fence **SC-1**



Definition and Purpose
A silt fence is a temporary linear sediment barrier of permeable fabric designed to intercept and slow the flow of sediment-laden sheet flow runoff. Silt fences allow sediment to settle from runoff before water leaves the construction site.

Appropriate Applications

Silt fences are placed:

- Below the toe of exposed and erodible slopes.
- Down-slope of exposed soil areas.
- Around temporary stockpiles.
- Along streams and channels.
- Along the perimeter of a project.

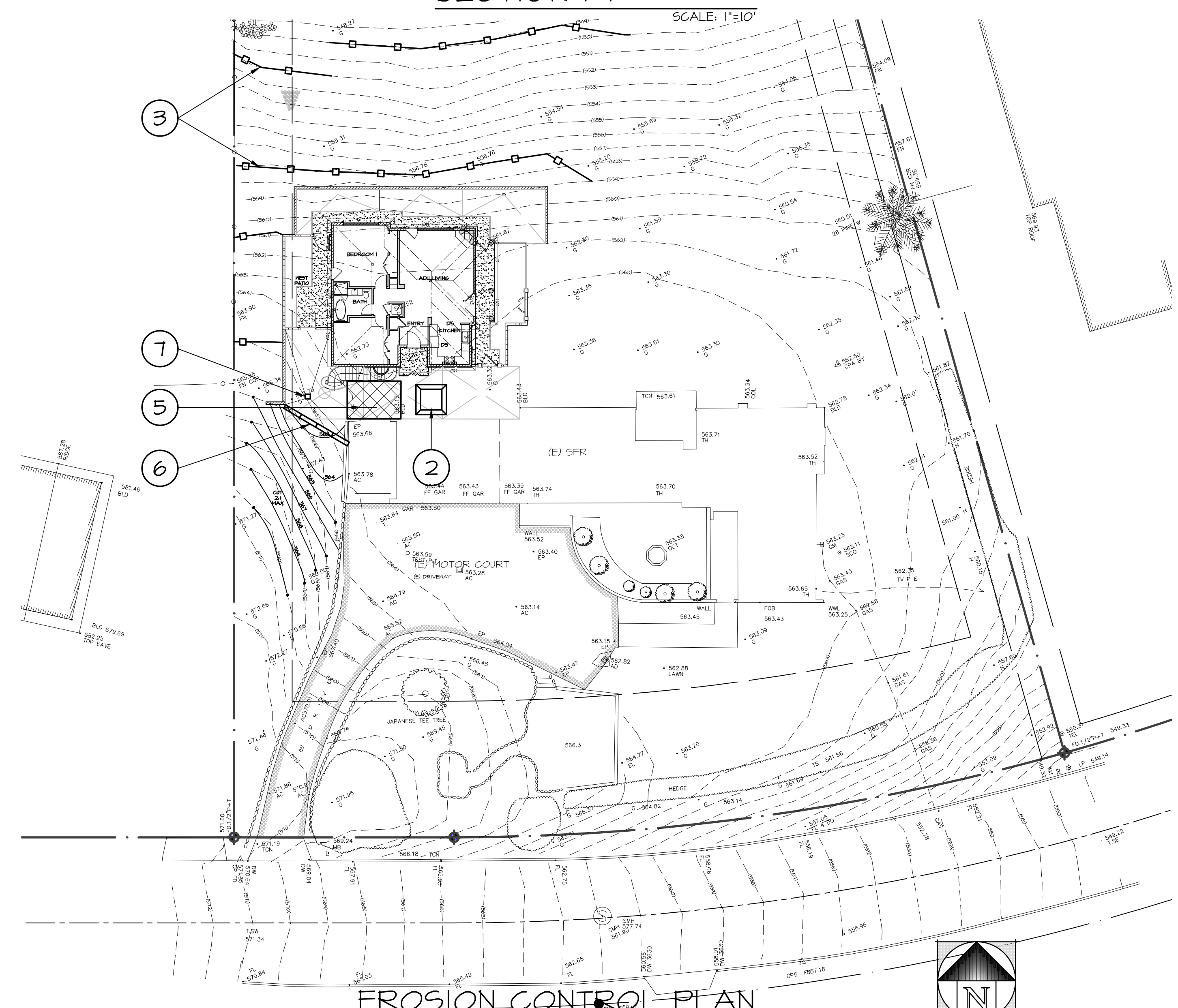
Limitations

- Not intended for use as mid-slope protection on slopes greater than 1:4 (V:H).
- Must be maintained.
- Must be removed and disposed of.
- Don't use below slopes subject to creep, slumping, or landslides.
- Don't use in streams, channels, drain inlets, or anywhere flow is concentrated.
- Don't use silt fences to divert flow.

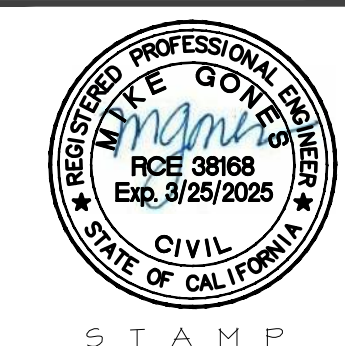
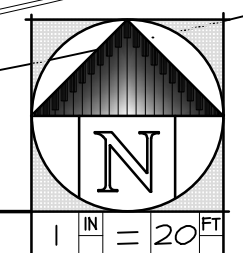
California Storm Water Quality Handbook
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Silt Fence SC-1
1 of 1

7 SEDIMENTATION BASIN **N.T.S.**

3 SILT FENCE **N.T.S.**



EROSION CONTROL PLAN

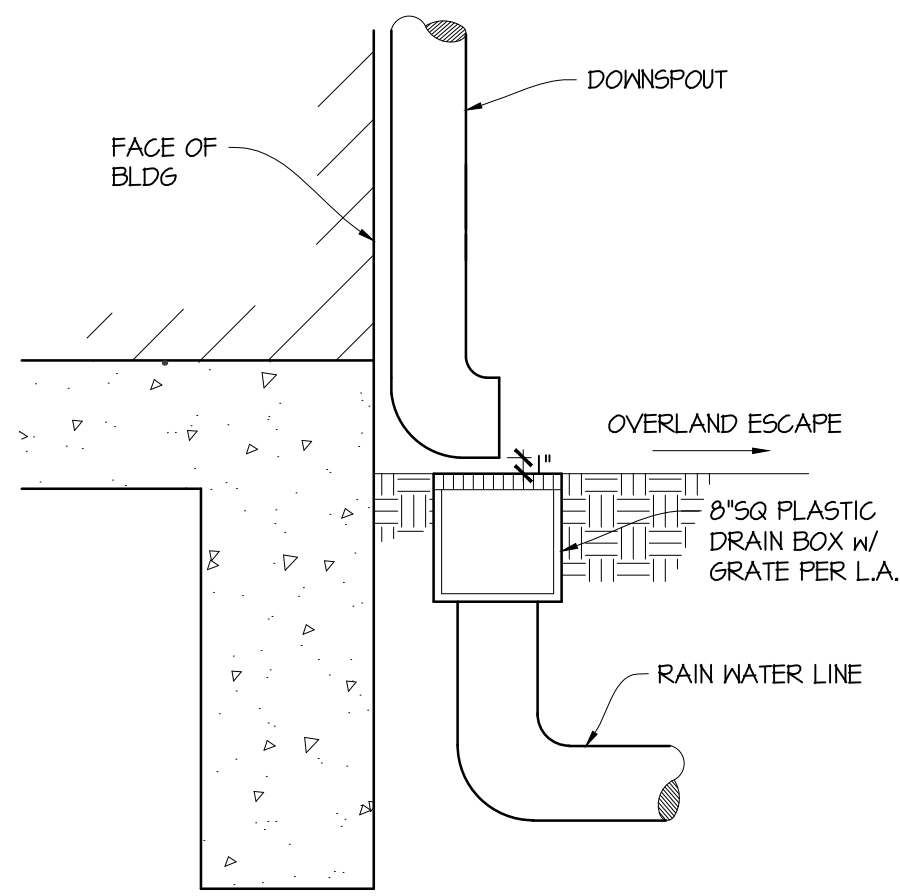


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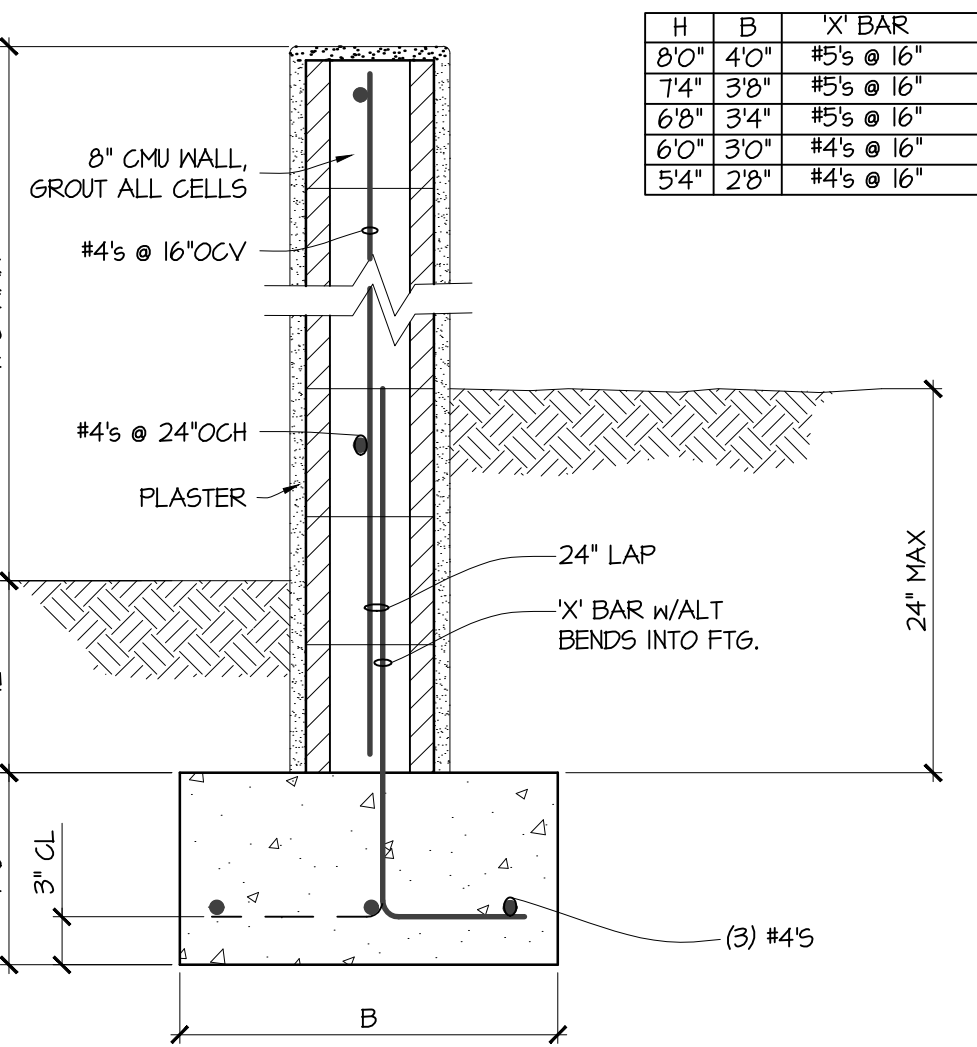
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Drawings by EG
DATE: 6/30/2023
REVISIONS:

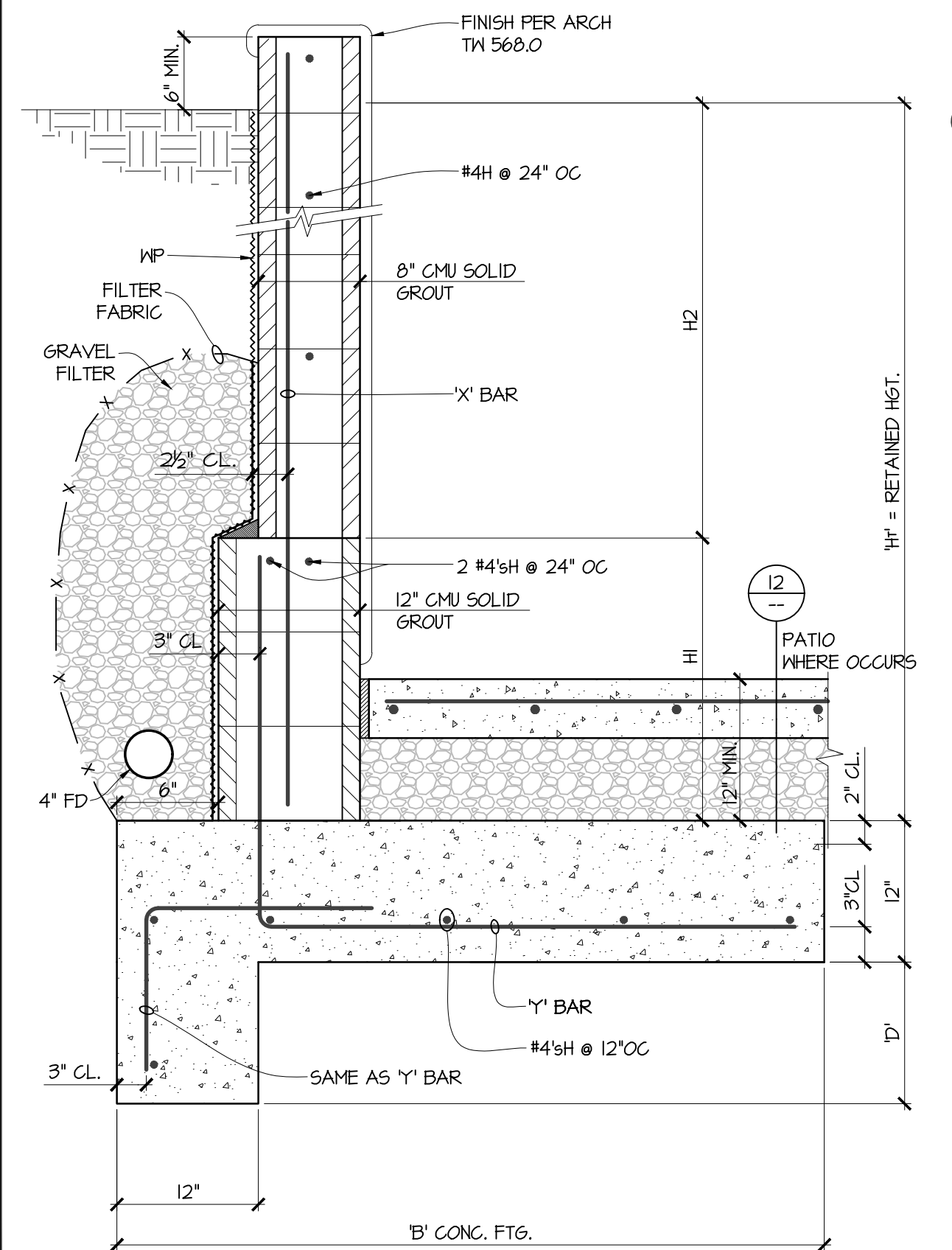
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SHEET 5 of 7



17 DS TO CATCH BASIN 1"=1'-0"

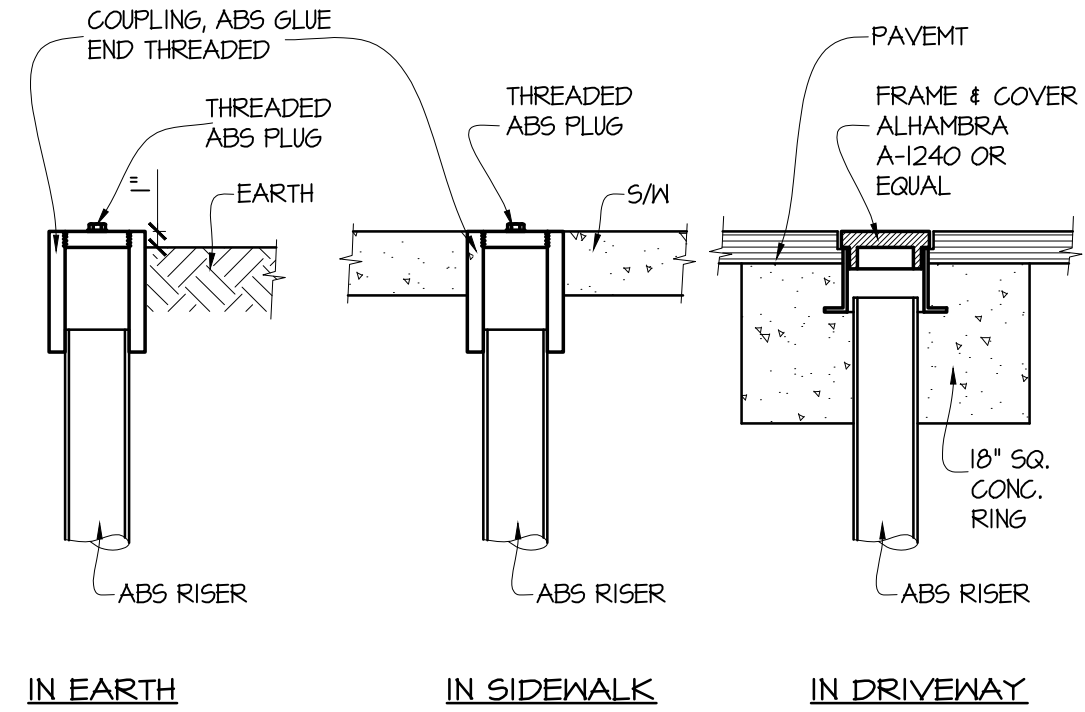


18 PRIVACY WALL 1"=1'-0"

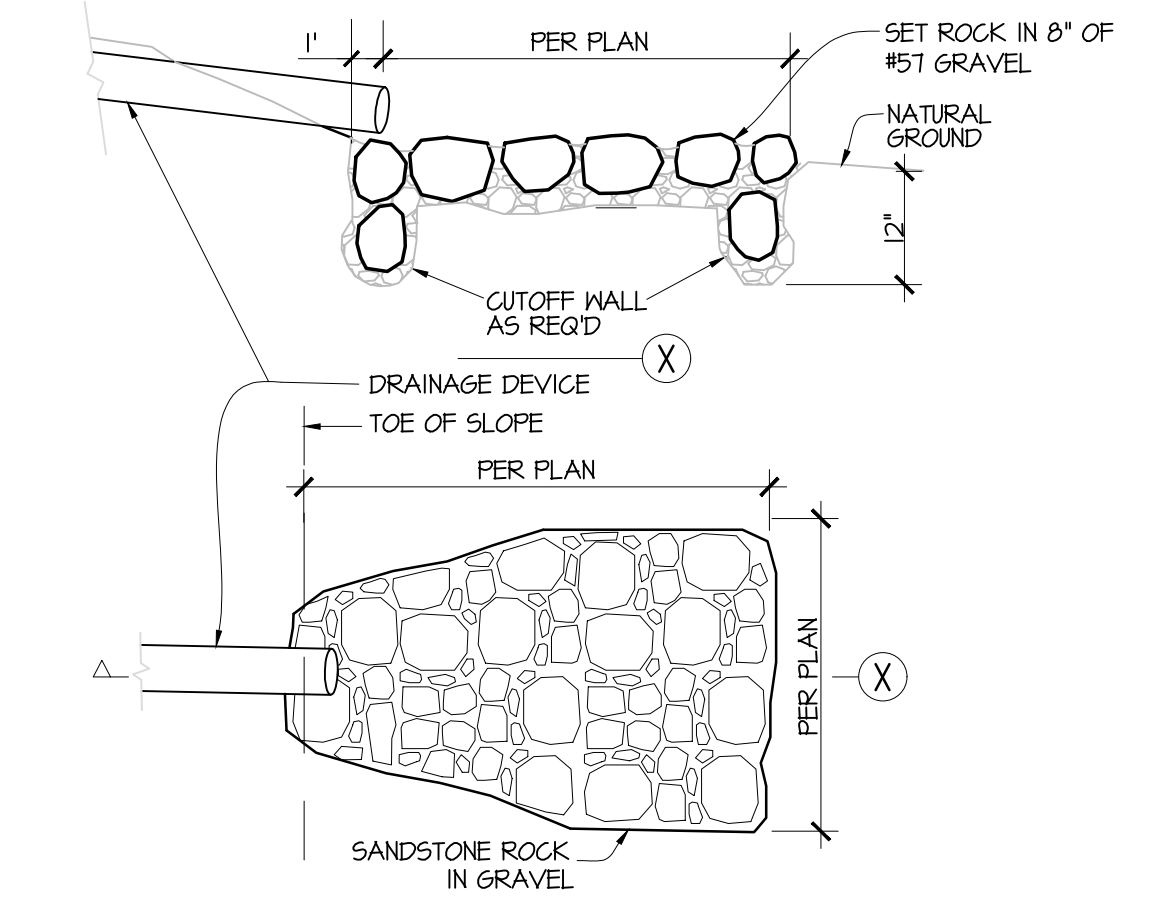


H	H1	H2	X BAR	Y BAR	B	D
6'0"	2'0"	4'0"	#4 @ 16"	#5 @ 16"	4'0"	13"
5'4"	2'0"	3'4"	#4 @ 16"	#5 @ 16"	3'9"	10"
4'8"	--	4'8"	#4 @ 16"	#5 @ 16"	3'6"	10"
4'0"	--	4'0"	#4 @ 16"	#5 @ 16"	3'3"	10"
3'4"	--	3'4"	#4 @ 16"	#5 @ 16"	2'9"	--

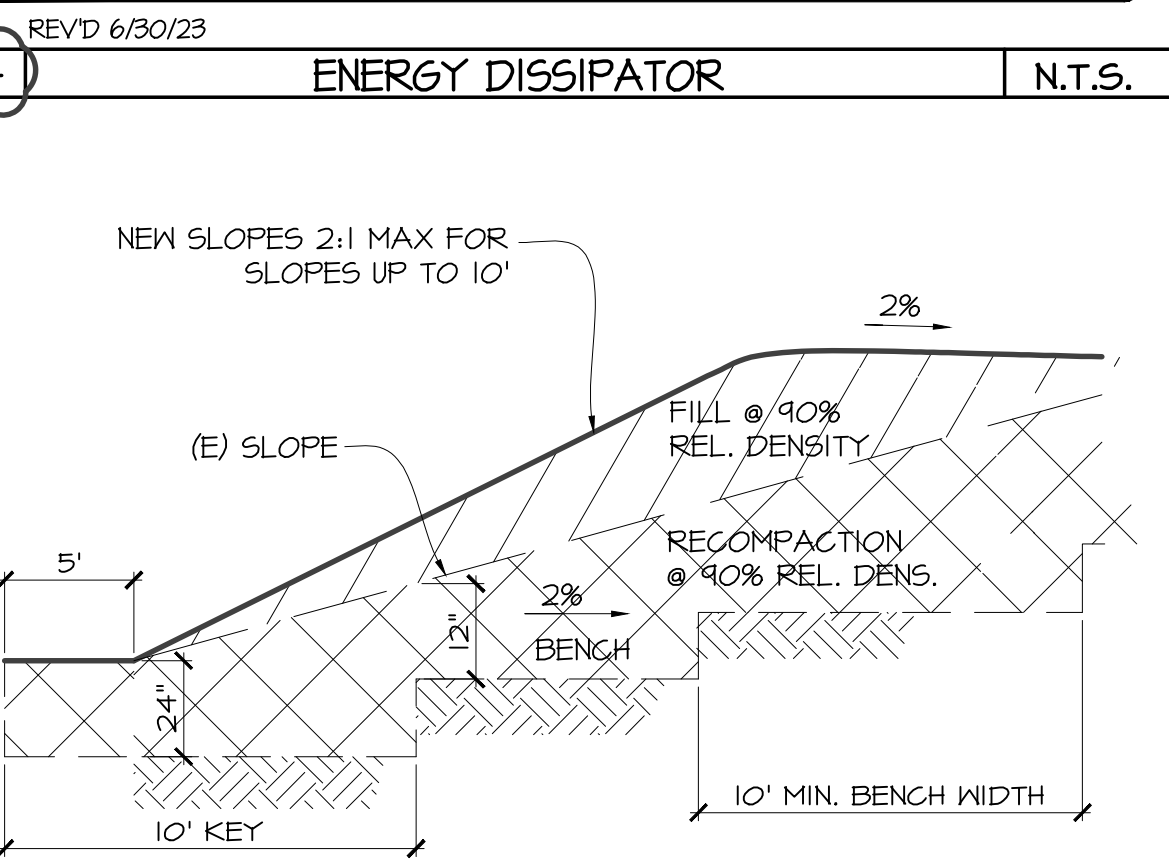
20 RETAINING WALL 1"=1'-0"



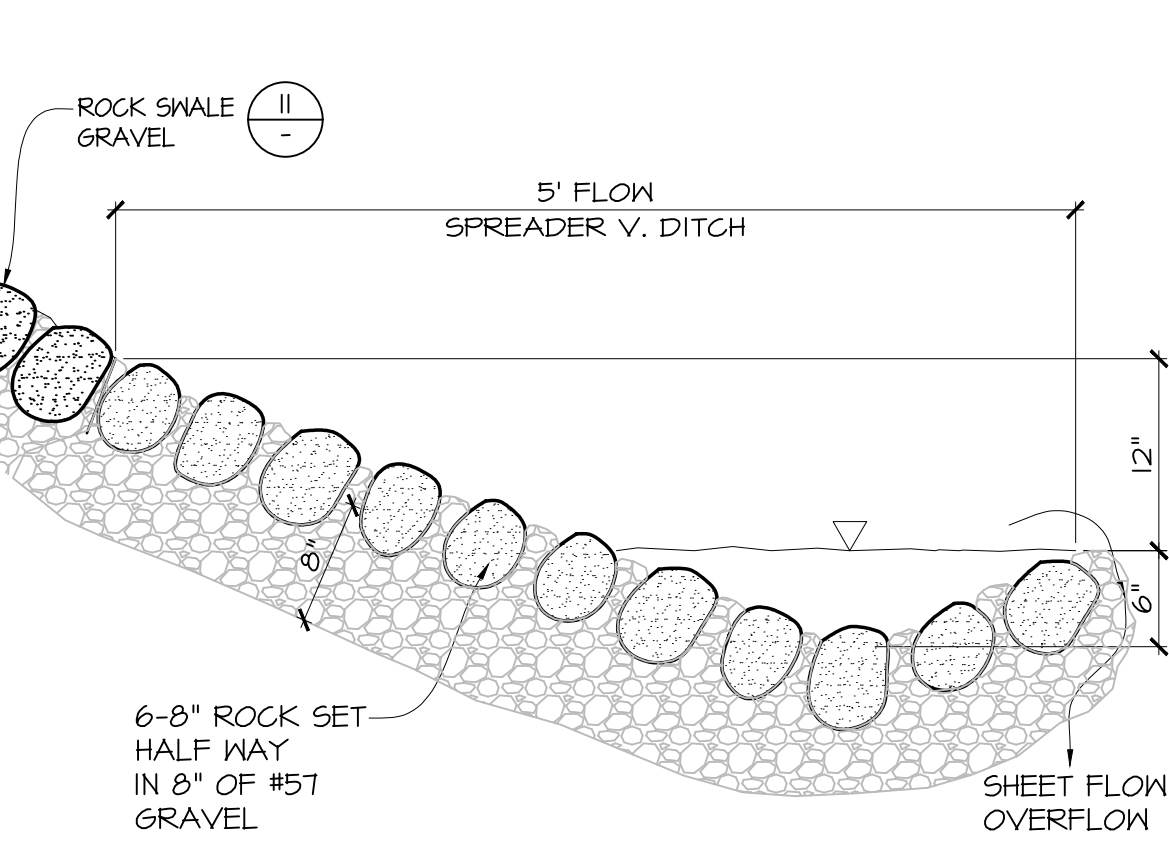
13 CLEANOUT DETAIL 1"=1'-0"



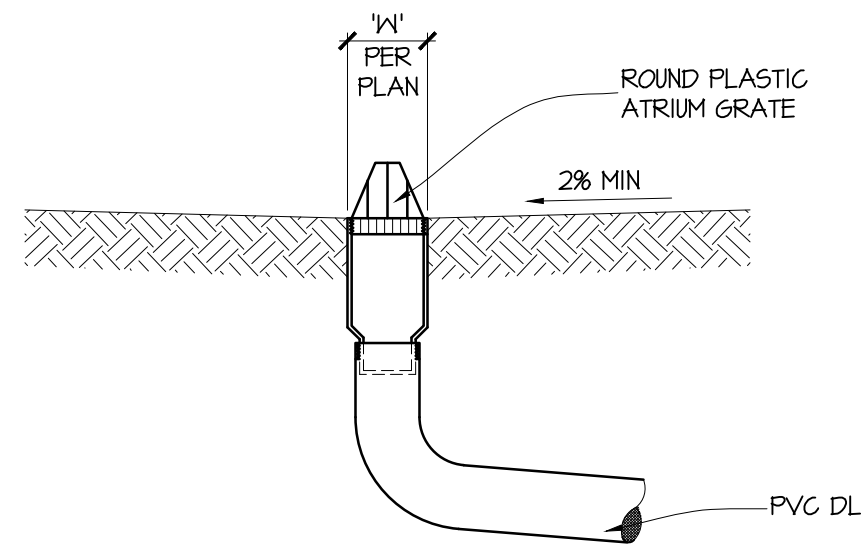
14 ENERGY DISSIPATOR N.T.S.



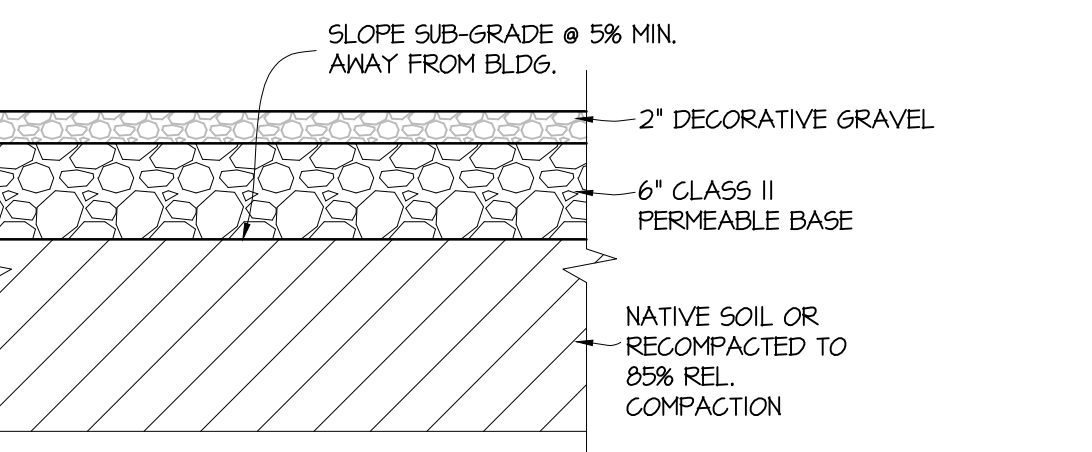
15 KEY & BENCH DETAIL N.T.S.



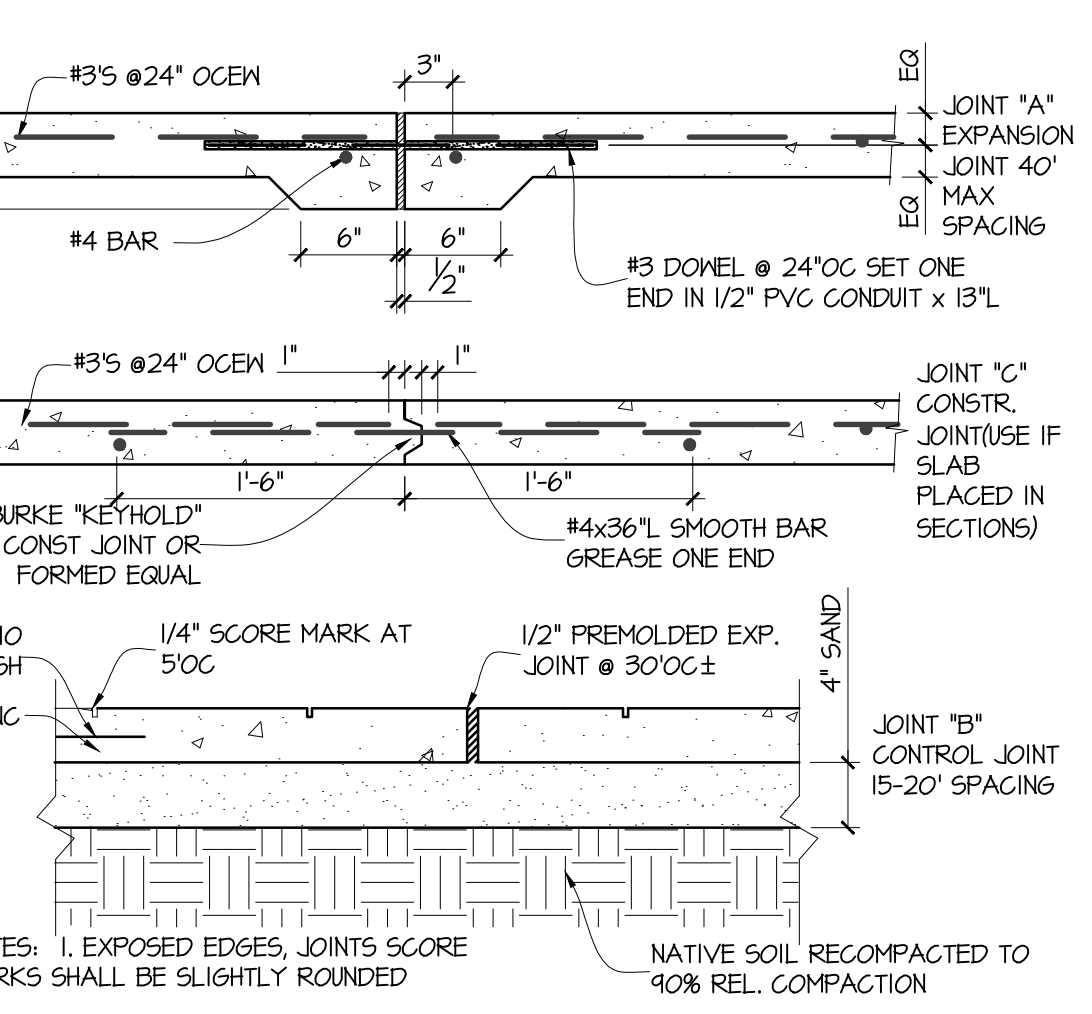
16 FLOW SPREADER V. DITCH 1"=1'-0"



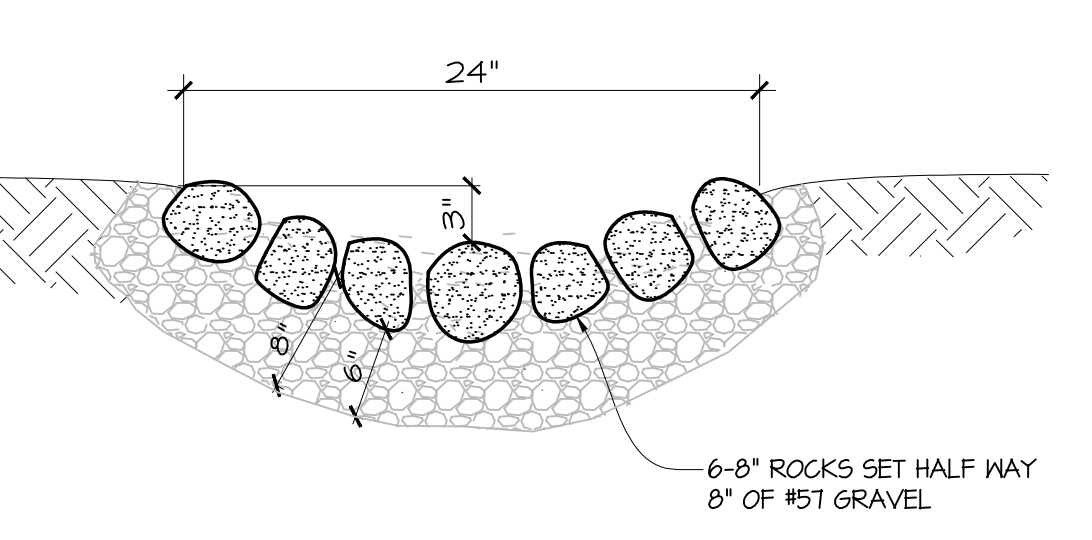
8 PLANTER DRAIN 1"=1'-0"



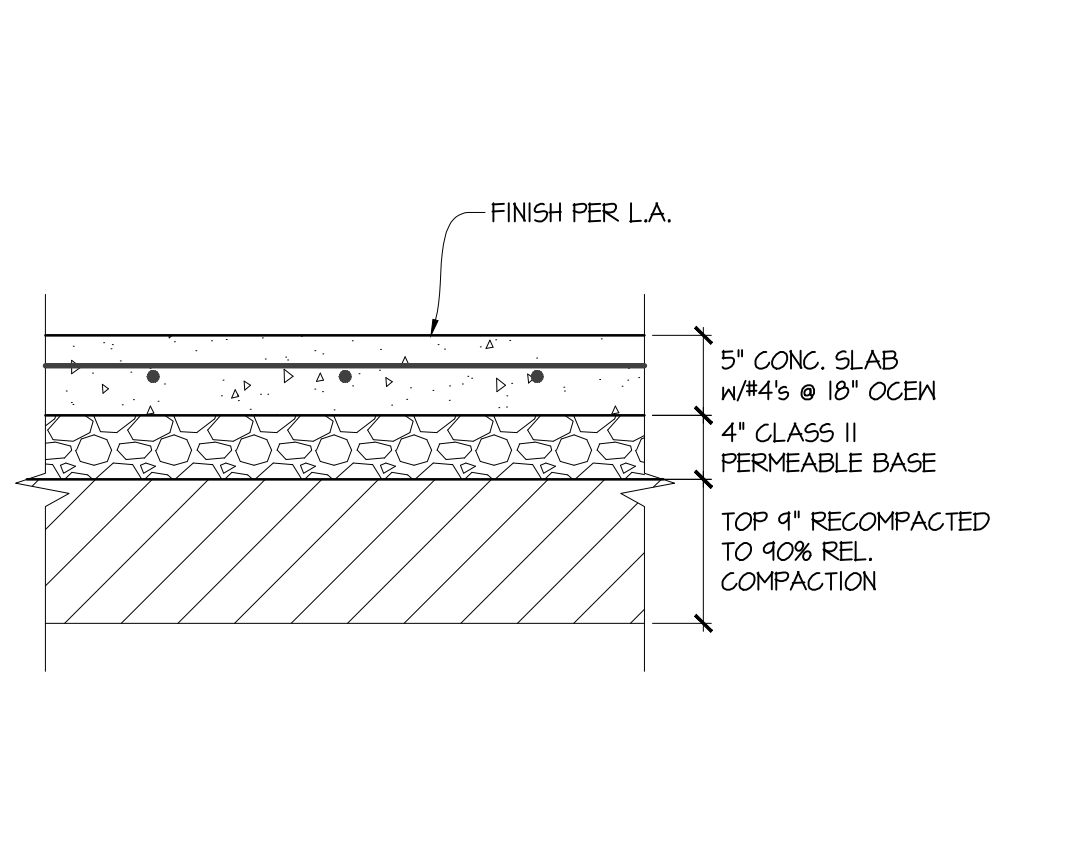
9 GRAVEL WALK N.T.S.



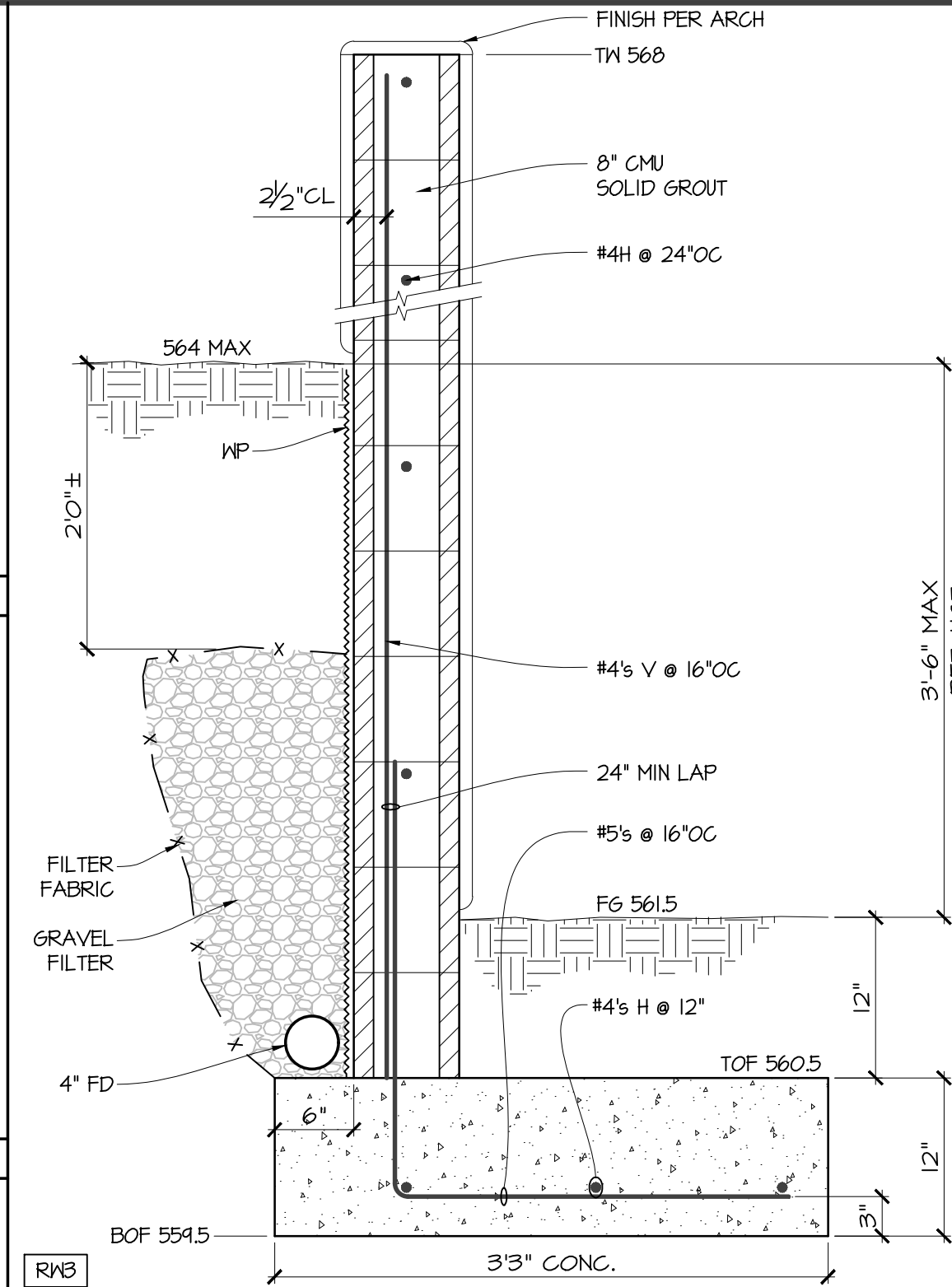
10 PAVEMENT JOINTS 1"=1'-0"



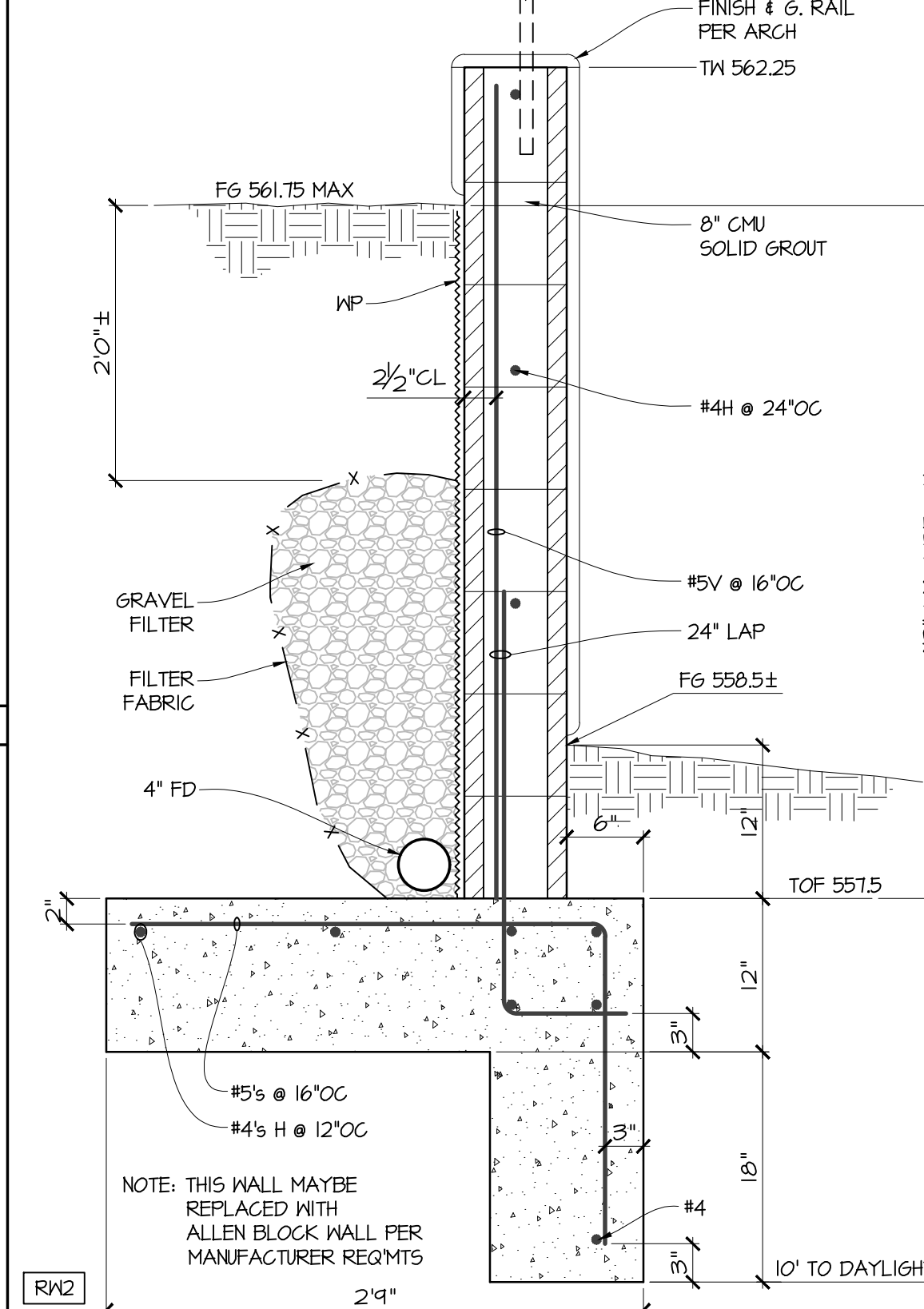
11 ROCK SWALE 1"=1'-0"



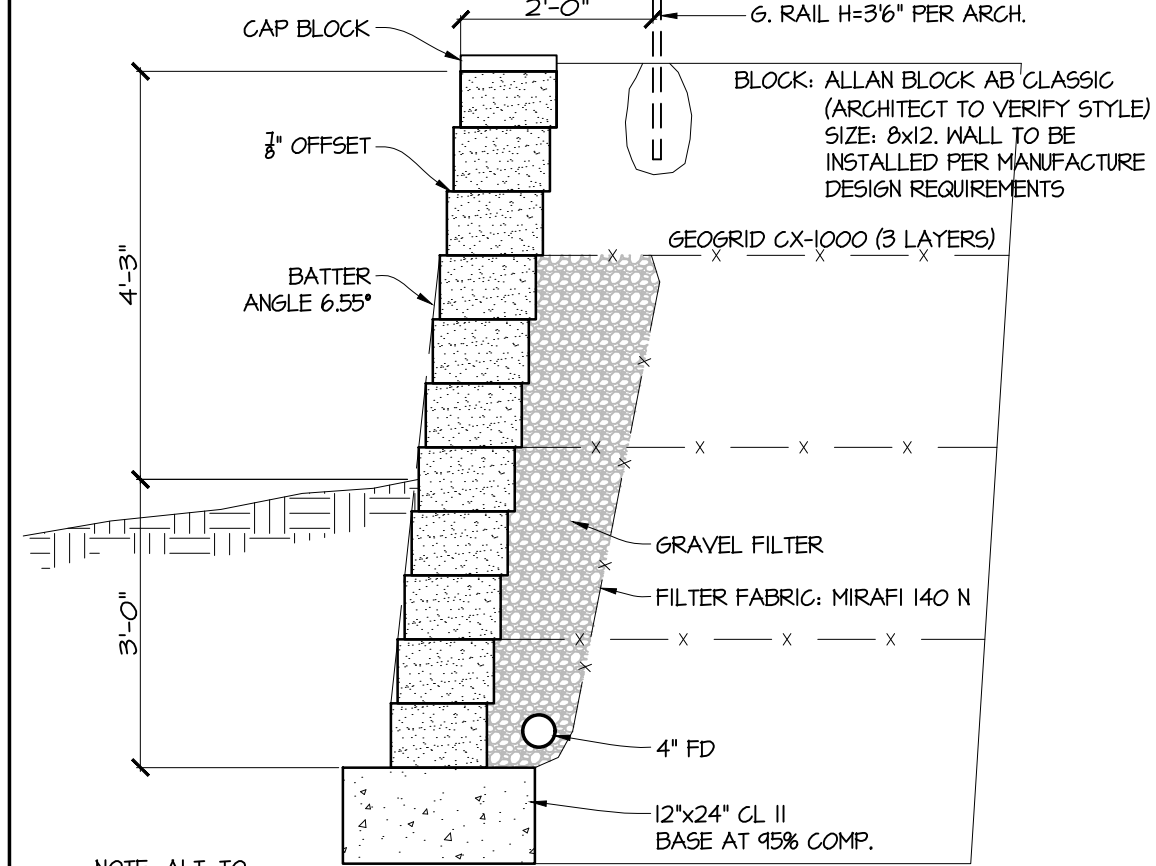
12 CONCRETE WALKWAY/PATIO 1"=1'-0"



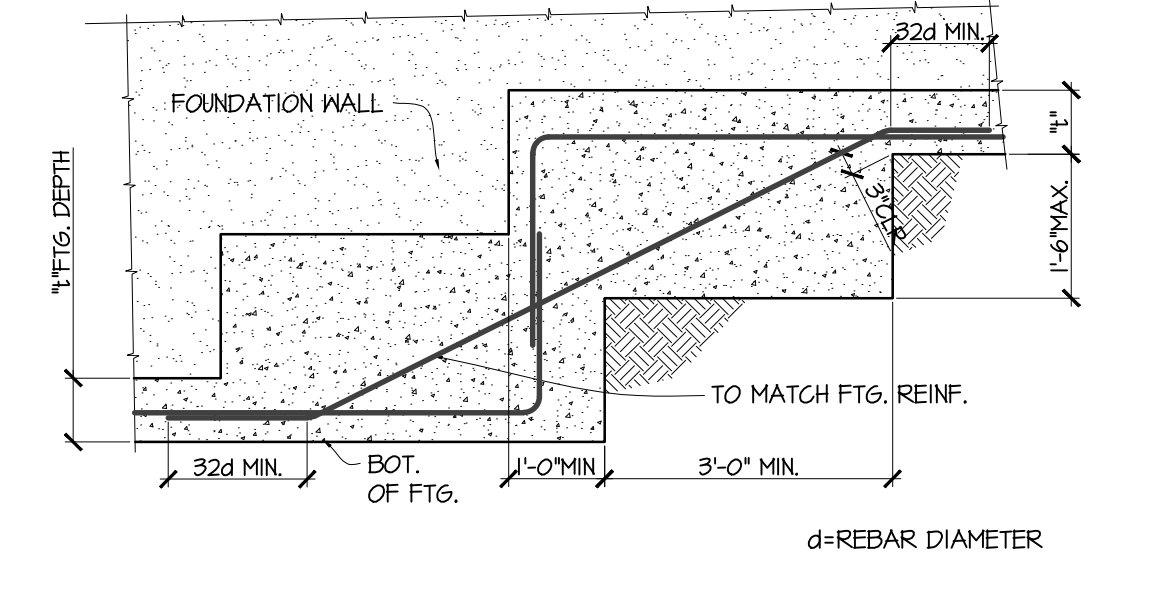
5 PRIVACY WALL 1"=1'-0"



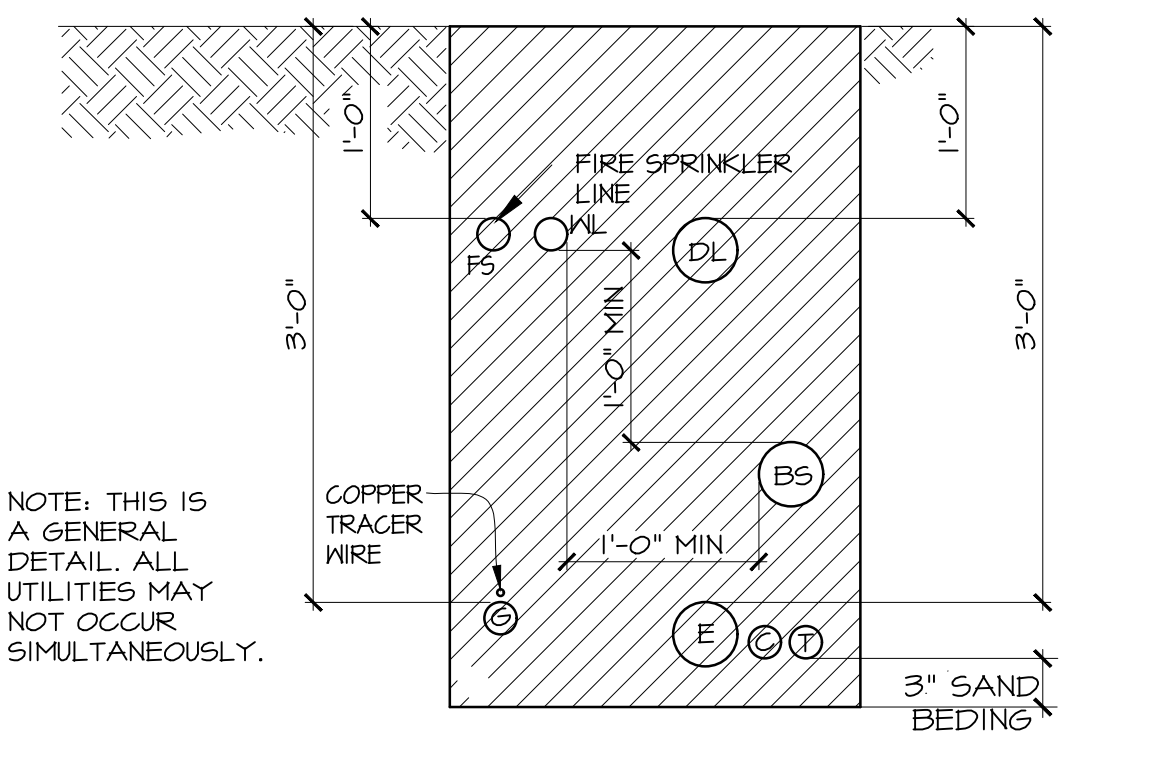
6 RETAINING WALL 1"=1'-0"



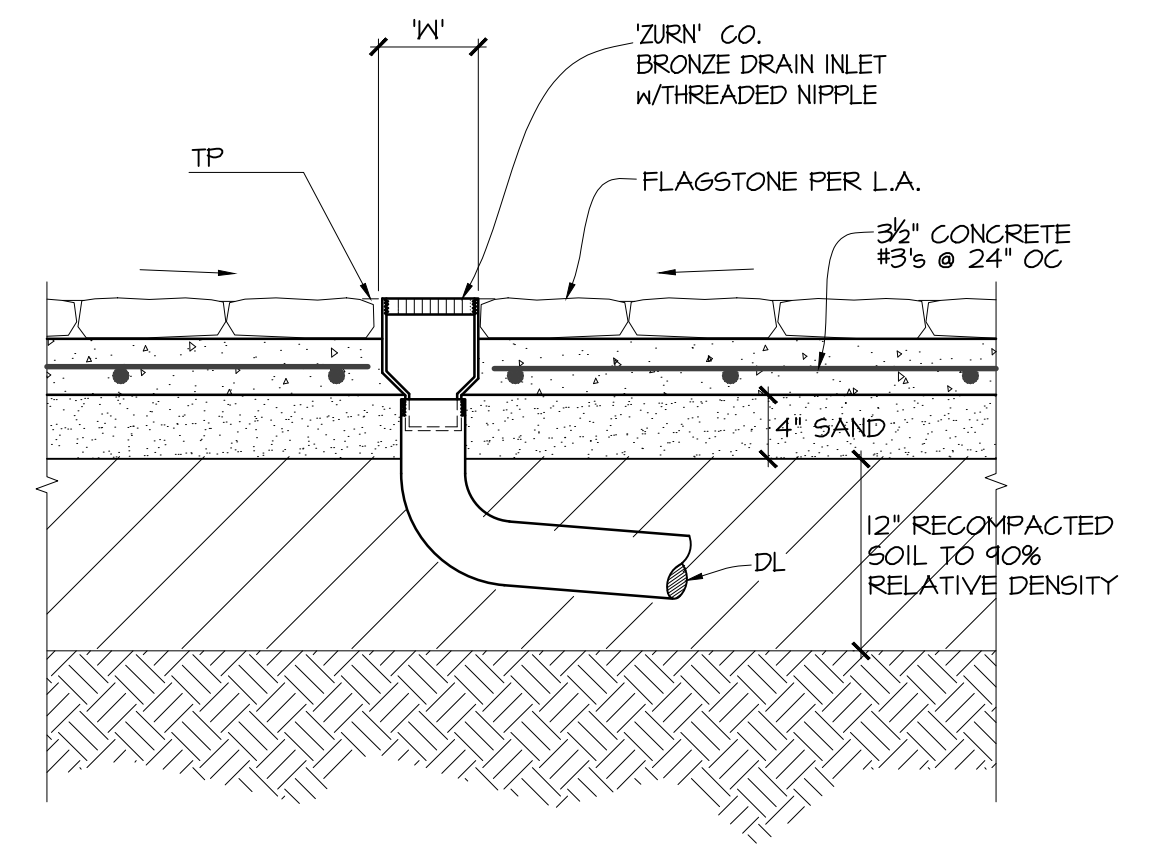
7 SEGMENTAL RETAINING WALL 1/2"=1'-0"



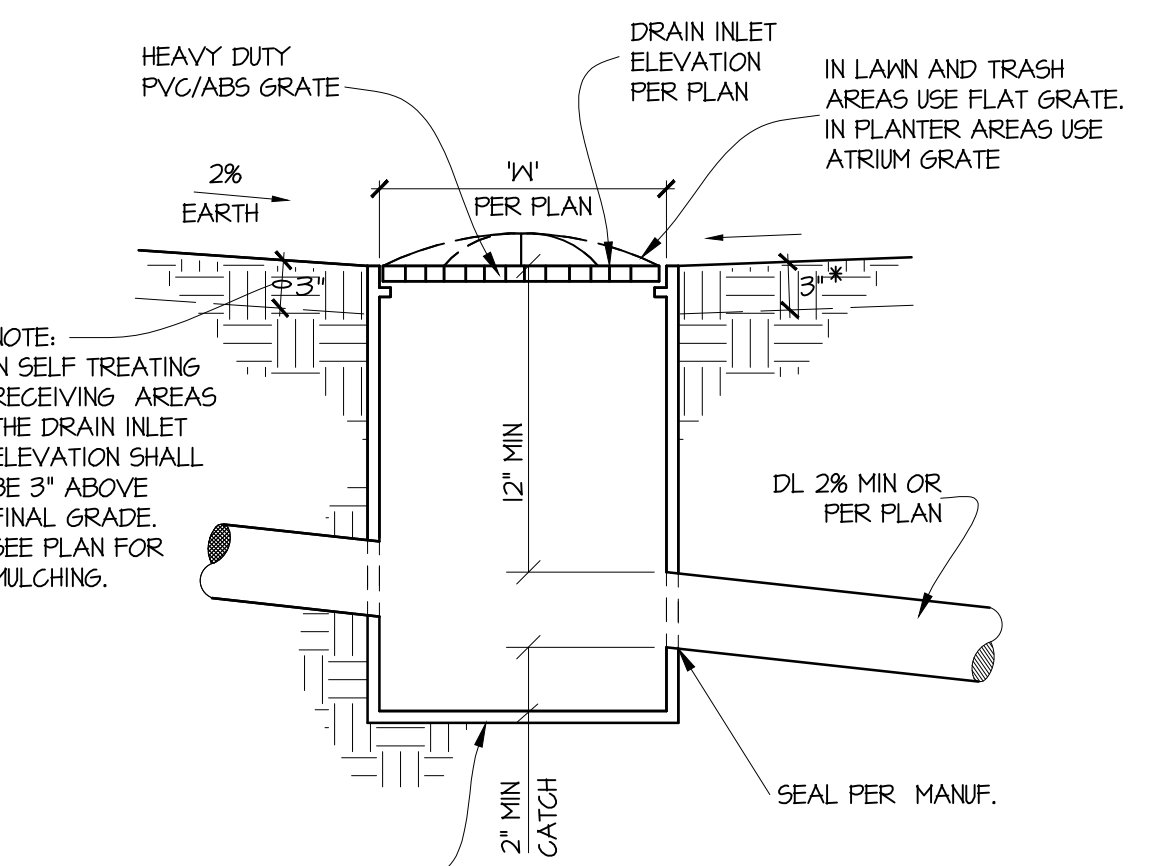
1 STEPPED FOOTING N.T.S.



2 UTILITY TRENCH 1"=1'-0"



3 PATIO DRAIN 1"=1'-0"



4 YARD DRAIN 1"=1'-0"

REGISTERED PROFESSIONAL ENGINEER
MIKE GONES
 CIVIL ENGINEER
 No. 18818
 1219 1/2 LACINA ST.
 SANTA BARBARA, CA 93101
 Ph: (805) 966-2250
 Fax: (805) 966-3800
 mikegones@cscc.net

KISLAK ADU
 3627 CAMPANIL DRIVE
 SANTA BARBARA, CA 93109

Drawings by EG
 DATE: 6/30/2023
 REVISIONS:
 JOB No: 22712
C4.0
 SHEET 6 of 7

RainHarvest Systems

4475 Alicia Lane
Cumming, GA 30028
770-889-2533
Sales@RainHarvest.com

Rainwater Harvesting Systems
Submittal For:

RainFlo 5100 IG Complete Rainwater Harvesting System

NOTES: FOR RAIN HARVESTING SYSTEM

- INFO SHOWN HERE IS FOR GENERAL PERMIT PURPOSES. GO TO WWW.RAINHARVEST.COM FOR FULL BROCHURE.COM FOR FULL BROCHURE, DETAILS & INSTALLATION REG'S.
- CONTRACTOR MAY ELECT TO SUBSTITUTE COMPARABLE SYSTEM IF APPROVED IN ADVANCE BY THE PROJECT ENGINEER.
- THIS SYSTEM IS ANTICIPATED TO REQUIRE 1-1250 GAL. TANKS.
- ALL SYSTEM DETAILS TO BE WORKED OUT IN ADVANCE WITH PROJECT ENGINEER AND LANDSCAPE ARCHITECT PRIOR TO ORDERING AND INSTALLING.
- SEE SHEET G.I. PLAN FOR CONNECTION TO IRRIGATION SYSTEM. FINAL PUMP SIZE TO BE SELECTED BASED ON PLANTING PLAN & IRRIGATION DEMAND.

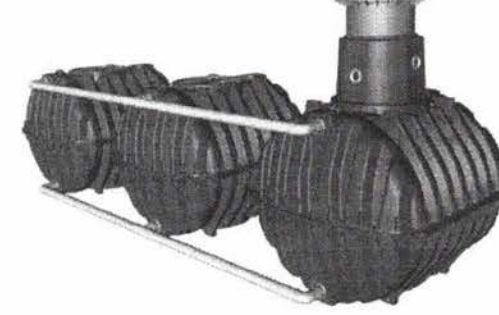


Graf Carat-S Rainwater Tanks

The new generation of Carat rainwater underground tanks has been specially developed for rainwater harvesting.

The Carat-S Underground Tank:

The lineup consists of four modular tank units ranging from 700 gallons to 1,700 gallons which are expandable up to thousands of gallons. The precision, modular, and ultra-high strength design of the Carat-S makes it the choice of professionals worldwide. The Graf Carat tank is guaranteed not to collapse when empty when installed per the manufacturer's specifications.



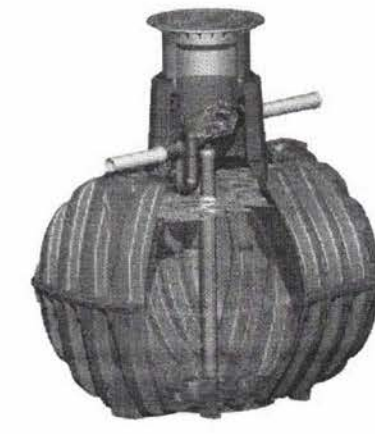
Carat-S Rainwater Tank Features:

- 15 Year manufacturer's warranty
- Suitable for vehicle loading (when combined with the cast iron lid option)
- Variable installation depth with double-sealed telescopic riser
- Convenient 31-1/2" manway opening
- Attractive locking green lid
- Internal pre-filtration option
- Easy to transport and install
- Ultra-high strength materials and design
- Frost-proof installation underground
- Groundwater stable to the middle of the tank

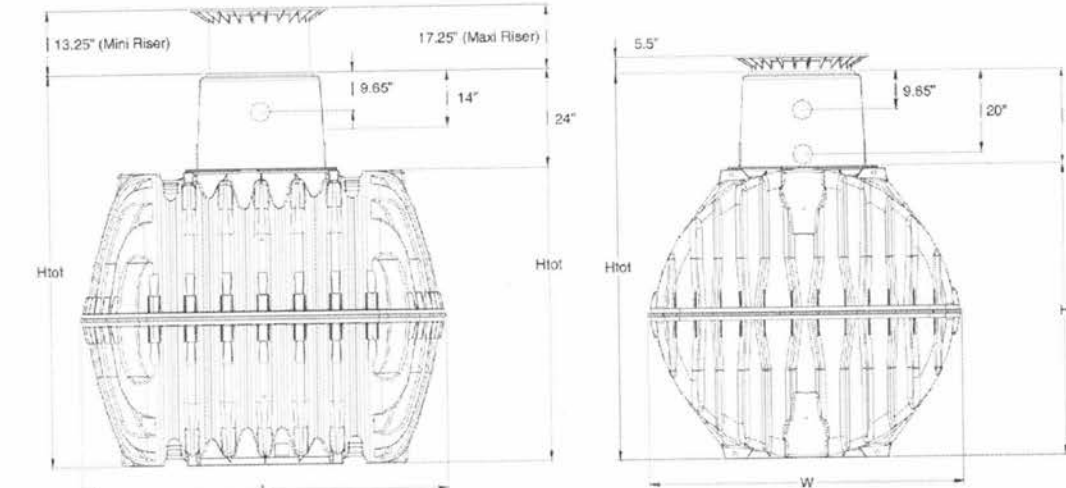
Internal Filter Package Option:

Specially designed for rainwater harvesting, the Graf Optimax Pro® internal self-cleaning filter uses patented filter technology to filter debris from roof areas up to 3,750 sq. ft.

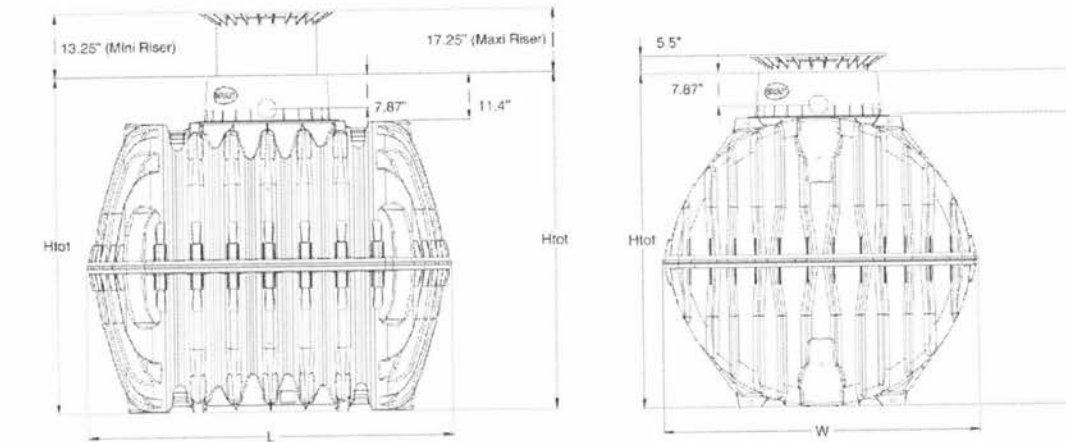
- Greatly simplified installation
- Only one manway and lid in the yard
- Provides over 95% water yield
- Self-cleaning
- Very low maintenance



Dimensions with Maxi Tank Dome:



Dimensions with Mini Tank Dome:



Tank	700 US Gallons	1000 US Gallons	1250 US Gallons	1700 US Gallons	1700 US Gallons (Expansion Tank)
Part No.	372001	372002	372003	372004	372014
Weight	265 lb.	331 lb.	408 lb.	485 lb.	485 lb.
L	82"	90"	90"	94"	94"
W	62"	69"	78"	86"	86"
H	55"	62.5"	71.5"	82.5"	82.5"
Htot*	79"	86.5"	95.5"	106.5"	NA
Htot**	66"	73.5"	82.5"	93.5"	NA

*Htot = total height **with Mini Tank Dome. Deeper burial depths can be achieved using optional extension rings.

	Optimax® Pro Internal Filter	Part Number 340037
US Adaptation by RainHarvest Systems		
<p>Dimensions:</p>		
<p>Overflow Siphon (with tank pkg. only)</p>		
<p>OTTO GRAF GMBH Lusthofweg 2 9331 Teningen</p>	<p>Carl-Zeiss-St. 2-6 Telefon 07641-689-0 Telefax 07641-689-50 E-Mail: info@graf-online.de www.graf-online.de</p>	<p>RainHarvest Systems, LLC 6075 Parkway North Drive Cumming, GA 30040 Tel: 770-889-2533 Fax: 770-889-2577 www.RainHarvest.com</p>

Internal filter technology

Optimax Pro, self-cleaning filter

Advantages

- Provides over 95% water yield
- Low maintenance (self-cleaning)
- Space-saving filter technology inside the tank
- Filter housing can be easily removed without tools
- Transparent lid for filter visibility
- Can manage roof areas up to 350 sqm
- Standard 100 mm connections
- Self-cleaning Opticlean system available as an optional extra
- Low offset height of 165 mm between inlet and outlet

Optimax-Pro Filter Internal
Order no. 340037

Accessories

Cleaning unit Opticlean® internal
without hose
Order no. 340040

Quick assembly sleeve Spanfix
patented (Pat. 47)
Order no. 340502

XL lift out device
for a convenient withdrawal of filter cover and filter insert, length 505 mm
Order no. 330220

Cleaning unit Opticlean®

- Very intense water jet for cleaning the filter sieve
- Routine maintenance intervals are kept to a minimum
- An automatic activation of the cleaning unit is carried out together with the automatic filter cleaning unit and the Aqua-Center-Silentio

Filter cartridge

Very smooth surface and, therefore, max. self-cleaning, mesh width 0.35 mm (0.01")

3-layer filter assembly

Collector surface
Diamond pattern as carrying surface

Stainless steel fine filter

Connecting dimensions for telescopic dome shaft

Dome shaft	Tank overflow	Emergency overflow
Mini	495-495 mm	660-860 mm
Mini	19.9-27.4"	25.9-33.9"
Maxi	400-700 mm	660-860 mm

Q: Webcode G2102

RAINFLO® Universal Rainwater Pumps

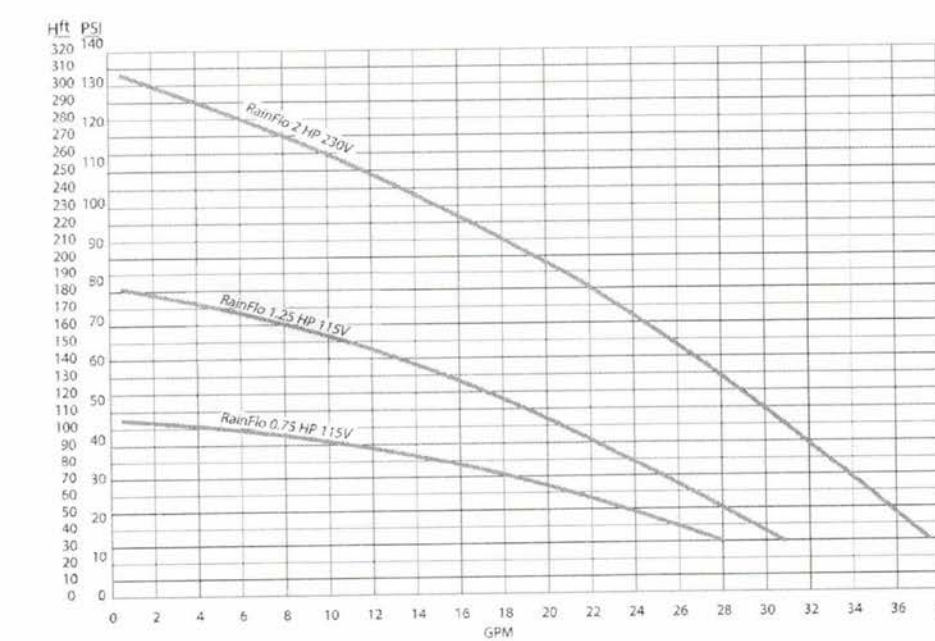
High performance multi-stage rainwater pumps for residential, commercial, and light-industrial rainwater collection systems.

Submersible and External Mounted Capability:

RainFlo universal pumps are specially designed for the unique requirements of rainwater collection systems. Equipped with a large threaded bottom inlet large for internal flow-based cooling and connection to a floating filter, these pumps can be installed vertically or horizontally and they can either be submersed inside a tank or mounted externally on the ground or other platform. Other features include a stainless steel base, adjustable float switch for run-dry protection, external capacitor housed in a wiring box with circuit breaker and master on/off switch for long life and ease of maintenance.



Pump Performance:



RainHarvest Systems | www.RainHarvest.com | 800-654-9283

Durable, Dependable and High Performance:

Available in 0.75HP/115V, 1.25HP/115V and 2.0HP/230V models, construction consists of 304 stainless steel housings, dual Italian mechanical seals, American thermal protection, GE-Noryl diffuser and impellers, external starting capacitor and a 45 foot power cord. The pump is particularly quiet and durable from its solid construction. The water end is installed under the motor which keeps the motor cooled with the pumped water. The Noryl impellers and diffusers offer high abrasion resistance while the Italian double mechanical seals ensure long life and enhanced reliability.

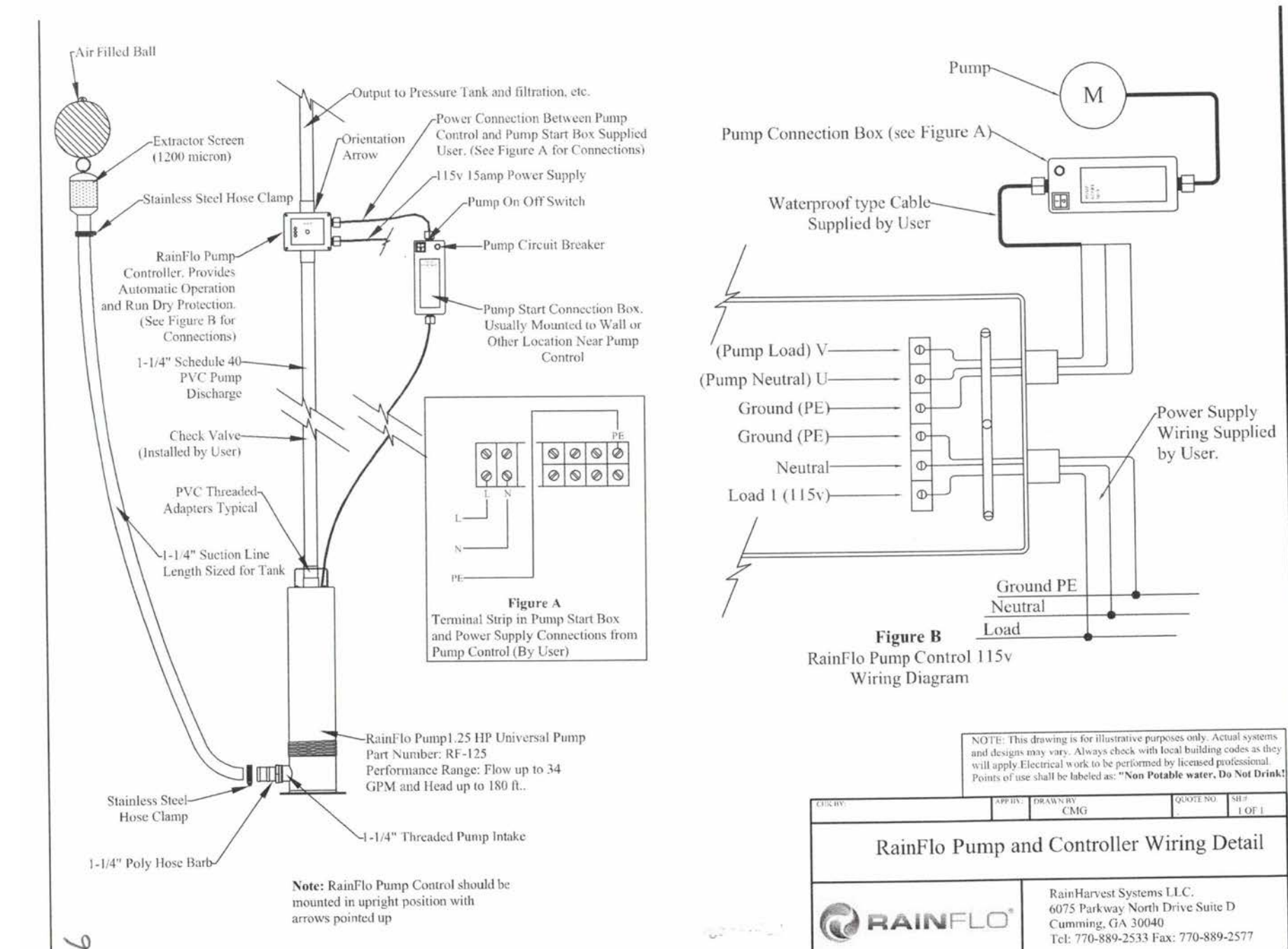
The oil chamber is filled with non-toxic cooling oil. Ball bearings are self-lubricating and internal cast iron components are electrocoated with polybutadiene varnish to prevent corrosion which is sometimes associated with the typical lower pH of rainwater.

Installation may be oriented either vertical or horizontal so long as water is available at the intake to prevent a run-dry condition.

Specifications:

RainFlo Submersible Pumps			
Model No.	RF075-S	RF125-S & SC	RF200-S
Horsepower:	0.75	1.25	2.0
Nominal Voltage Range:	115V, 60Hz, 8.6A max.	115V, 60Hz, 14A max.	230V, 60Hz, 9A max.
PHW:	1.0	1.24	2.3
P2KW:	0.6	0.95	1.6
Impeller stages:	2	3	5
Maximum flow:	29 GPM	34 GPM	36 GPM
GPM at 50 psi (0 Head):	See Curve	17 GPM	29 GPM
GPM at 40 psi (0 Head):	8 GPM	22 GPM	31 GPM
Maximum head:	105' TDH	180' TDH	310' TDH
System pressure:	Up to 46 PSI	Up to 78 PSI	Up to 135 PSI
Inlet/Outlet size:	1-1/4" FPT	1-1/4" FPT	1-1/4" FPT
Weight:	36 Lbs.	41 Lbs.	46 Lbs.
Dimensions:	7" X 7" X 20" (incl. base)	7" X 7" X 22" (incl. base)	7" X 7" X 24" (incl. base)
Thermal protection:	Yes	Yes	Yes
Motor:	2-pole induction, Continuous duty	Same	Same
RPM:	3450	3450	3450
Cooling:	Water cooled/intake	Water cooled/intake	Water cooled/intake
Insulation class:	F	F	F
Protection:	IP68	IP68	IP68
Certifications:	CE	CE	CE
Warranty:	1 Year	1 Year	1 Year

RainHarvest Systems | www.RainHarvest.com | 800-654-9283



MIKE GONIES
CIVIL ENGINEER
P.E. (0602) 986-2269
mikegonies@earthlink.net
REC. 08/08/09 ACTION, 07
SANTA BARBARA, CA 93101

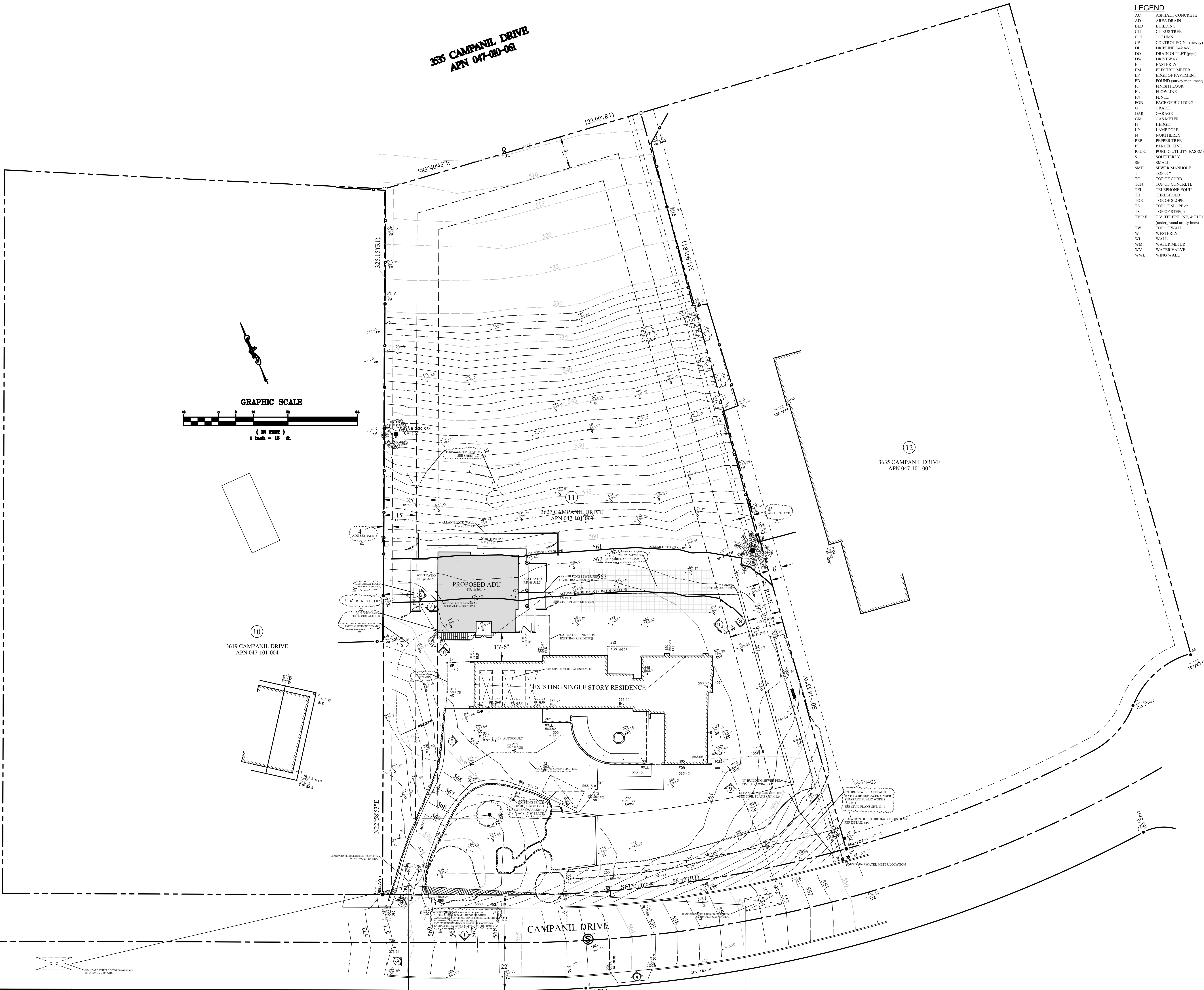
VISLAK ADU
3627 CAMPANIL DRIVE
SANTA BARBARA, CA 93109

Drawings by EG
DATE: 6/30/2023
REVISIONS:

JOB No: 22712

C5.0

SHEET 7 of 7

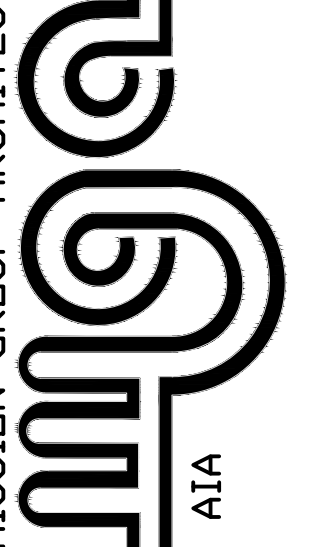


LEGEND

- AC ASPHALT CONCRETE
- AD AREA DRAIN
- BLD BUILDING
- CIT CITRUS TREE
- COL COLUMN
- CP CONTROL POINT (survey)
- DL DRIPLINE (60k trees)
- DO DRAIN OUTLET (pipe)
- DW DRIVEWAY
- E EASTERLY
- EM ELECTRIC METER
- EP EDGE OF PAVEMENT
- FD FOUND (survey monument)
- FF FINISH FLOOR
- FL FLOWLINE
- FN FENCE
- FOB FACE OF BUILDING
- G GRADE
- GAR GARAGE
- GM GAS METER
- H HEDGE
- LP LAMP POLE
- N NORTHERLY
- PEP PEPPER TREE
- PL PARCEL LINE
- P.U.E. PUBLIC UTILITY EASEMENT
- S SOUTHERLY
- SM SMALL SEWER MANHOLE
- T TOP OF
- TC TOP OF CURB
- TCN TOP OF CONCRETE
- TEL TELEPHONE EQUIP.
- TH THRESHOLD
- TOE TOE OF SLOPE
- TS TOP OF SLOPE or
- TS TOP OF STEIN
- TVPE T.V. TELEPHONE, & ELECTRIC (underground utility lines)
- TW TOP OF WALL
- W WESTERLY
- WL WALL
- WM WATER METER
- WV WATER VALVE
- WWL WING WALL

OVERALL SITE PLAN

SCALE : 1" = 16'-0"



MISSION GROUP ARCHITECTS
R E JOHNSON AIA

PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

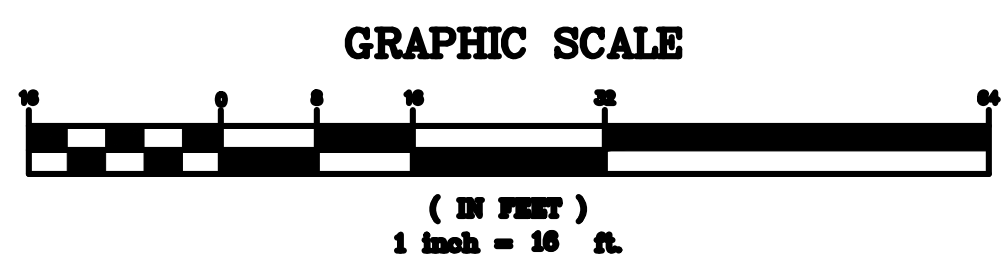
DATE
7/14/2023

SHEET
A-1

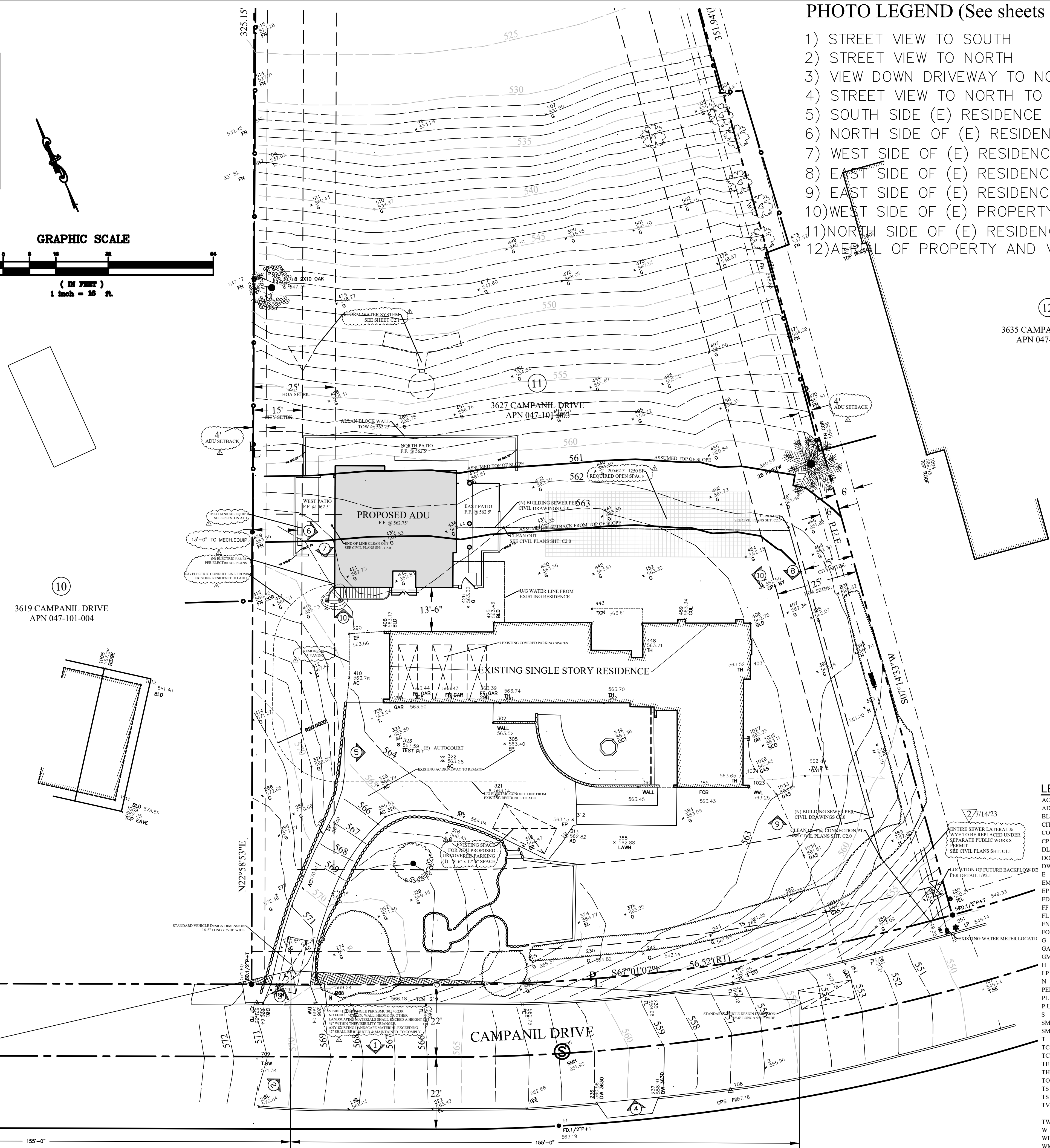
SITE PLAN

APPROVED ARCHITECT
SANTA BARBARA, CALIFORNIA 93108 805-569-6910
© 2023 MISSION GROUP ARCHITECTS

SAMSUNG		SUBMITTAL AC030BNHDCH/AA / AC030BXADCH/AA (CNH30HDB / CXH30ADB) Page 1 of 4	
Job Name		Location	
Purchaser		Engineer	
Submitted to		Reference <input type="checkbox"/> Approval <input type="checkbox"/> Construction <input type="checkbox"/>	
Unit Designation		Schedule #	
Specifications			
Model	Indoor Unit Model Number (US Code)	AC030BNHDCH/AA (CNH30HDB)	
	Outdoor Unit Model Number (US Code)	AC030BXADCH/AA (CXH30ADB)	
Performance	Nominal Capacity (Cooling) (Btu/h)	30,000 / 32,000	
	Capacity Range (Cooling) (Btu/h)	8,600 - 34,000	
	SEER / EER	7.800 / 41.000	
	COP (nominal heating)	17.7 / 13.3	
Power	Voltage (V) / Hz	1 / 208-230 / 60	
	Working Voltage Range (VAC)	176 - 254 (max. 3% deviation from each)	
	Operating Current (Cooling) (A)	3.5 / 13.3 / 17.2	
	(min. / std. / max.) Heating (A)	3.1 / 13.0 / 24.0	
Dimensions	W X H X D (in.)	47 1/4 X 13 1/8 X 27 9/16	
	Indoor Unit	37 X 39 5/16 X 13	
	Outdoor Unit	48 5/16 X 8 1/16	
	Sturdy (in.)	45 5/16 X 8 1/16	
Sound Pressure Level	Indoor Unit dBA (L / M / H)	33 / 32 / 34	
	Outdoor Unit dBA (Cooling / Heating) (high)	50 / 52	
	Operating Temperatures	Outdoor Cooling: 23° - 122°F (5° - 50°C)	
	Heating: 0° - 122°F (-18° - 50°C) w/ buffer		
Pipe Connections	Maximum (R)	3/8"	
	Maximum Vertical Separation (R)	84"	
	Condensate Connection (with indicated adapter)	1 1/16" ID for 3/4" PVC	
	Refrigerant	Type: R410A	
Compressor	Control Method	Electronic Expansion Valve	
	Factory Charge (lbs)	5.7	
	Charged for	24.6 lb	
	Additional Refrigerant	0.11 lbs/over 24.6 lb	
Evaporator Fan	Manufacturer	Samsung	
	Type	Inverter Driven, Twin BLDC Rotary	
	RLA	18.2	
	Additional Refrigerant	0.11 lbs/over 24.6 lb	
Condenser Fan	Type	BLDC With Axial Type Fan (1)	
	FLA / Watts / CFM (max.)	1.25 A / 125 W / 2,884 CFM	
	Motor	BLDC With Axial Type Fan (1)	
	FLA / Watts / CFM (max.)	1.25 A / 125 W / 2,884 CFM	



ECO2 SYSTEMS		SUBMITTAL : GS4-45HPC & SAN-43SSAQA 43 Gallon Tank	
Job Name		Location	
Purchaser		Engineer	
Submitted to		Reference <input type="checkbox"/> Approval <input type="checkbox"/> Construction <input type="checkbox"/>	
Unit Designation		Schedule #	
Specifications			
Performance			
Uniform Energy Factor	3.10		
Uniform First Hour Rating	69 Gallons		
Nom Heating Capacity (Btu/h)	15,400 Btu/h		
Nom Heating Capacity (kw)	4.5kw		
Heating COP @ 80/47/17°F	5.5 / 4.2 / 2.8		
Water Temperature Setting (°F)	45 or 149 DegF		
Refrigerant Type	R744 (CO ₂)		
Refrigerant Charge (Oz)	25.4oz (720g)		
Power Voltage	208/230v-1Ph-60Hz		
Breaker Size	15A		
MCA (Amps)	10.2A		
Compressor RLA/LRA (Amps)	7.5A / 9.8A		
Fan Motor RLA/Watts	0.4A / 70W		
Pump RLA/Watts	0.4A / 40W		
Noise Level (DbA)	37		
Weight (lbs)	108lbs		
Storage Tank			
SAN-43SSAQA			
Nominal Volume	43 Gallons		
Pressure Relief Valve (Psig & °F)	125 & 210°F		
Temperature Sensor	Thermistor		
Tank Weight (lbs)	88lbs		
Standby Loss in 67°F Ambient	91 Btu/h		
Tank Connection Sizes			
Cold Water Inlet	3/4" NPT		
Hot Water Outlet	3/4" NPT		
Cold Water to Heat Pump	3/4" NPT		
Hot Water Return from HP	3/4" NPT		
Pipe Size - Tank to Heat Pump			
Cold Water pipe - Tank to HP	1/2"		
Hot Water pipe - HP to Tank	1/2"		
Max Pipe Length inc	66ft		
Max Vertical Separation of	23ft		
Certifications			
Safety	ETL & ETLc Pending		
Performance	Energy Star Pending		
Warranty - System			
Heat Pump	3 Years Labor		
Tank	10 Years Parts		
	15Yrs Limited Lifetime		



3619 CAMPANIL DRIVE
APN 047-101-004

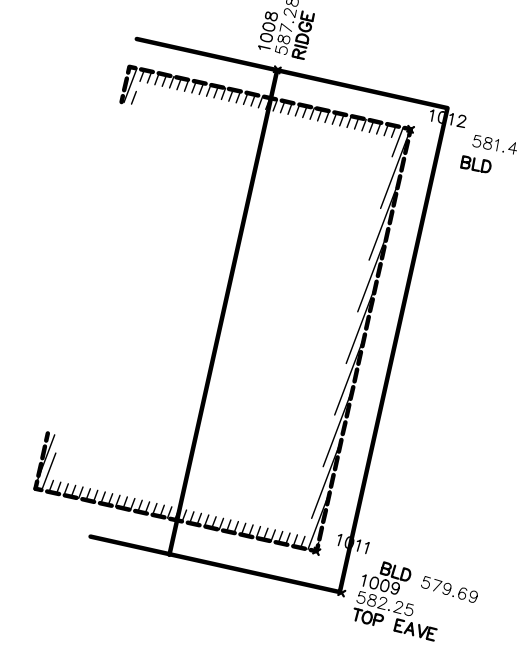


PHOTO LEGEND (See sheets A0.5 & A0.6)

- 1) STREET VIEW TO SOUTH
- 2) STREET VIEW TO NORTH
- 3) VIEW DOWN DRIVEWAY TO NORTH
- 4) STREET VIEW TO NORTH TO PROPERTY
- 5) SOUTH SIDE (E) RESIDENCE LOOKING EAST
- 6) NORTH SIDE OF (E) RESIDENCE LOOKING EAST
- 7) WEST SIDE OF (E) RESIDENCE LOOKING SOUTH
- 8) EAST SIDE OF (E) RESIDENCE LOOKING SOUTH
- 9) EAST SIDE OF (E) RESIDENCE LOOKING NORTH
- 10) WEST SIDE OF (E) PROPERTY LOOKING NORTH
- 11) NORTH SIDE OF (E) RESIDENCE LOOKING WEST
- 12) AERIAL OF PROPERTY AND VICINITY

3635 CAMPANIL DRIVE
APN 047-101-002

LEGEND

- AC ASPHALT CONCRETE
- AD AREA DRAIN
- BLD BUILDING
- CIT CITRUS TREE
- COL COLUMN
- CP CONTROL POINT (survey)
- DL DRIPLINE (oak tree)
- DO DRAIN OUTLET (pipe)
- DW DRIVEWAY
- E EASTERLY
- EM ELECTRIC METER
- EP EDGE OF PAVEMENT
- FD FOUND (survey measurement)
- FF FINISH FLOOR
- FL FLOWLINE
- FN FENCE
- FOB FACE OF BUILDING
- G GRADE
- GAR GARAGE
- GM GAS METER
- H HEDGE
- LP LAMP POLE
- N NORTHERLY
- PEP PEPPER TREE
- PL PARCEL LINE
- P.U.E. PUBLIC UTILITY EASEMENT
- S SOUTHERLY
- SM SMALL
- SMH SEWER MANHOLE
- T TOP of "
- TC TOP OF CURB
- TCN TOP OF CONCRETE
- TEL TELEPHONE EQUIP.
- TH THRESHOLD
- TOE TOE OF SLOPE
- TS TOP OF SLOPE or TOP OF STEPS
- TS T.V. TELEPHONE & ELECTRIC (underground utility lines)
- TW TOP OF WALL
- W WESTERLY
- WL WALL
- WM WATER METER
- WV WATER VALVE
- WWL WING WALL

PARTIAL SITE PLAN


 MISSION GROUP ARCHITECTS
 R. E. JOHNSON AIA
 1230 COAST VILLAGE CIRCLE STE H SANTA BARBARA, CALIFORNIA 93108 805-969-5910 © COPYRIGHT MISSION GROUP ARCHITECTS

PROPOSED ADU FOR:
KISLAK RESIDENCE
 3627 CAMPANIL DRIVE
 SANTA BARBARA, CALIFORNIA 93109

SHEET
A-1.1
 DATE
 07/14/2023

DOOR SCHEDULE											
MARK NO.	TYPE	SIZE			MATL	DETAILS			REMARKS		
		W	H	T		SHT. A-6.1 U.N.O.	H	J		S	
1		3'-6"	9'-0"	1 1/4"	SC WD.	X/AX	X/AX	X/AX	SOLID CORE WOOD PLANK FINISH, RADIUS ARCH TOP		
2		PR. 6'-0"	9'-0"		STL/GLS	5/A6.1	8/A6.1	11/A6.1	THREE LIGHT FRENCH DOOR w/TEMPERED GLASS		
3		3'-6"	9'-0"	1 1/4"	SC WD.				INTERIOR POCKET DOOR		
4		3'-0"	8'-0"	1 1/4"	STL/GLS	5/A6.1	8/A6.1	12/A6.1	3 LIGHT FRENCH DOOR w/TEMPERED GLASS		
5		PR. 5'-0"	8'-0"	1 1/4"	SC WD.	10/A6.1			INTERIOR DOOR		
6		3'-0"	8'-0"	1 1/4"	SC WD.	10/A6.1			INTERIOR DOOR		
7		2'-8"	8'-0"	1 1/4"	SC WD.	10/A6.1			INTERIOR DOOR		
8		2'-0"	6'-8"	1/2"	TEMP GL.				TEMPERED GLASS SHOWER DOOR		
9		2'-8"	8'-0"	1 1/4"	SC WD.	10/A6.1			50% OPEN LOUVER INT. DOOR, APPROX. 950 S.I. NFA		
10		3'-0"	8'-0"	1 1/4"	SC WD.	10/A6.1			INTERIOR DOOR		
11		PR. 2'-0"	8'-0"	1 1/4"	SC WD.	10/A6.1			INTERIOR DOOR		
12		2'-2"	7'-0"	1 1/4"	SC WD.				VERT. PLNK. DR. w/LVRED. 12"x12" opn'g tp. and btm.		
13		2'-4"	8'-0"	1 1/4"	SC WD.				INTERIOR DOOR		
14		PR. 1'-6"	5'-0"		WRT. IRON				WROUGHT IRON GATE, DESIGN TBD		
15		4'-0"	5'-8"	1 1/4"	SC WD.				SOLID CORE WOOD PLANK GATE, FLAT ARCH TOP		
16		2'-9"	6'-0"		WRT. IRON				WROUGHT IRON GATE, DESIGN TBD		
17		4'-0"	6'-0"		WRT. IRON				WROUGHT IRON GATE, DESIGN TBD		
18		4'-0"	6'-0"		WRT. IRON				WROUGHT IRON GATE, DESIGN TBD		
19											
20											

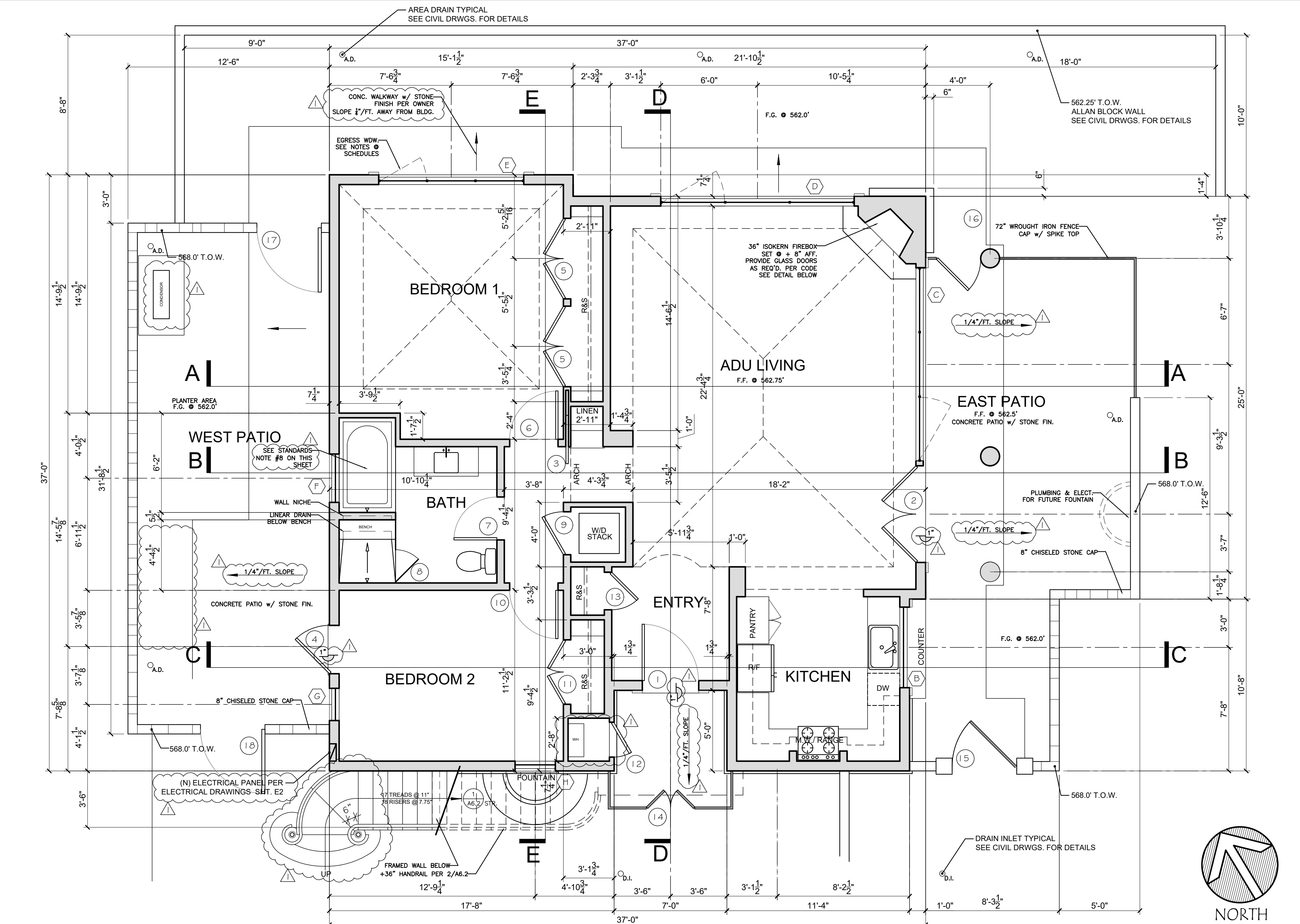
WINDOW SCHEDULE											
MARK NO.	TYPE	FRAME SIZE		MATL	DETAILS				MFG.	QTY.	REMARKS
		W	H		SHT. A-6.1 UNO	ROOM					
A											
B	CSMT.	5'-0"	4'-0"	STL/GL	5/A6.1	6/A6.1	2/A6.1	KITCHEN		1	
C	FIXED	12'-0"	8'-4"	STL/GL	5/A6.1	6/A6.1	7/A6.1	LIV. RM.		1	12 LIGHT STL. FRAME w/ TEMP. GLASS
D	FIXED	12'-0"	8'-4"	STL/GL	5/A6.1	6/A6.1	7/A6.1	LIV. RM.		1	12 LIGHT STL. FRAME w/ TEMP. GLASS
E	FIXED	9'-0"	7'-4"	STL/GL	5/A6.1	6/A6.1	7/A6.1	BDRM. 1		1	9 LIGHT STL. FRAME w/ TEMP. GLASS
F	CSMT.	3'-0"	4'-0"	STL/GL	1/A6.1	3/A6.1	1/A6.1	BATH 1		1	6 LIGHT STL. FRAME w/TEMP. GLASS
G	CSMT.	2'-0"	7'-4"	STL/GL	1/A6.1	3/A6.1	1/A6.1	BDRM. 2		1	10 LIGHT STL. FRAME w/ TEMP. GLASS
H	CSMT.	2'-6"	5'-0"	STL/GL	5/A6.1	6/A6.1	7/A6.1	BDRM. 2		1	EGRESS WINDOW-SEE NOTES
I											
J											
K											
L											
M											

DOOR NOTES:

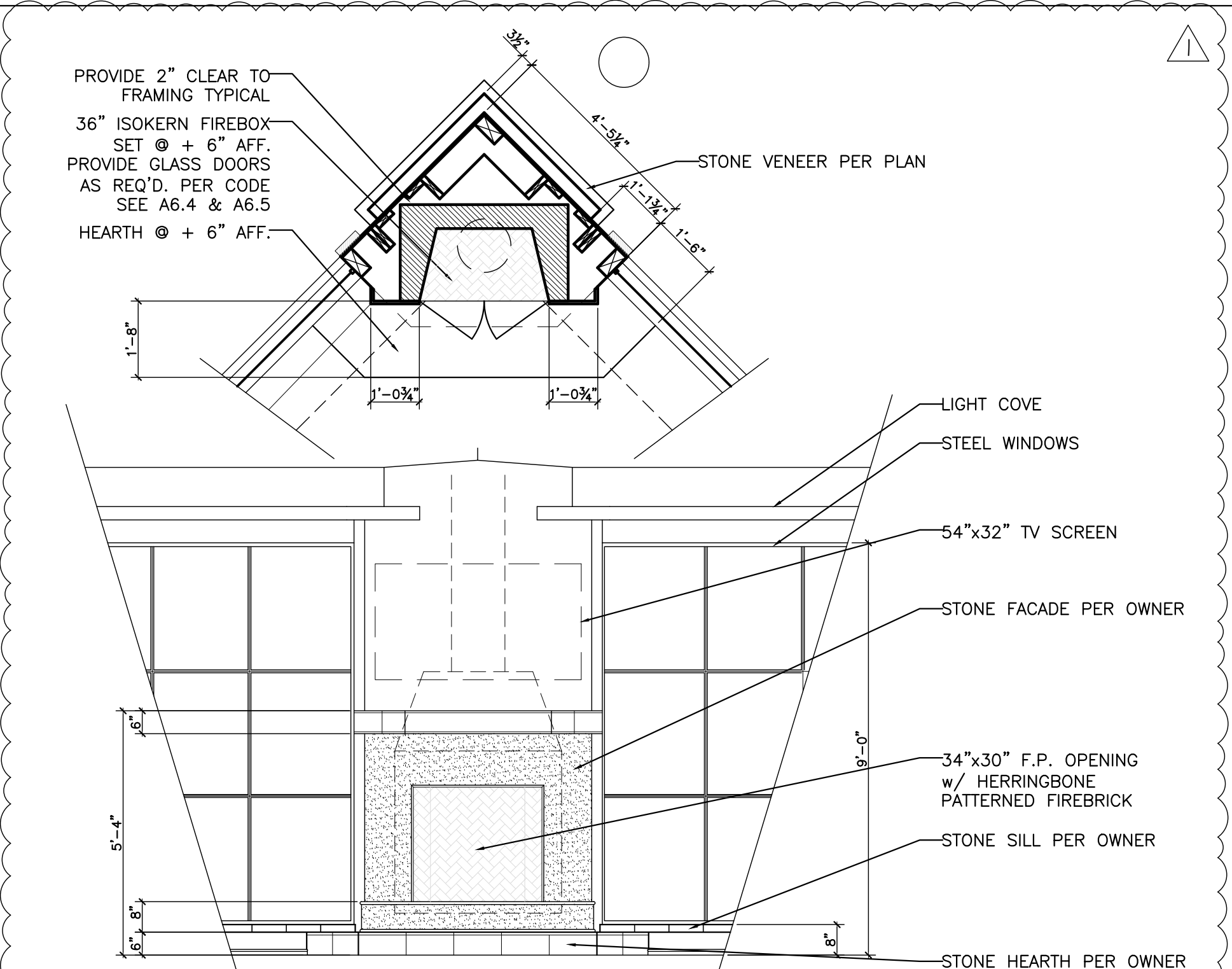
- THE FOLLOWING DOORS SHALL HAVE MATCHING COLOR ALUMINUM SCREENS: 2,4.
- ALL EXTERIOR DOORS TO BE DUAL GLAZED WITH A MINIMUM OF ONE TEMPERED PANE. MUNTINS (IF APPLICABLE) SHALL BE "AUTHENTIC DIVIDED LIGHT".
- PANEL DOORS TO BE (2) PANEL AS MANUFACTURED BY T.M. COBB. TYPICAL INTERIOR DOORS WITH V.G. DOUGLAS FIR RAILS. PANELS TO BE MULTIFIBER BOARD UNLESS OTHERWISE NOTED.
- ALL EXTERIOR DOORS SHALL BE WEATHERSTRIPPED. PEMKO 356DV ASTRAGAL ON INACTIVE LEAF OF EACH PAIR OF DOORS.
- ALL DOORS TO BE PROTECTED FROM MOISTURE AND PRIMED AND SEALED (TOP AND BOTTOM) IMMEDIATELY UPON ARRIVAL TO SITE. GENERAL CONTRACTOR TO ASSUME THE RESPONSIBILITY FOR CONDITION OF DOORS DURING CONSTRUCTION PERIOD. ALL PRIMING OF DOORS TO BE PER DOOR MANUFACTURER'S SPECIFICATIONS. IF DOOR TOPS AND BOTTOMS ARE RECUT AFTER PRIMING OR PAINTING, THEY ARE TO BE REPRIMED OR PAINTED IMMEDIATELY.
- HARDWARE NOT SPECIFIED IN NOTES OR ABOVE SHALL BE INCLUDED IN FINISH HARDWARE ALLOWANCE.
- DOORS WHICH HAVE SPECIAL DECORATIVE GLASS SPECIFIED ARE TO COME FROM MANUFACTURER WITHOUT GLAZING UNLESS OTHERWISE NOTED.
- ALL EXTERIOR DOORS SHALL BE HOPE DOOR/WINDOW SYSTEMS w/ DUAL 20% ≥ LEED-H ENERGY STAR GLAZING. MAX. U-FACTOR OF .35 & MAX. SHGC OF .35 PER ENERGY CALCS. ON SHEET A-X.O.

WINDOW NOTES:

- ALL OPERABLE CASEMENT OR PARTIAL CASEMENT WINDOWS SHALL HAVE REMOVABLE ALUMINUM SCREENS. COLOR: MATCH WINDOW COLOR.
- WINDOWS SHALL RECEIVE SHADING DEVICES PER ENERGY CALCS. (IF REQUIRED IN TITLE 24 NOTES.)
- ALL WINDOWS TO BE DUAL GLAZING WITH MINIMUM ONE PANE TEMPERED. MUNTINS SHALL BE "AUTHENTIC DIVIDED LIGHT".
- ALL WINDOWS TO PROTECTED FROM MOISTURE AND PRIMED AND SEALED (TOP AND BOTTOM) IMMEDIATELY UPON ARRIVAL TO SITE. GENERAL CONTRACTOR TO ASSUME THE RESPONSIBILITY FOR CONDITION OF WINDOWS DURING CONSTRUCTION PERIOD. ALL PRIMING OF WINDOWS TO BE PER MANUFACTURER'S SPECIFICATIONS.
- SCHEDULED "CUSTOM" WINDOWS SHALL CONFORM TO HEAD, JAMB, AND SILL DETAILS. VERIFY DETAILS OF EACH SPECIFIC WINDOW CONFIGURATION AND CONSTRUCTION WITH ARCHITECT PRIOR TO FABRICATION.
- ALL DIMENSIONS SHOWN ON WINDOW SCHEDULE ARE FINISHED OPENINGS. VERIFY R.O.
- HARDWARE NOT SPECIFIED IN NOTES OR ABOVE SHALL BE INCLUDED IN FINISH HARDWARE ALLOWANCE.
- ALL FACTORY WINDOWS TO BE FACTORY PRIMED UNLESS OTHERWISE NOTED.
- WINDOWS WHICH HAVE SPECIAL DECORATIVE GLASS SPECIFIED ARE TO COME FROM MANUFACTURER WITHOUT GLAZING UNLESS OTHERWISE NOTED.
- ALL EGRESS WINDOWS SHALL HAVE AT MINIMUM: 20" WIDTH CLEARANCE, 24" HEIGHT CLEARANCE, 5.7 SF OPENING & SHALL HAVE A MAX. SILL HEIGHT OF NO MORE THAN 44" ABOVE FINISH FLOOR.
- ALL WINDOWS SHALL BE MILGARD ALUMINUM SERIES w/ DUAL 20% ≥ LEED-H ENERGY STAR GLAZING. MAX. U-FACTOR OF .35 AND MAX. SHGC OF .35. SEE ENERGY CALCS. ON SHEET A-X.O.



MAIN LEVEL FLOOR PLAN



FIREPLACE DETAIL

STANDARD GENERAL NOTES:

- PROVIDE MIXING VALVES AT SHOWERS PER CPC.
- EACH SHOWERHEAD SHALL NOT EXCEED A WATER FLOW RATE OF 1.8 GPM.
- KITCHEN FAUCET SHALL NOT EXCEED A WATER FLOW RATE OF 1.8 GPM.
- RESIDENTIAL LAVATORY FAUCET SHALL NOT EXCEED A FLOW RATE OF 0.5 GPM.
- TOILETS SHALL BE 1.28 GPF MAX. NOTE: ALL PLUMBING FIXTURES ARE TO MEET THE STANDARDS REFERENCED IN THE TABLE 4.303.3 OF CGSBC 4.303.1 SEE SHEET A0.2.
- REFERENCE MANDATORY MEASURE REQUIREMENTS FOR CALIFORNIA ENERGY CODE AND CGSBC ON SHEETS A0.2 & A0.3.
- EXHAUST FANS @ WATER CLOSET COMPARTMENTS SHALL PROVIDE MIN. OF 5 AIR CHANGES PER HOUR AND BE CONTROLLED BY A HUMIDISTAT SENSOR.
- SHOWERS AND WALLS ABOVE BATHTUBS WITH SHOWERHEADS SHALL BE FINISHED WITH A NONABSORBENT SURFACE TO A HEIGHT OF NOT LESS THAN 6'-0" ABOVE THE FLOOR. (CRC R307.2)

LEGEND:

- NEW WALL
- EXISTING WALL TO REMAIN
- VERIFY ALL TO DIMENSIONS REPORT DISCREPANCIES TO ARCHITECT
- EXISTING WALL TO BE REMOVED
- HIDDEN WALL BELOW
- TEMPERED GLASS AS REQUIRED BY CODE
- INTR. ELEVATION REFERENCE SEE SHEET INDICATED
- DOOR REFERENCE SEE SHEET A-X
- WINDOW REFERENCE SEE SHEET A-X
- FINISH NOTES
- 2x4 STUD WALL @ 16" O.C.
- 2x8 STUD WALL @ 16" O.C.
- 2x10 STUD WALL @ 16" O.C.
- 2x12 STUD WALL @ 16" O.C.

PROPOSED ADU FOR:
KISLAK RESIDENCE
 5627 CAMPANIL DRIVE
 SANTA BARBARA, CALIFORNIA 93109

DATE: 7/8/2024
 SHEET: A-2

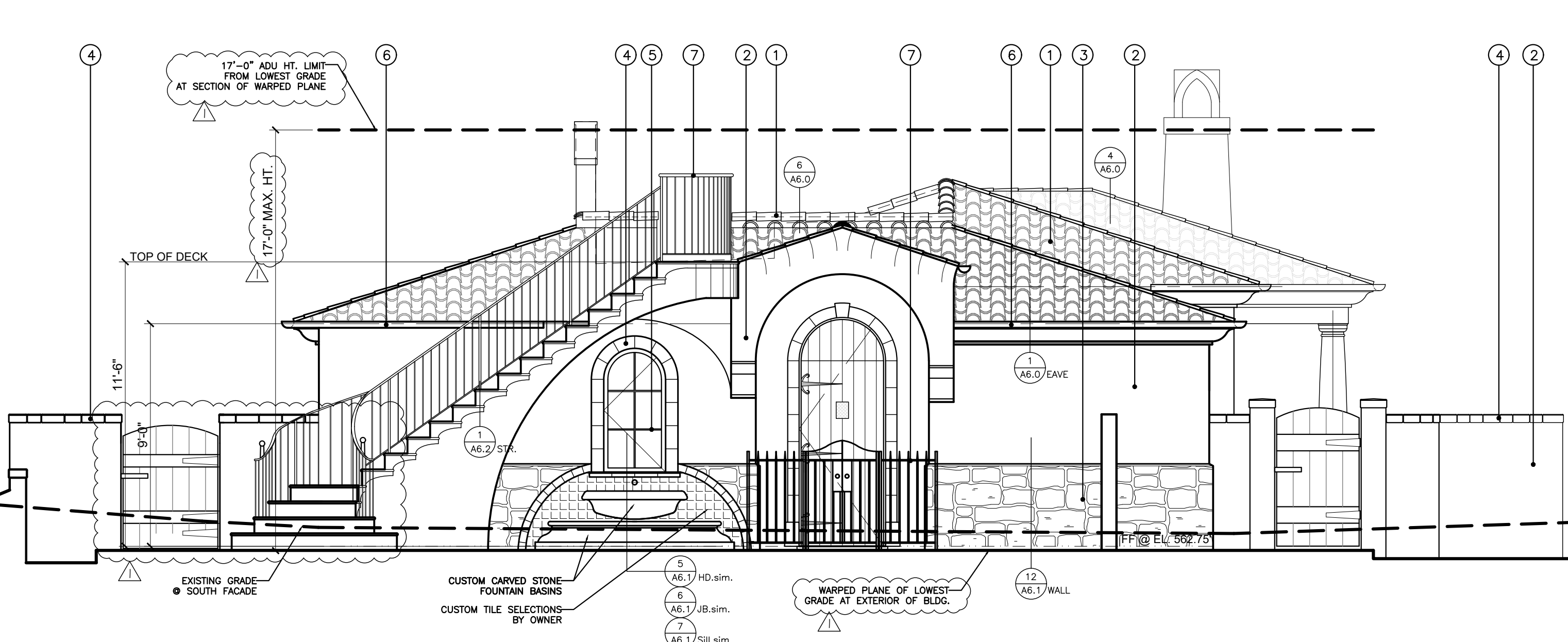
REVISIONS: 7/8/2024

MISSION GROUP ARCHITECTS
 R. E. JOHNSON AIA

1230 COAST VILLAGE CIRCLE STE H SANTA BARBARA CALIFORNIA 93108 805-969-5910

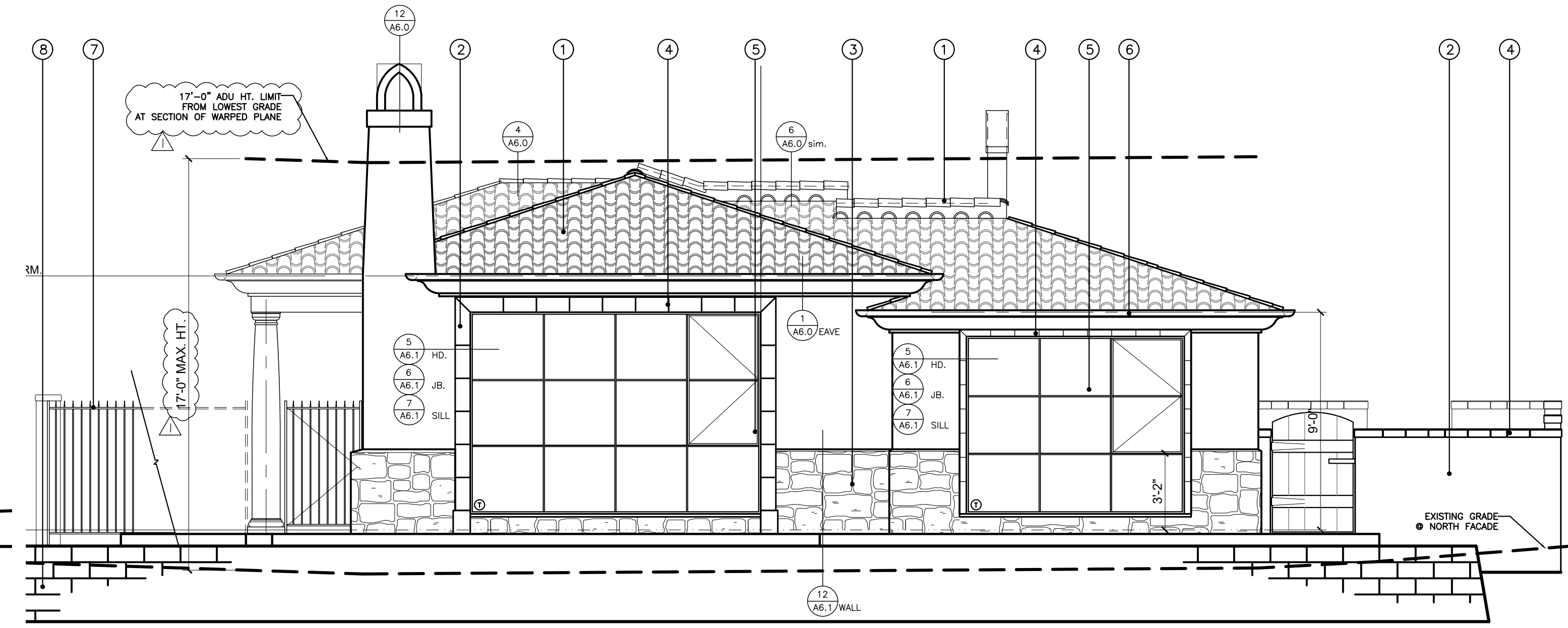
REGISTERED ARCHITECT
 STATE OF CALIFORNIA
 LICENSE NO. 425

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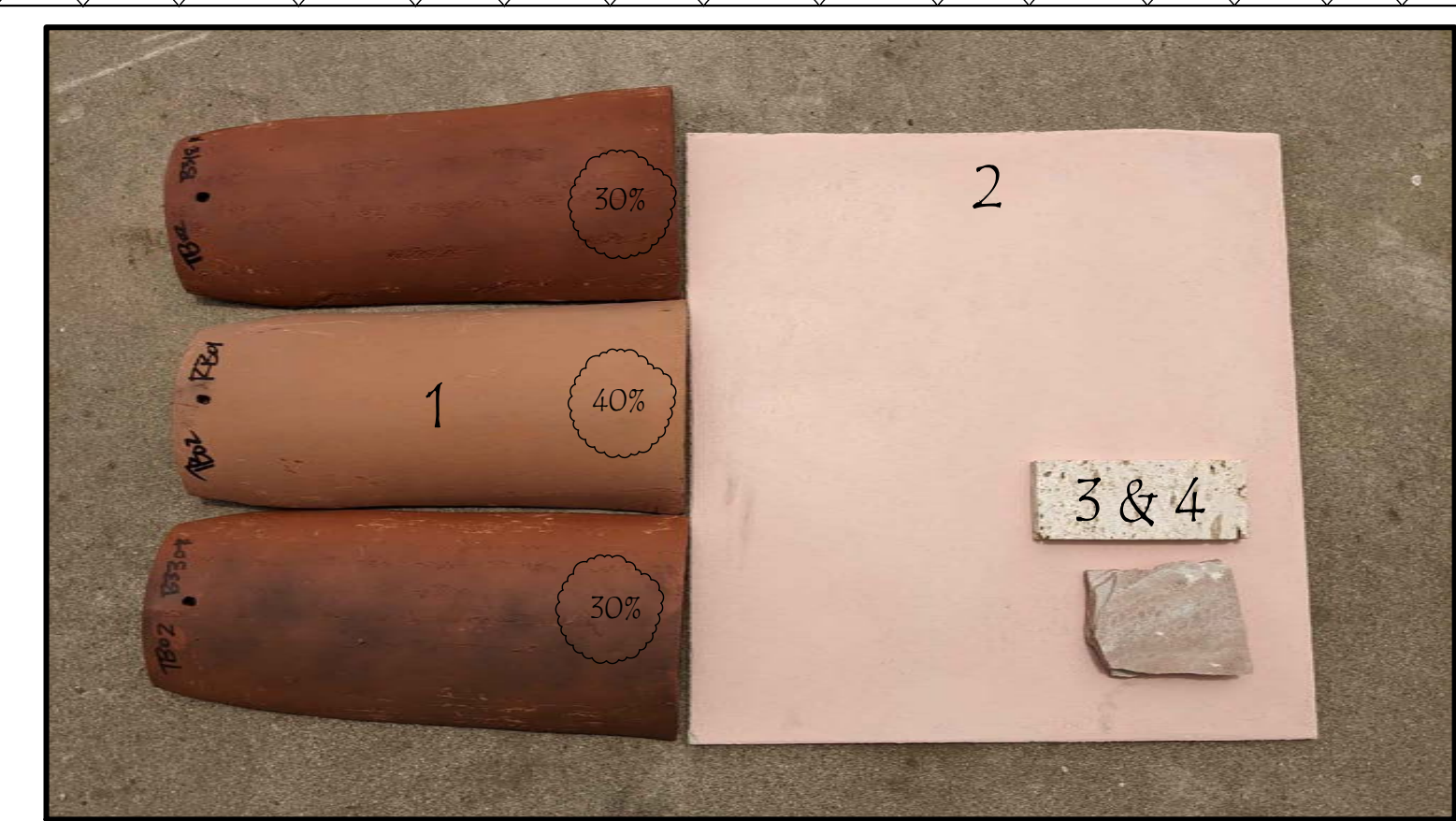
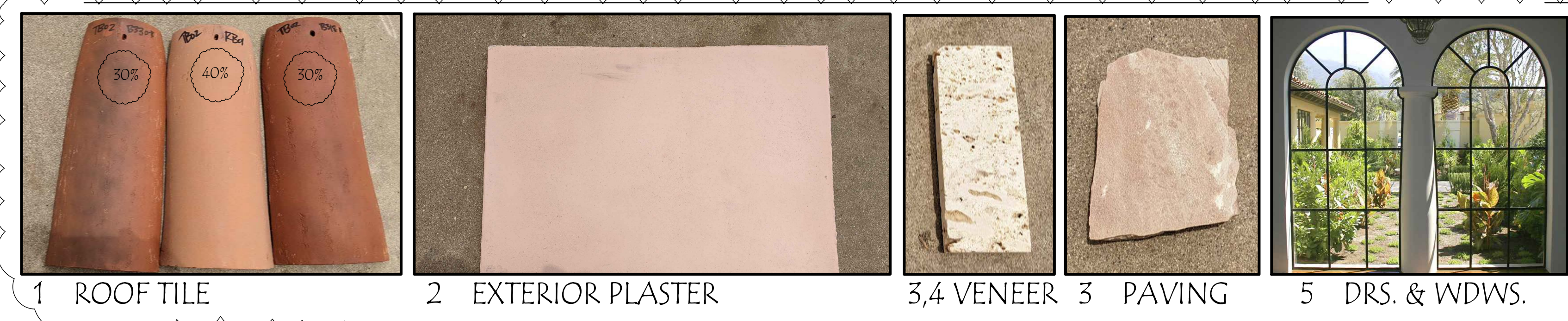
SOUTH ELEVATION

1/4" = 1'-0"



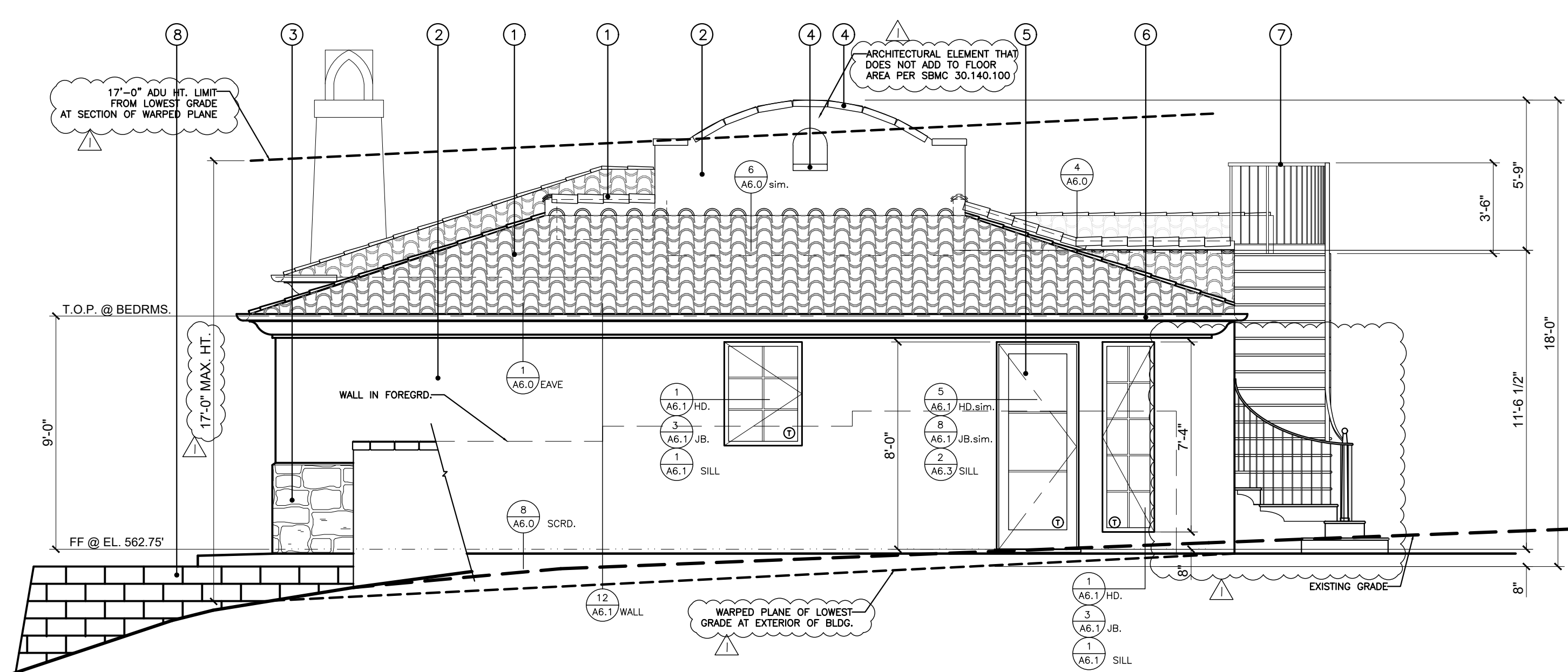
NORTH ELEVATION

1/4" = 1'-0"



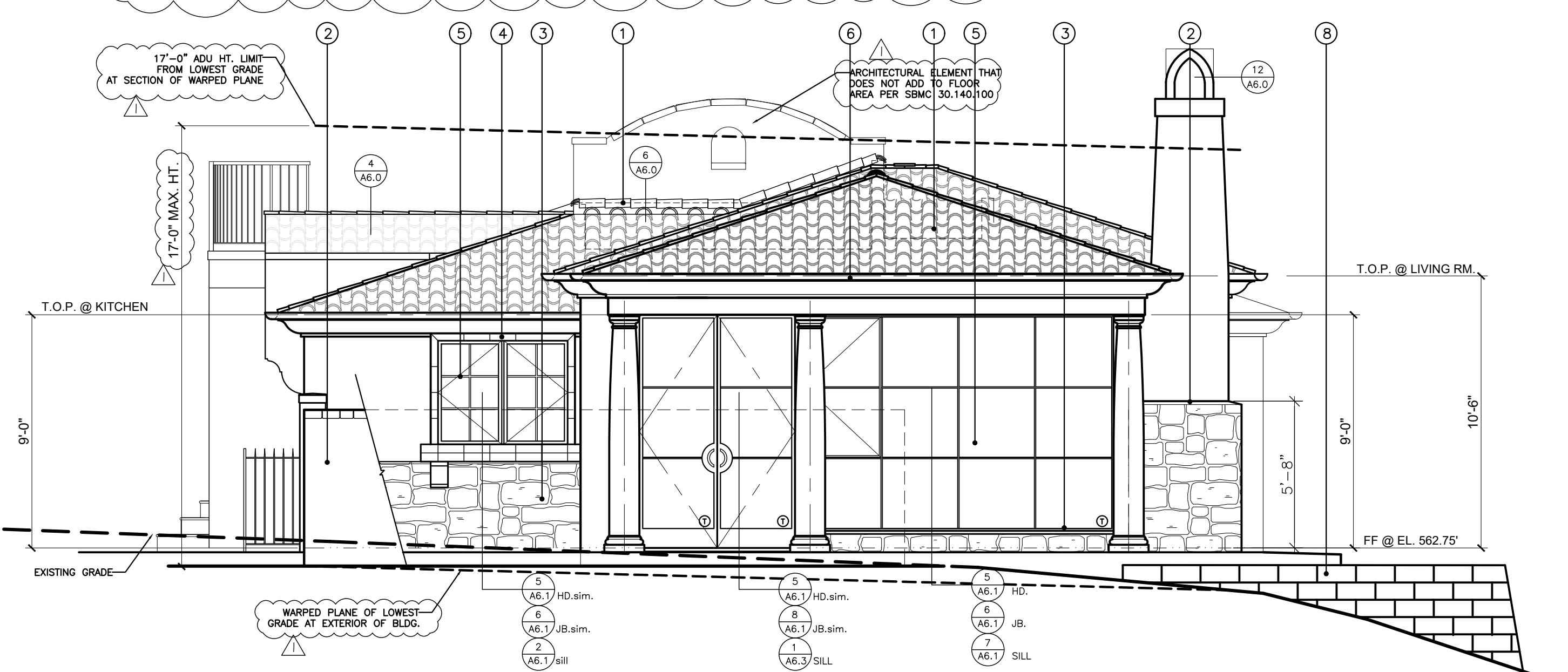
OVERALL COLOR PALETTE

- MATERIALS LEGEND:**
NOTE: VERIFY ALL COLORS AND MATERIALS WITH OWNER PRIOR TO ORDERING
- ① ROOFING: TWO PIECE MISSION TILE BY REDLANDS CLAY TILE
LUL CLASS "A" FIRE RATING
ICC: ESR-4395
COLOR - 3 TILE CUSTOM BLEND
40% LIGHT TILE, 30% & 30% DARK TILES
 - ② EXTERIOR PLASTER: 7/8" SMOOTH TROWEL EXTERIOR PLASTER
OVER MTL. LATH. & TYVEK HOMEWRAP TYPICAL
COLOR: CUSTOM COLOR AS SHOWN (SOFT PINK)
 - ③ STONE VENEER: BEIGE TRAVERTINE VENEER OVER SCRATCH COAT/GAL. LATH
OVER TYVEK WATERPROOF BARRIER OVER PLYWD.
FLOORING TO BE CHEROKEE SANDSTONE
 - ④ CARVED STONE TRIM: BEIGE TRAVERTINE VENEER (SEE ABOVE)
 - ⑤ DOORS & WDWS.: STEEL WINDOWS & DOORS BY MIRAMAS MFG. INC.
ALL GLASS TO MEET CRITERIA SET FORTH IN ENERGY CALC.
& MAX. SHGC OF .35
COLOR: BLACK
 - ⑥ GUTTERS & FLASHING: 1/2" GA. MIN. COPPER GUTTER & FLASHING
NEW GUTTERS TO HAVE DEERS GUARD
 - ⑦ CUSTOM WROUGHT IRON: DESIGN TO BE DETERMINED
DESIGN SHALL COMPLY WITH [CRC SECTION R312]
SEE DETAIL 2/A6.2 FOR GENERAL NOTES
 - ⑧ STACKED ALLANBLOCK: AB EUROPA COLLECTION



WEST ELEVATION

1/4" = 1'-0"



EAST ELEVATION

1/4" = 1'-0"

MISSION GROUP ARCHITECTS
 R. E. JOHNSON
 AIA

LICENSED ARCHITECT
 STATE OF CALIFORNIA
 NO. 425

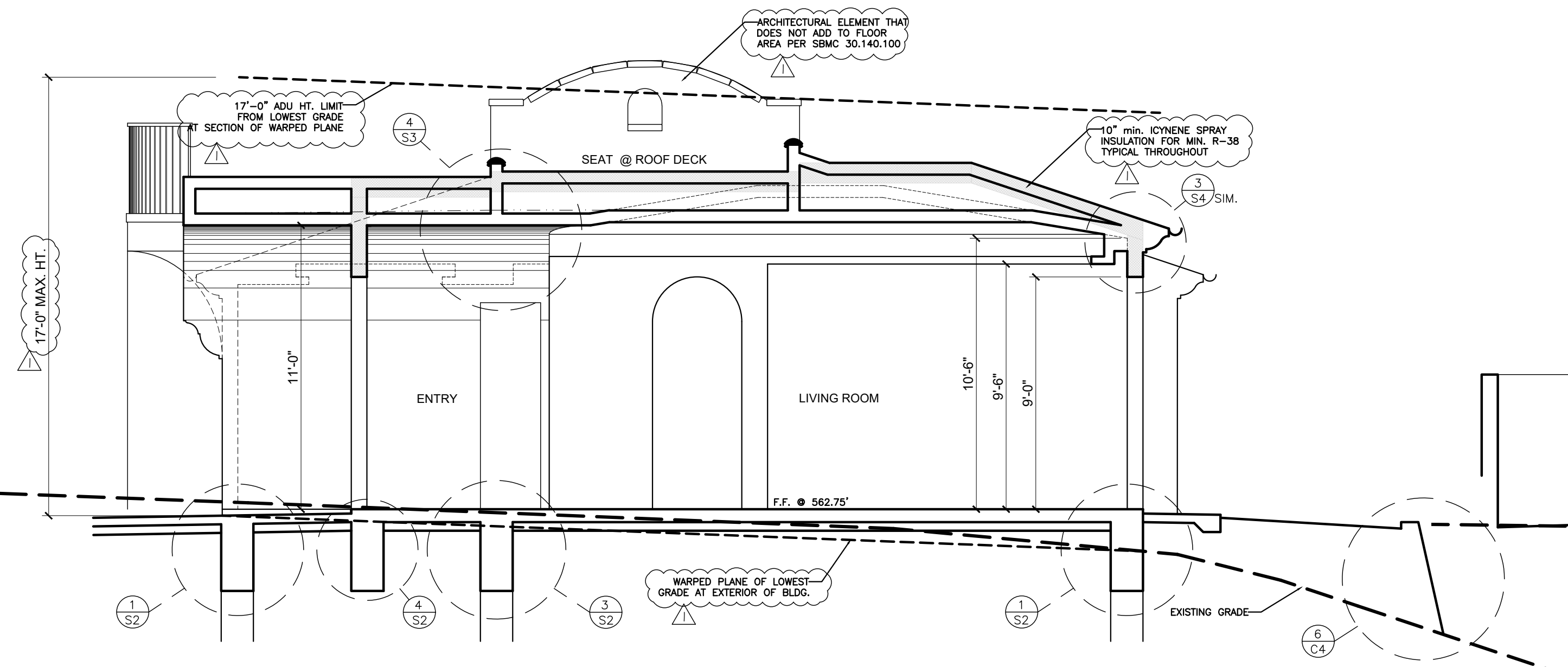
REVISIONS
 12/16/2024
 EXT. COLORS

PROPOSED ADU FOR:
KISLAK RESIDENCE
 5627 CAMPANIL DRIVE
 SANTA BARBARA, CALIFORNIA 93109

DATE
 9/14/2024

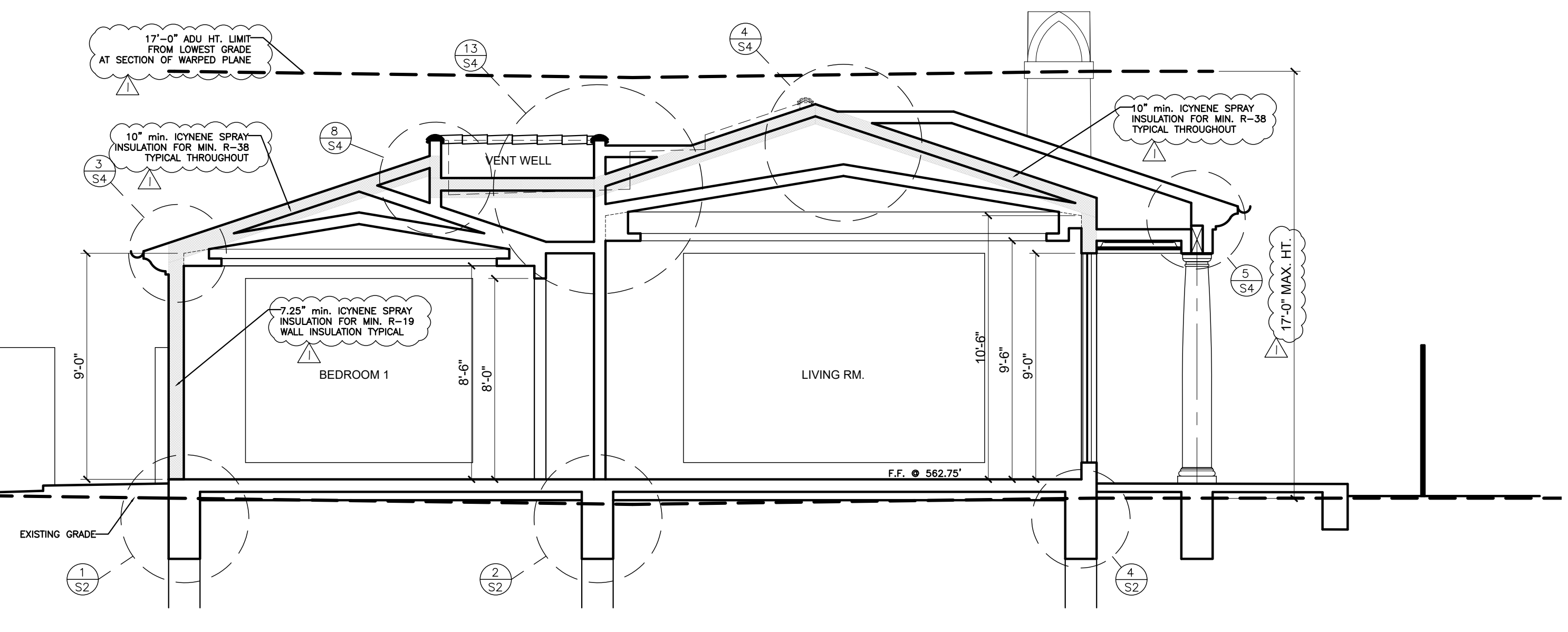
SHEET
A-4

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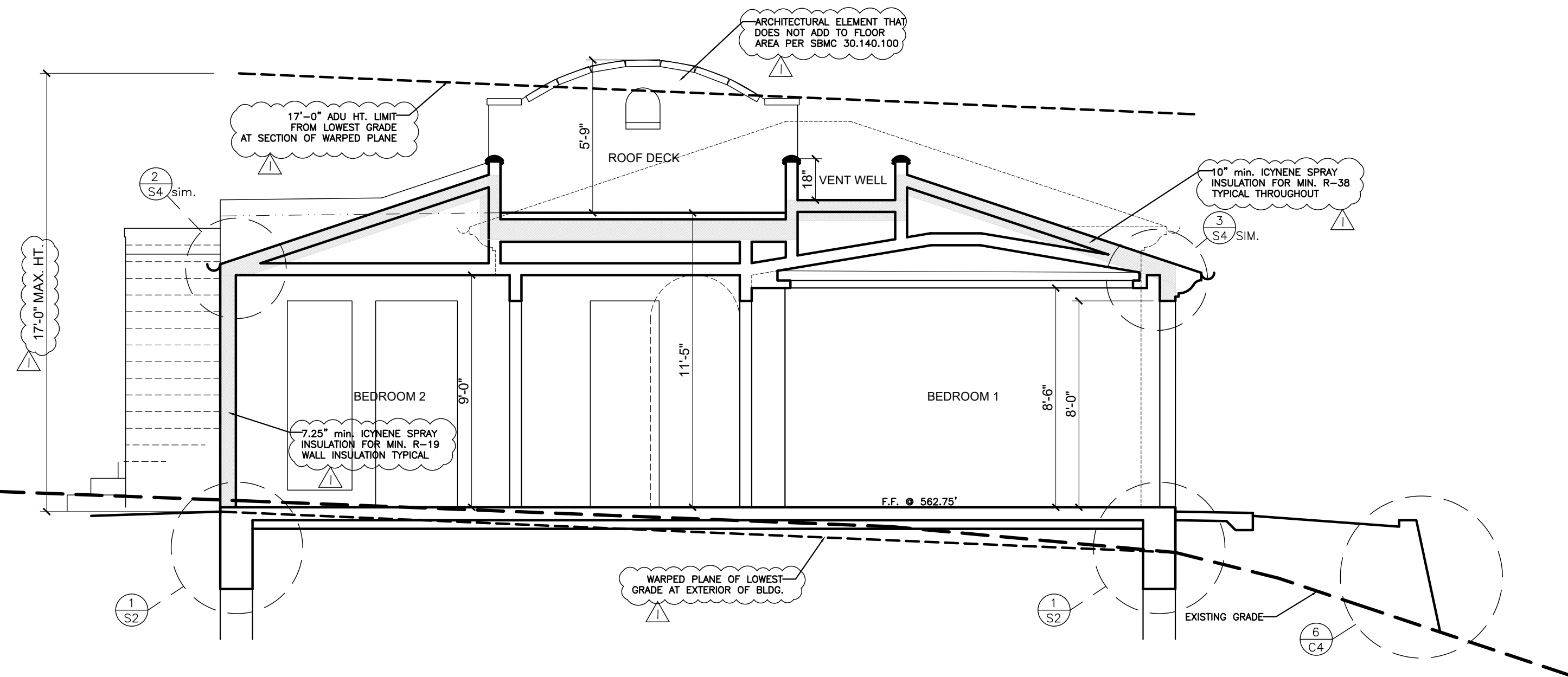
SECTION D

1/4" = 1'-0"



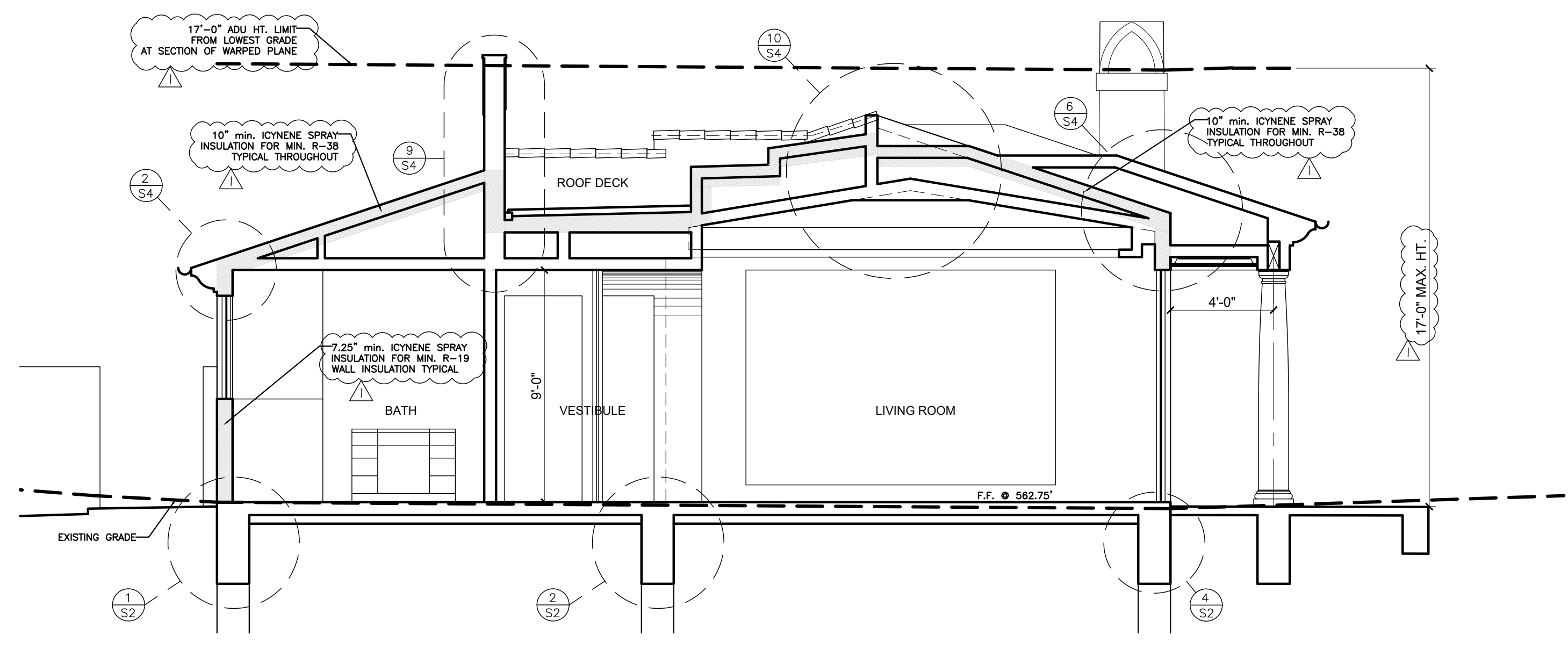
SECTION A

1/4" = 1'-0"



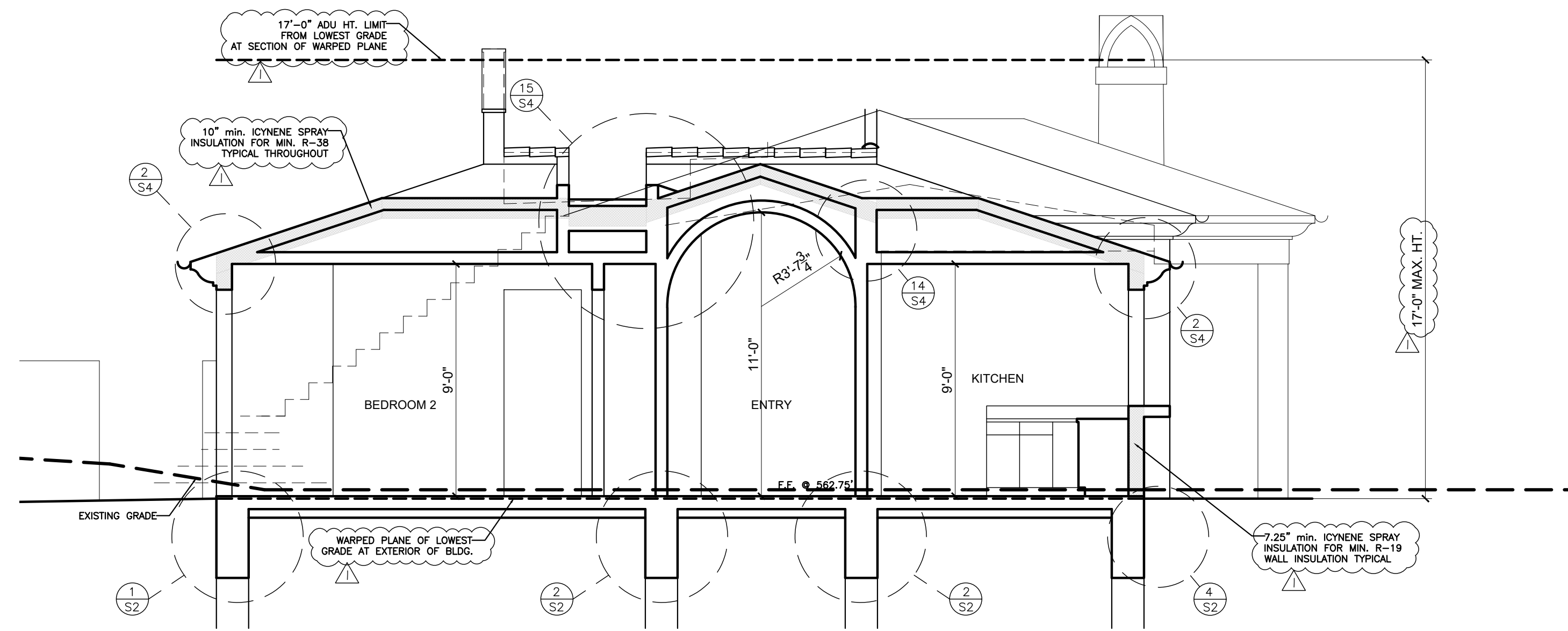
SECTION E

1/4" = 1'-0"



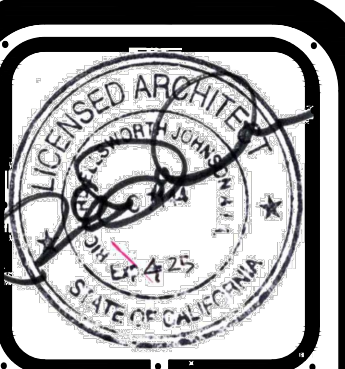
SECTION B

1/4" = 1'-0"



SECTION C

1/4" = 1'-0"



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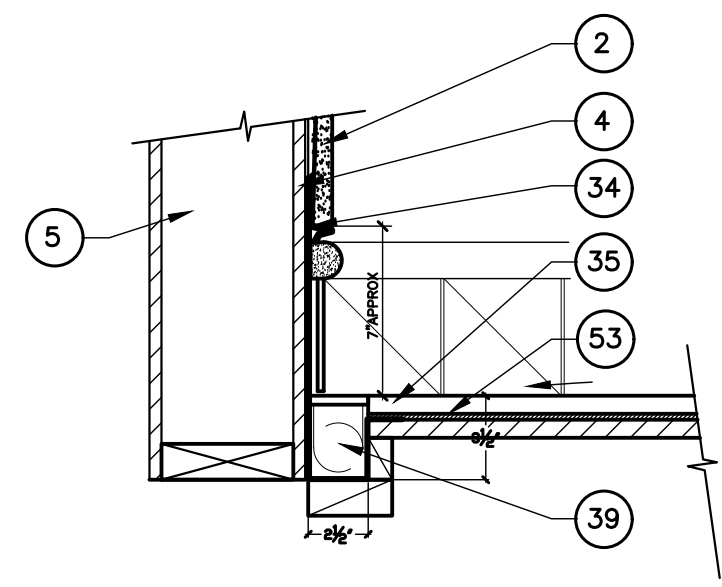
SECTIONS

PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

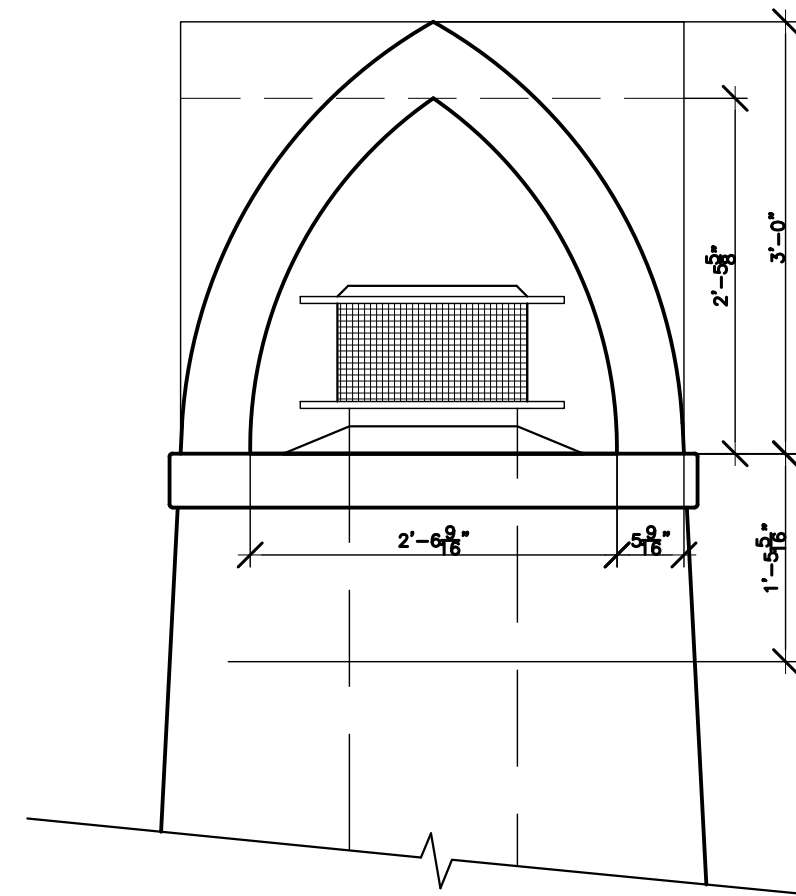
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SHEET
A-5

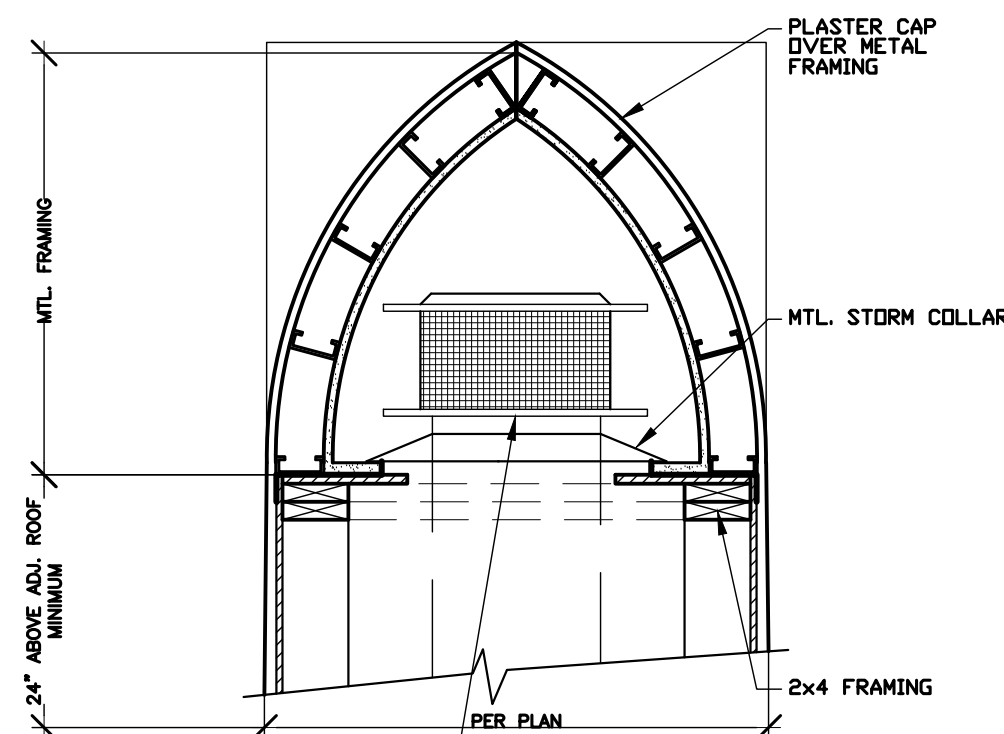
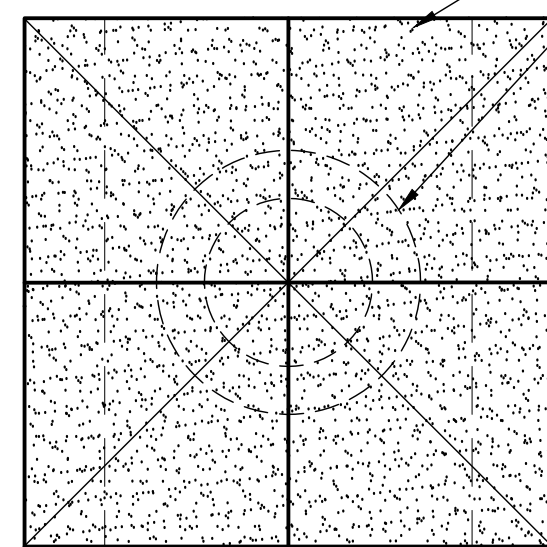
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9 SLOTTED DRAIN @ ROOF DECK
SCALE: 1/2" = 1'-0"



PLASTER CAP
4 WAY GOTHIC ARCH
TERMINATION CAP
BEYOND SHOWN DASHED

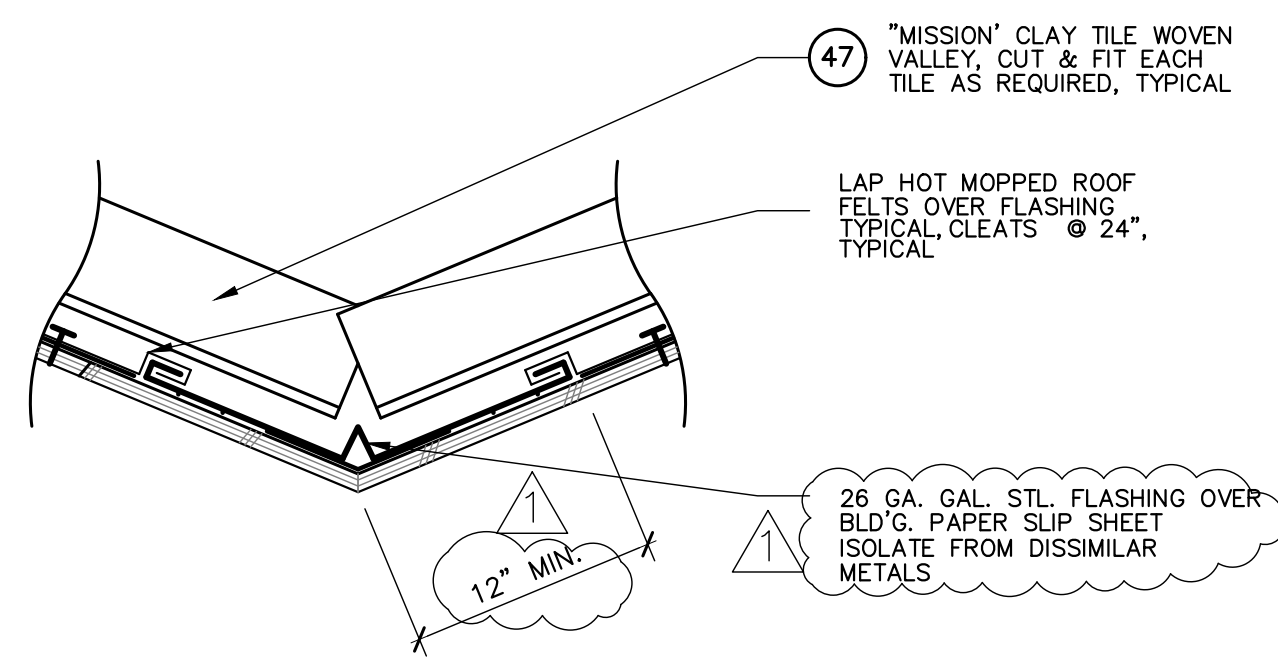


REMOVABLE TERMINATION
CAP SPARK ARRESTOR TO
BE DURAVENT UL103
LISTED MTL. FLUE
CLASS A PIPE
BY DURATECH

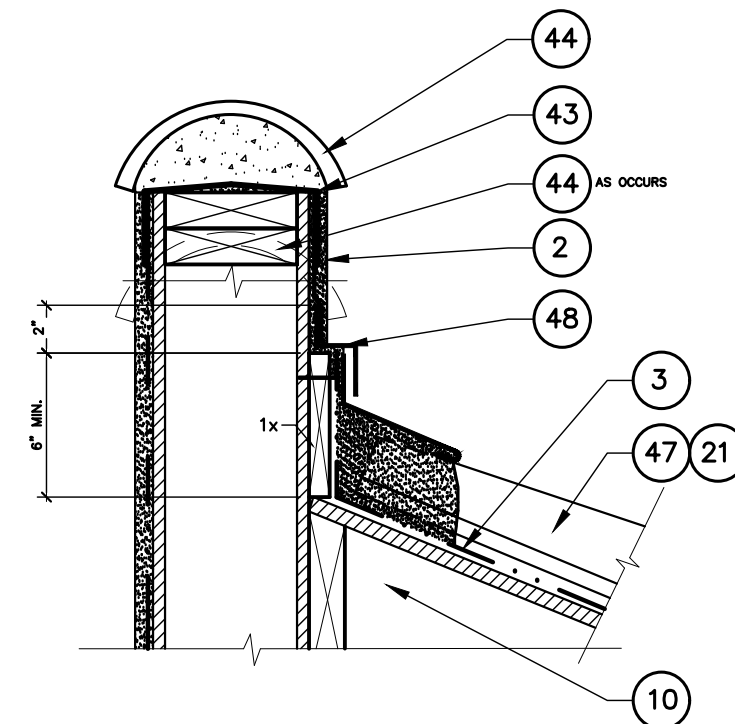
NOTE:
THE SPARK ARRESTOR SHALL BE ACCESSIBLE
FOR CLEANING AND THE SCREEN OR CHIMNEY
CAP SHALL BE REMOVABLE TO ALLOW FOR
CLEANING OF CHIMNEY FLUE. (CRC R1003.9.2.4)

NOTE:
FACTORY BUILT CHIMNEYS SHALL BE LISTED
AND LABELED AND SHALL BE INSTALLED AND
TERMINATED IN ACCORDANCE WITH THE
MANUFACTURER'S INSTRUCTIONS. (CRC R1005.1)

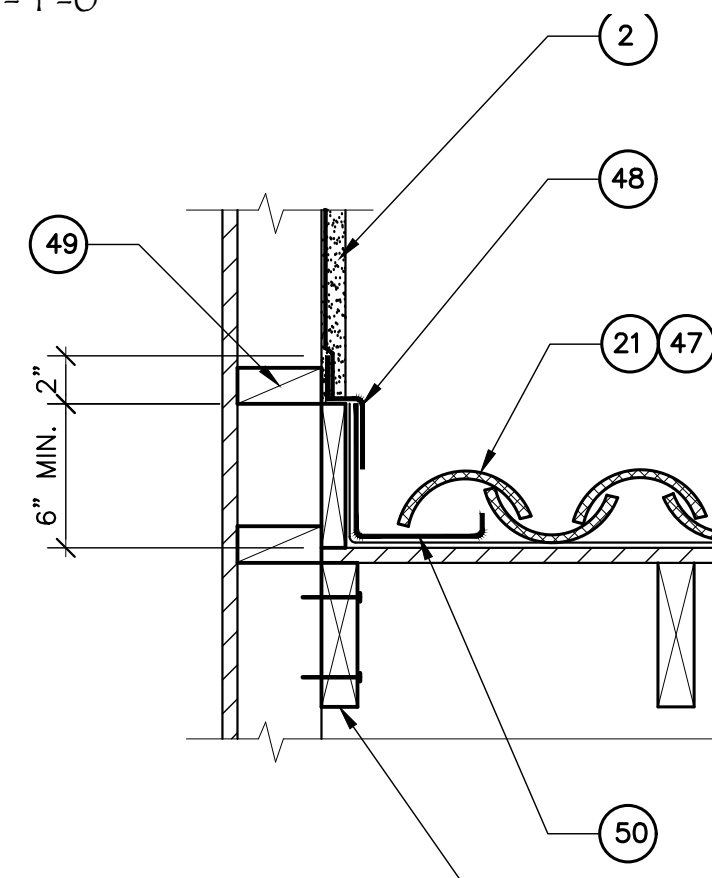
12 CHIMNEY CAP
SCALE: 3/4" = 1'-0"



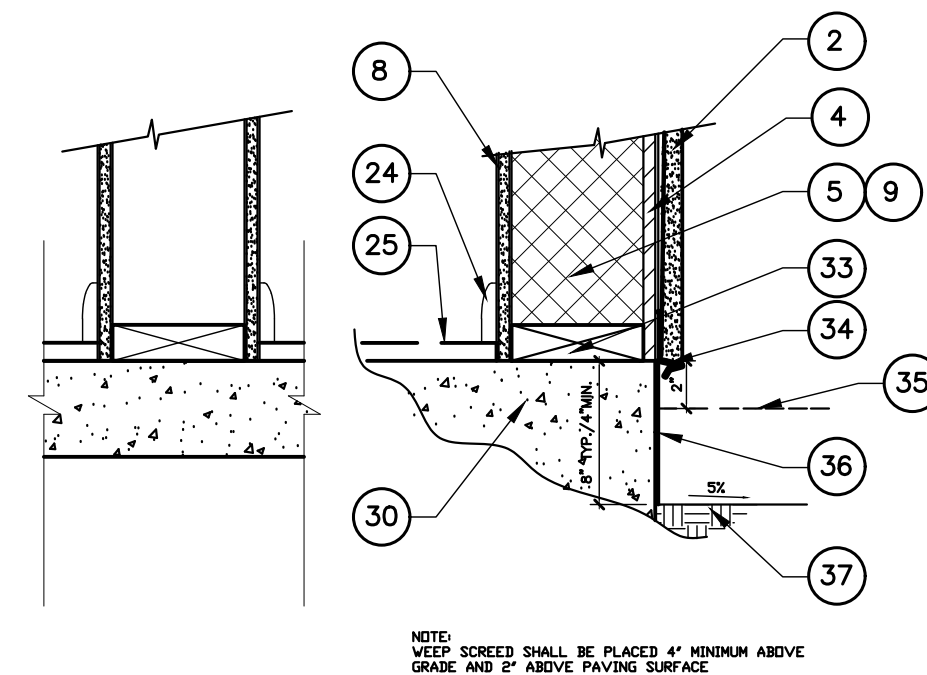
5 TYPICAL VALLEY
SCALE: 3" = 1'-0"



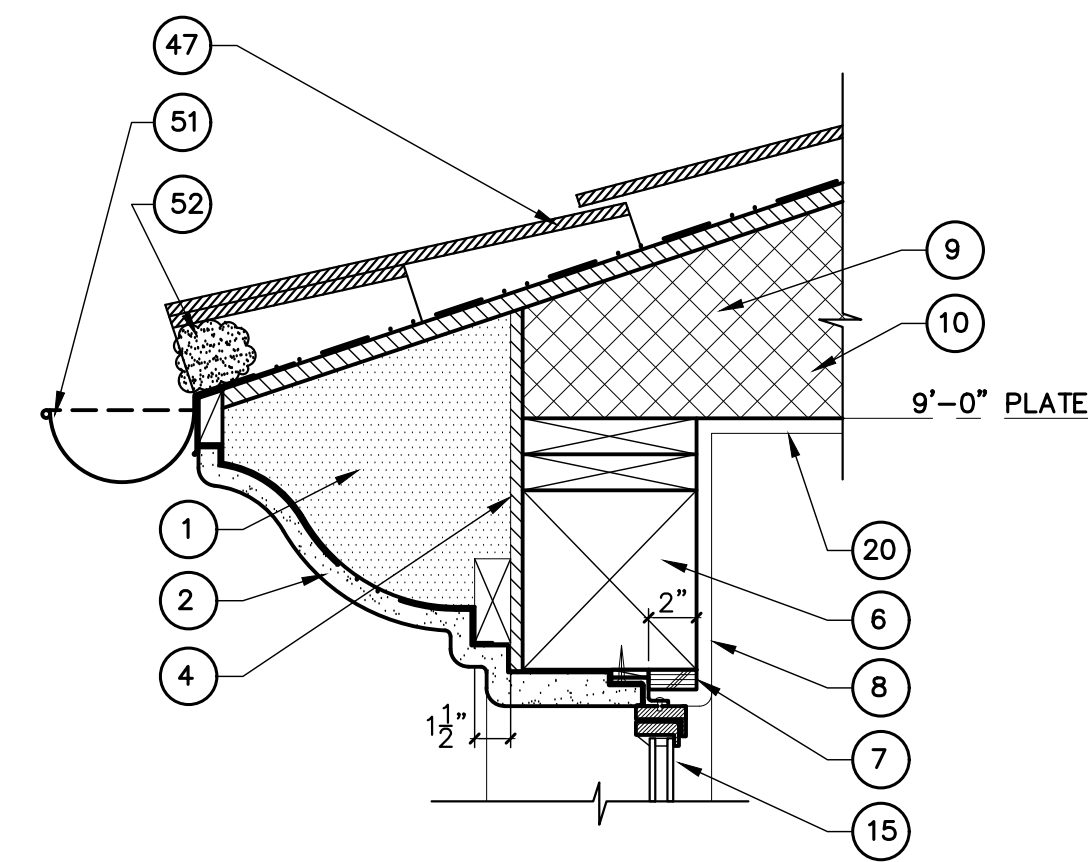
6 TYPICAL RAKE
SCALE: 1/2" = 1'-0"



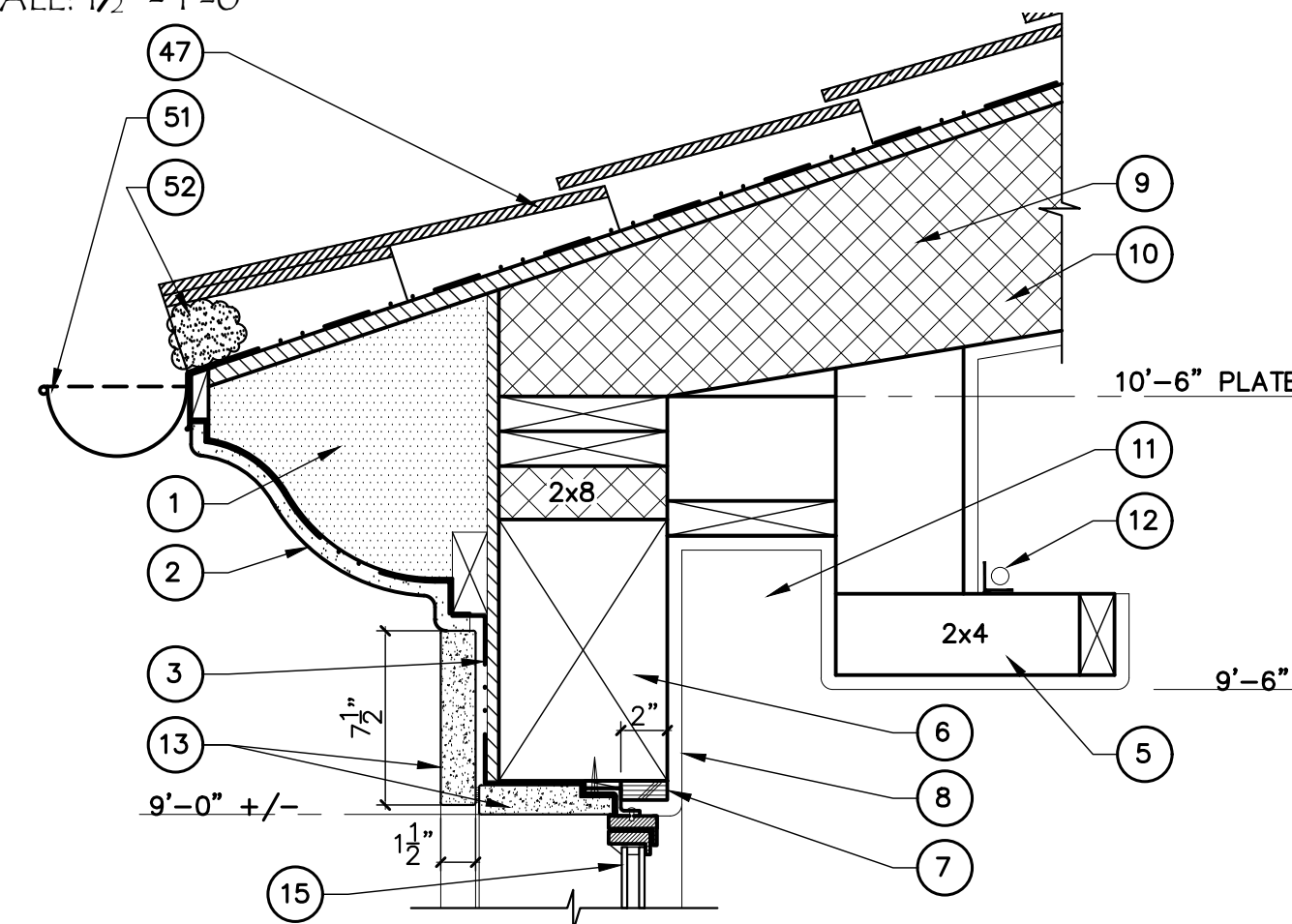
7 TYPICAL RAKE
SCALE: 3" = 1'-0"



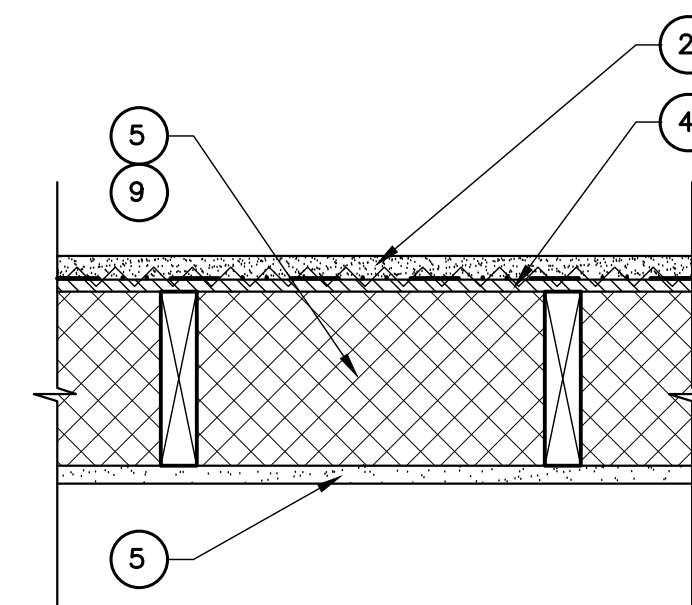
8 TYPICAL BASE SCREED
SCALE: 3" = 1'-0"



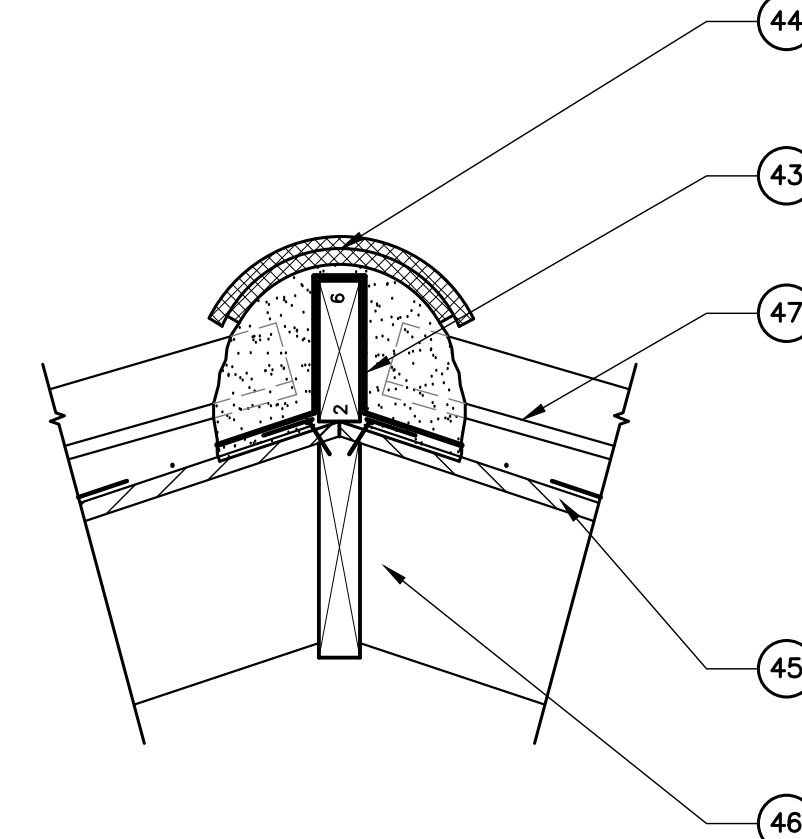
1 TYPICAL EAVE
SCALE: 1/2" = 1'-0"



2 TYPICAL EAVE
SCALE: 1/2" = 1'-0"



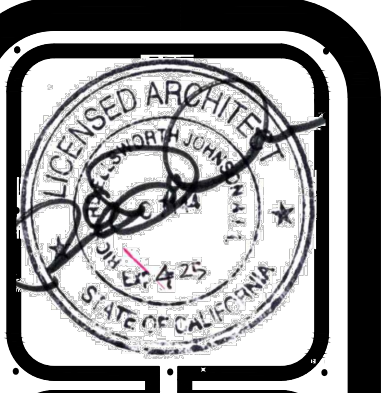
3 TYPICAL WALL ASSEMBLY
SCALE: 1/2" = 1'-0"



4 TYPICAL HIP/RIDGE
SCALE: 3" = 1'-0"

KEY NOTES:

- 1 STYROFOAM CORBEL PER ARCHITECT/OWNER
- 2 7" EXTERIOR MASONRY PLASTER OVER WATERPROOF MEMBRANE PER SPECS. OR MIN. 2 LAYERS GRADE "D" OR BETTER
- 3 WATERPROOF MEMBRANE PER SPECS
- 4 SHEAR PANEL PER STRUCTURAL
- 5 2x FRAMING PER PLAN
- 6 HEADER PER STRUCTURAL
- 7 3/4" PLYWOOD SHIM
- 8 5/8" DRYWALL W/ PLASTER SKIM COAT
- 9 R-19 WALL INSULATION PER ENERGY CALCS. R-30 ICYNENE ATTIC INSULATION PER ENERGY CALCS.
- 10 TRUSS TO BE DETERMINED
- 11 6"x6" DRAPE POCKET
- 12 LIGHT COVE PER LIGHTING SCHEDULE
- 13 ELDERADO STONE SURROUND (SEE MATERIALS LEGEND FOR DETAILS)
- 14 ELDERADO STONE VENEER (SEE MATERIALS LEGEND FOR DETAILS)
- 15 METAL WINDOW PER PLAN
- 16 DOOR PER PLAN
- 17 SILICONE SEALANT w/BACKING ROD
- 18 SHEET METAL PAN @ SILL LAP OVER W.P. MEMBRANE AS OCCURS
- 19 SLOPE @ SILL ONLY
- 20 CEILING AS OCCURS
- 21 TILE PER OWNER OVER MORTAR BASE OVER WATERPROOF MEMBRANE.
- 22 8"x8"x1/2" METAL "L" BRACKET
- 23 FINISH COUNTER TOP AS OCCURS
- 24 MDF BASE BOARD
- 25 FINISH FLOOR PER PLAN
- 26 2x NAILER
- 27 CARVED SLOPED STONE SILL OVER MORTAR
- 28 STONE BASE
- 29 EXTERIOR STONE FLOOR
- 30 FOUNDATION PER STRUCTURAL
- 31 EXPANSION JOINT MATERIAL
- 32 PLYWOOD GUSSET
- 33 PTDF PLATE PER PLAN
- 34 PROVIDE PLASTER SCREED NEAR TOP OF FOUNDATION
- 35 PAVED SURFACE AS OCCURS (2% MIN. SLOPE)
- 36 PLASTER FINISH COAT ON ALL EXPOSED FOUNDATION WALLS
- 37 FINISH GRADE. SLOPE 5% MINIMUM FROM BLD'G FOR 10'-0" MIN.
- 38 MILLED 2x K.D.V.G.D.F. JAMB
- 39 ZURN Z880 SLOT DRAIN w/ REMOVABLE DECORATIVE GRATE OR EQ. INSTALLED OVER MTL. FLASHING
- 40 3x6 HSS VERTICAL COLUMN PER STRUCT.
- 41 STEEL DOOR FRAME PER PLAN
- 42 SLOPED STONE SILL OVER MTL. PAN
- 43 90# CAP SHEET @ RIDGE
- 44 CAP TILE SET w/MORTAR OVER WP
- 45 PLYWD SH'T'G. PER STRUCTURAL
- 46 RAFTERS/TRUSS PER STRUCTURAL
- 47 2 PIECE MISSION TILE PER PLAN OVER WATERPROOF MEMBRANE PER SPEC.
- 48 "Z" BAR G.I. FLASH & COUNTERFLASH'G. LAP G.I. FLASHING w/ BUILDING PAPER
- 49 2x SOLID BLOCKING
- 50 G.I. CHANNEL FLASHING
- 51 COPPER GUTTER w/ LEAF GUARD
- 52 DBL. STARTER TILE w/ BIRD BLOCK MORTAR
- 53 ELASTA-TUFF 5000/6000 PED WATERPROOFING SYSTEM BY TUFFLEX AT EXT. STAIRS AND ROOFDECK



REVISIONS

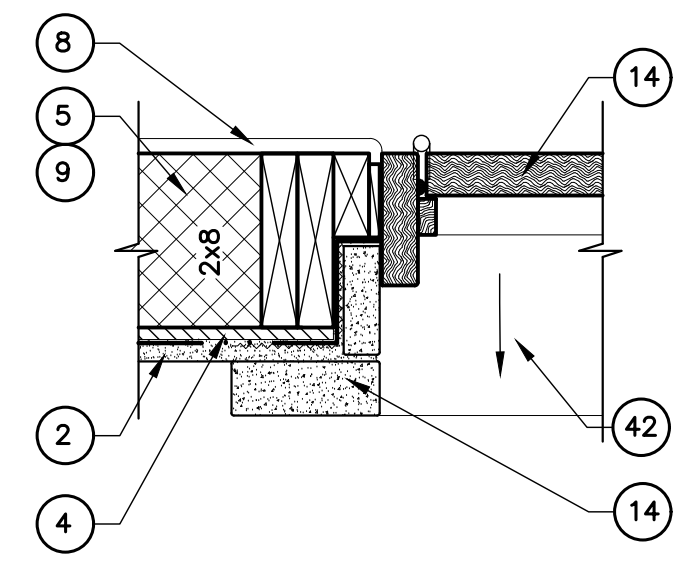
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R. E. JOHNSON
AIA

ARCHITECTURAL
DETAILS

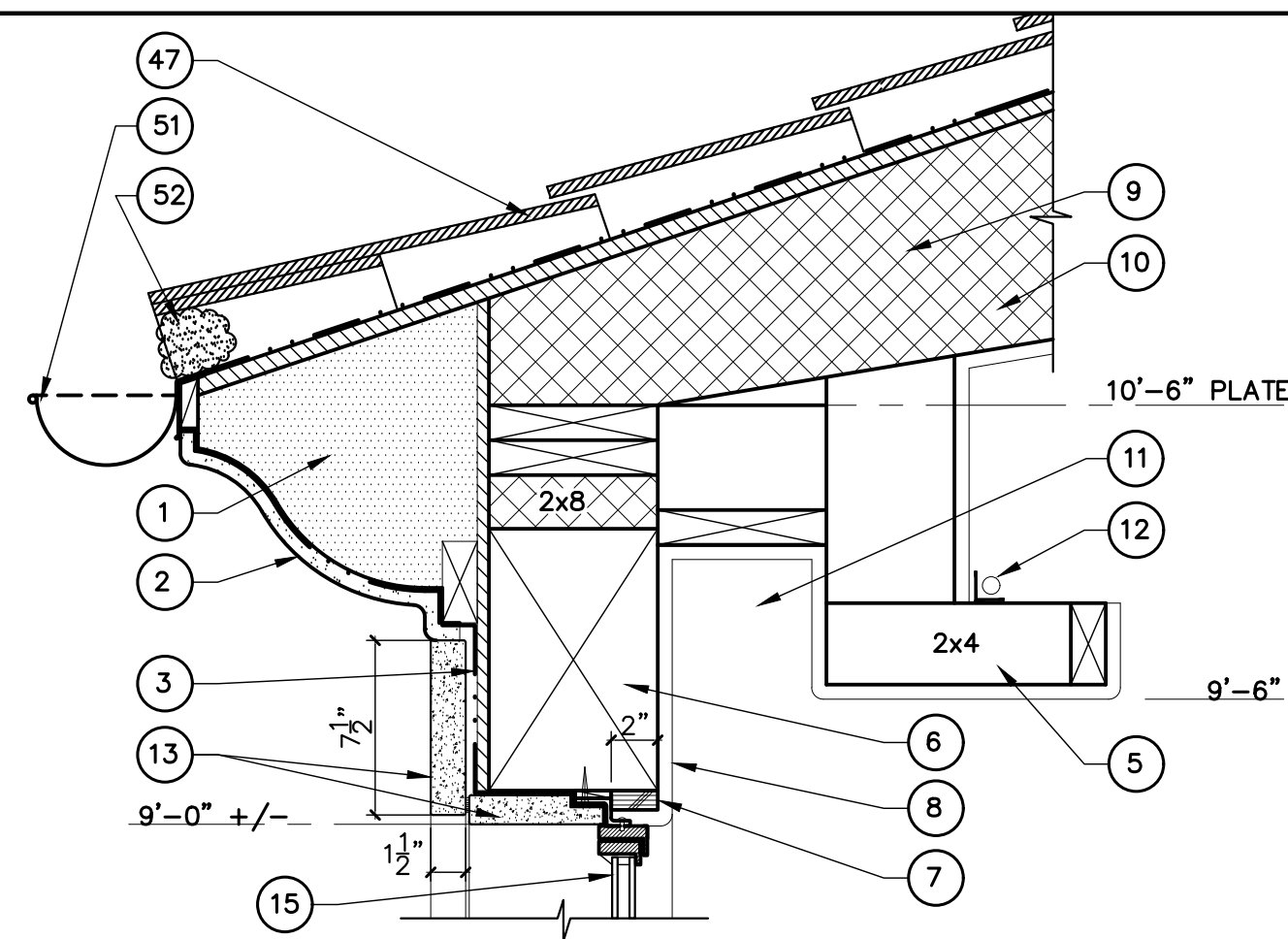
PROPOSED ADV FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

DATE
06/30/2025

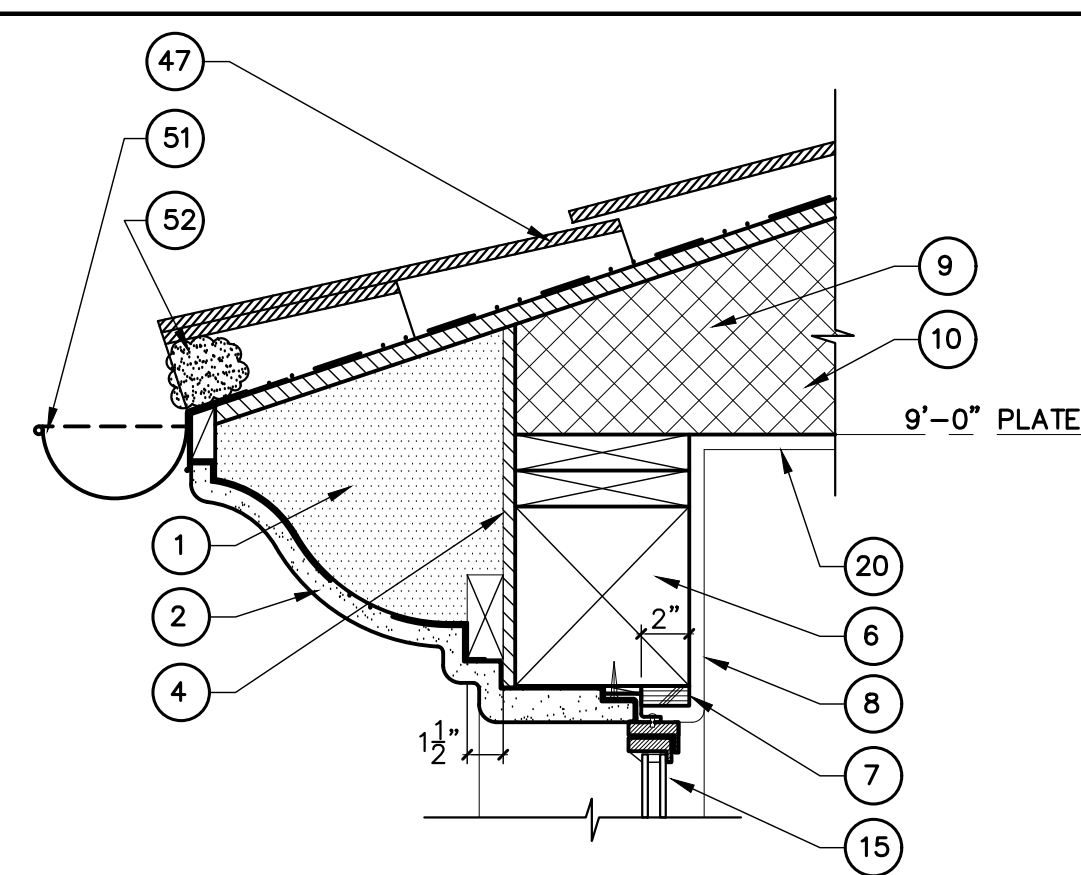
SHEET
A6.0



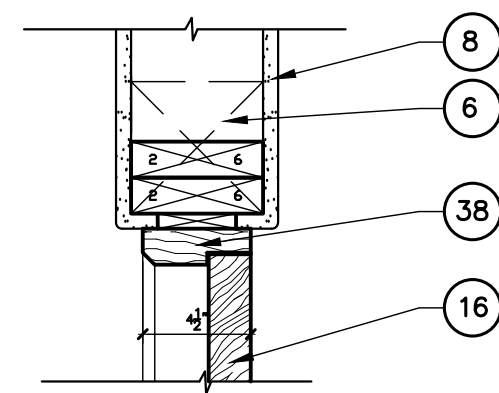
9 ENTRY DOOR JAMB
SCALE: 1/2" = 1'-0"



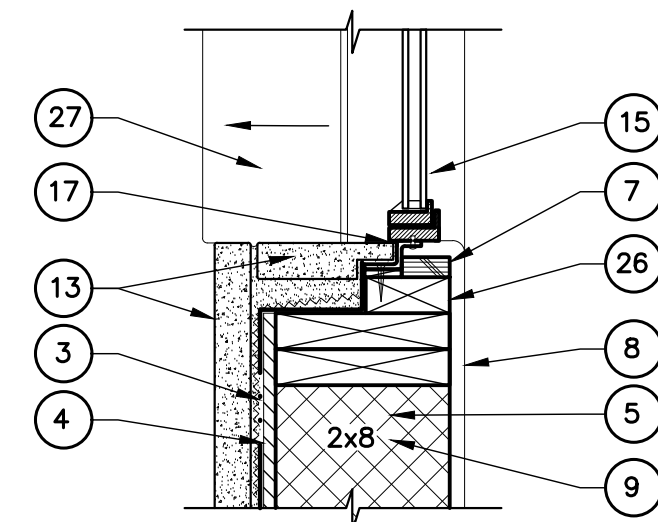
5 WDW. HEAD w/ STONE
SCALE: 1/2" = 1'-0"



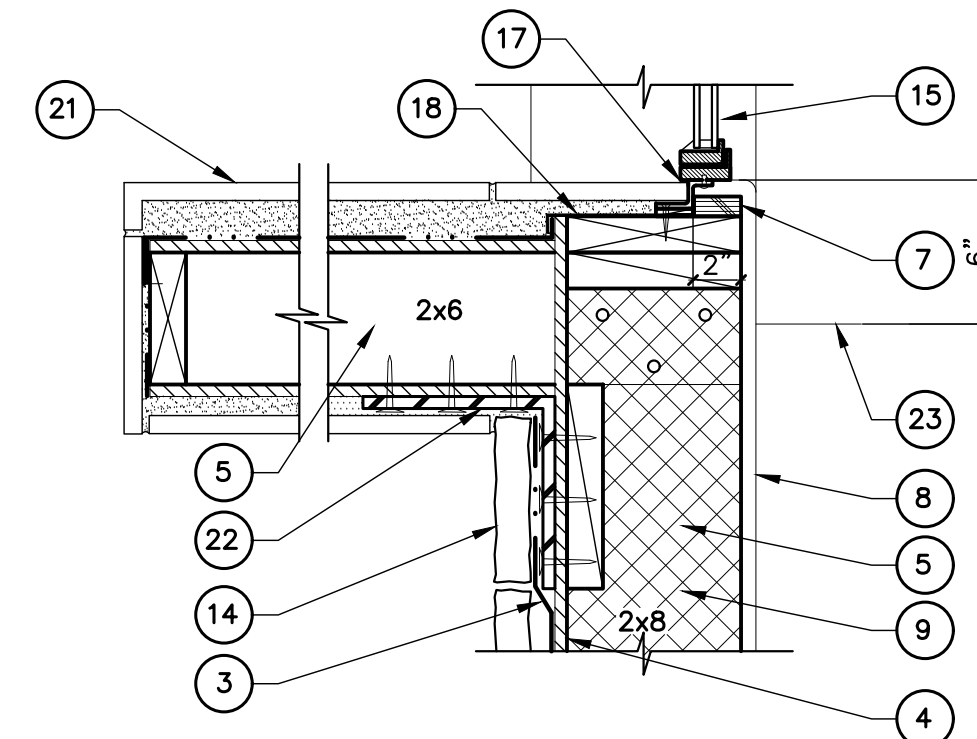
1 TYPICAL WDW. HEAD/SILL
SCALE: 1/2" = 1'-0"



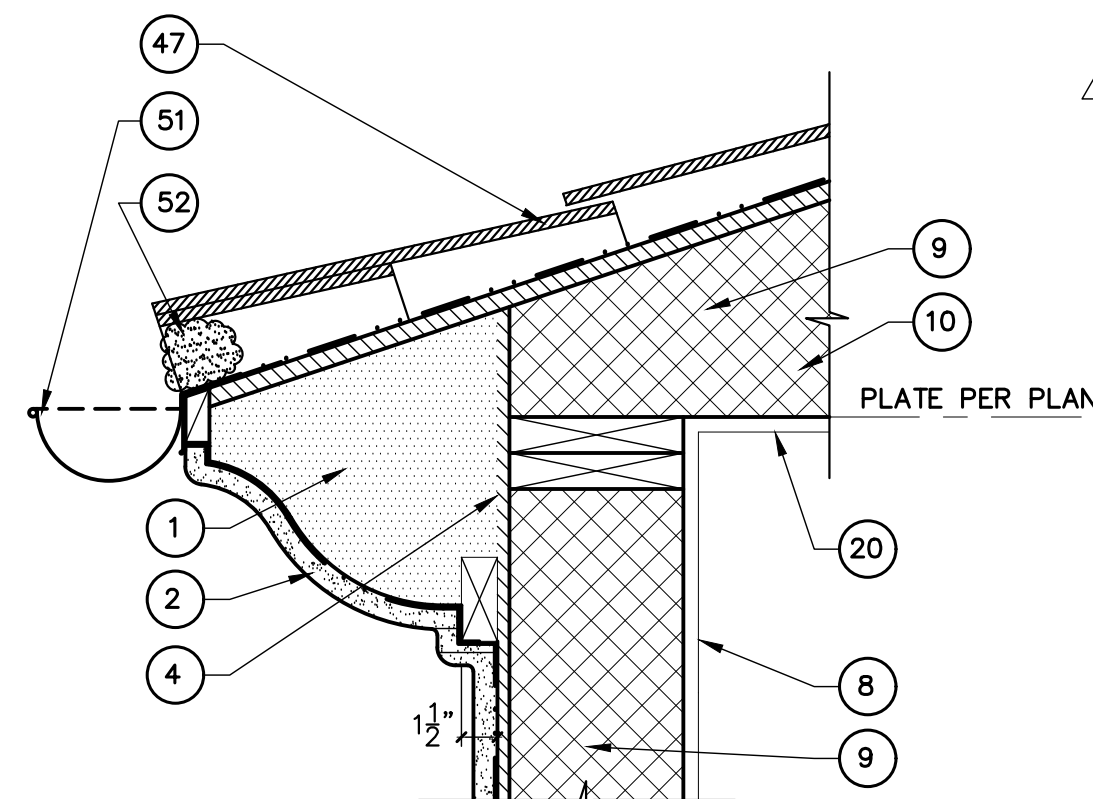
10 TYPICAL INTERIOR DOOR HEAD
SCALE: 1/2" = 1'-0"



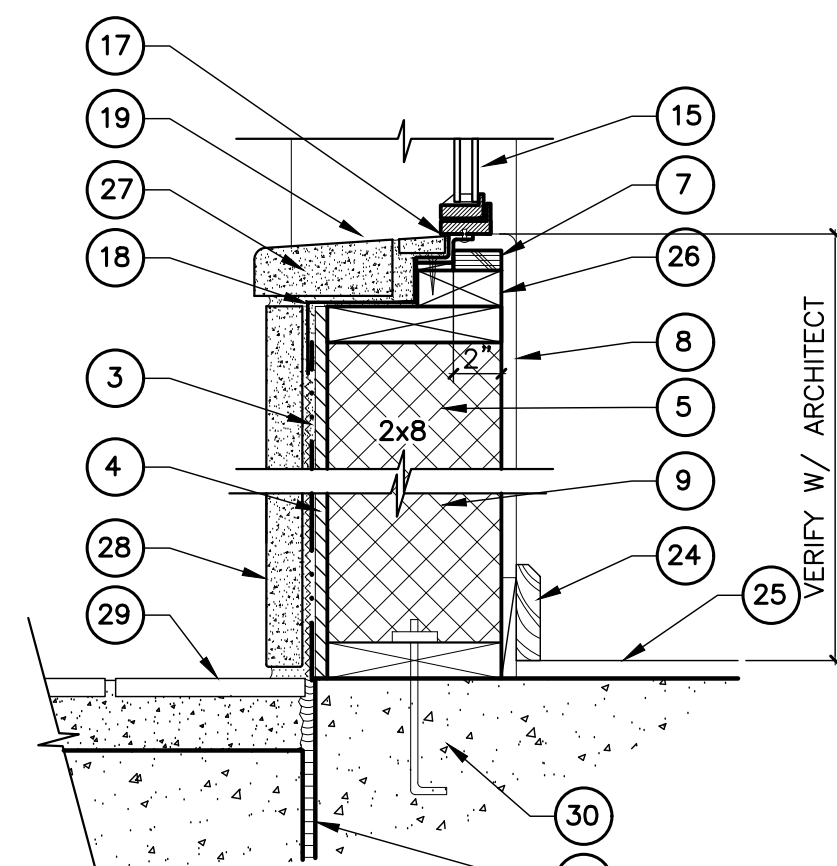
6 WDW. JAMB w/ STONE
SCALE: 1/2" = 1'-0"



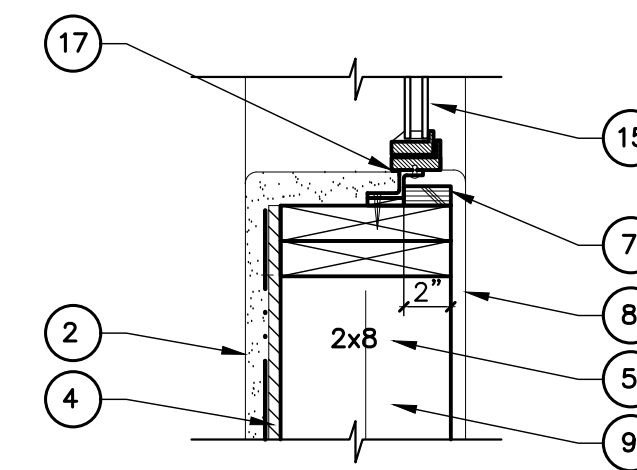
2 WDW. SILL @ KITCHEN
SCALE: 1/2" = 1'-0"



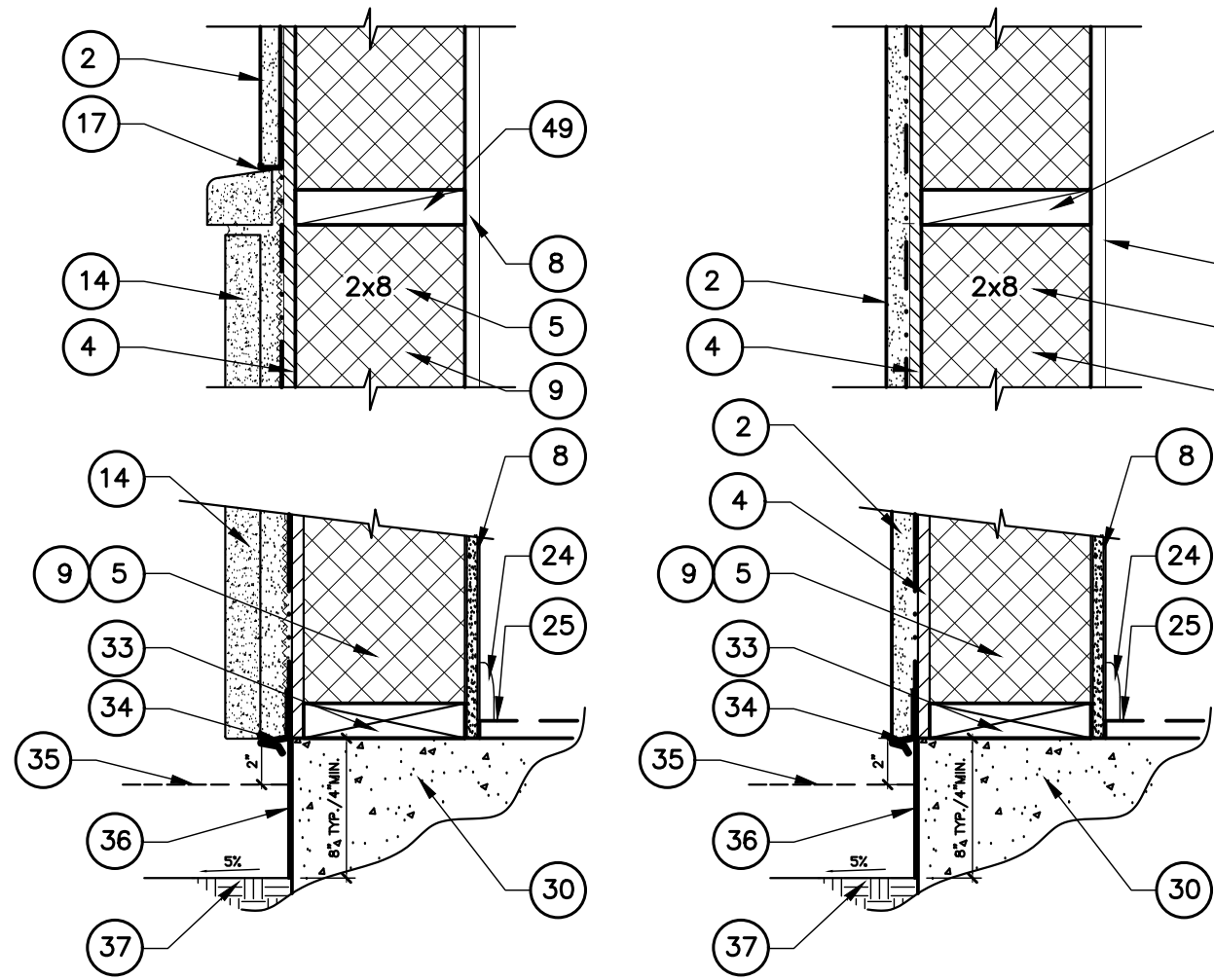
12 TYPICAL EXT. WALL DETAIL
SCALE: 1/2" = 1'-0"



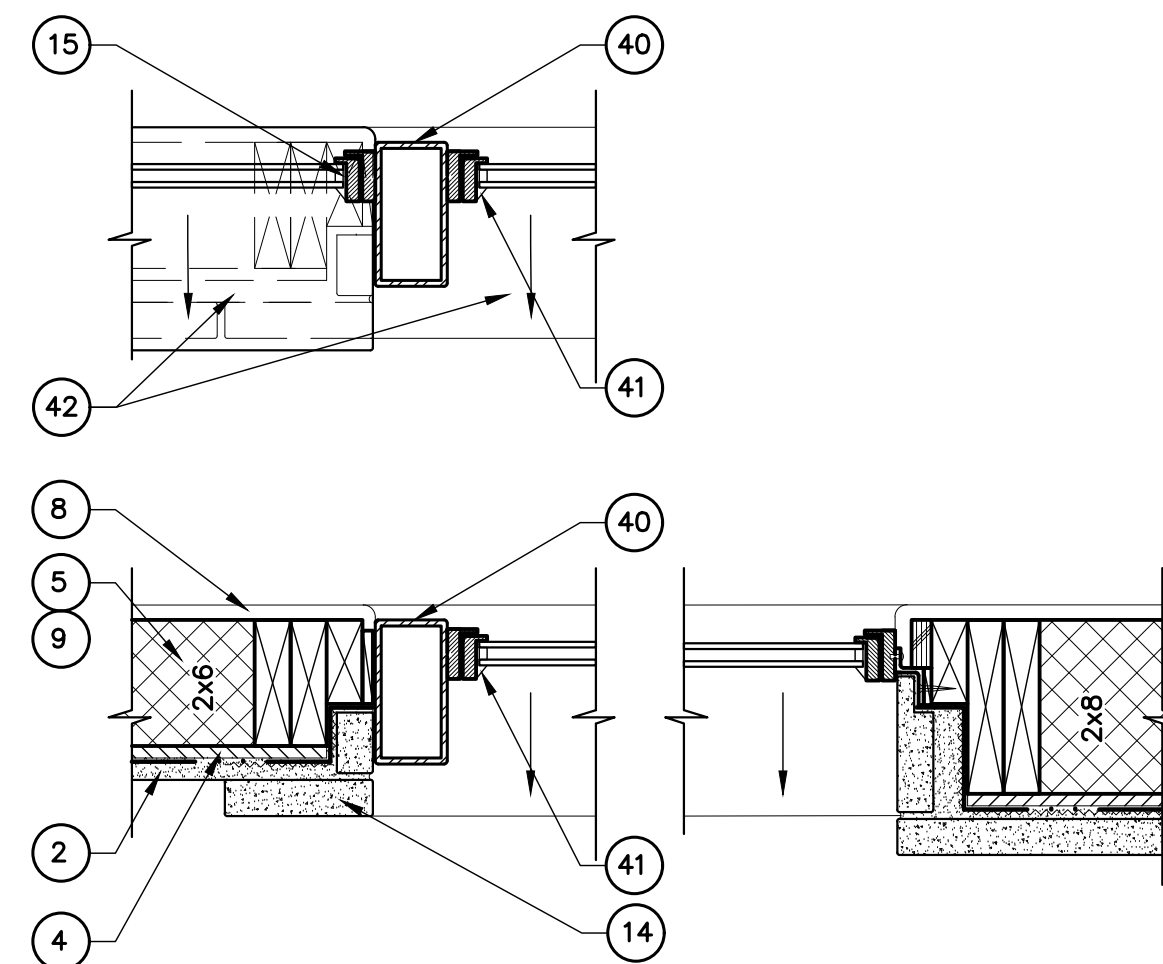
7 WDW. SILL w/ STONE
SCALE: 1/2" = 1'-0"



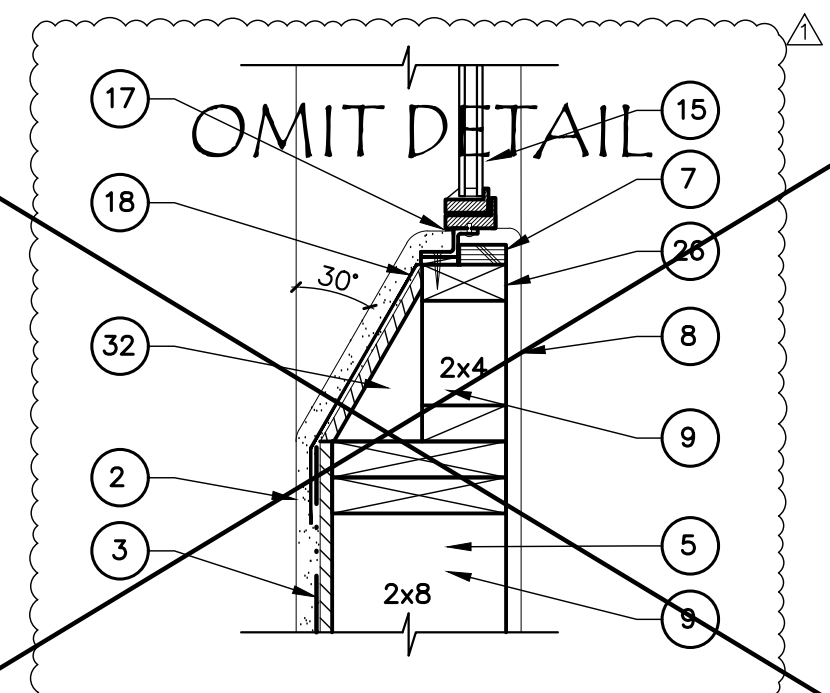
3 WDW. SILL @ KITCHEN
SCALE: 1/2" = 1'-0"



8 DOOR/WDW. JAMB
SCALE: 1/2" = 1'-0"



4 WDW. SILL @ KITCHEN
SCALE: 1/2" = 1'-0"



4 WDW. SILL @ KITCHEN
SCALE: 1/2" = 1'-0"

KEY NOTES:

- 1 STYROFOAM CORBEL PER ARCHITECT/OWNER
- 2 7" EXTERIOR MASONRY PLASTER OVER WATERPROOF MEMBRANE PER SPECS. OR MIN. 2 LAYERS GRADE "D" OR BETTER
- 3 WATERPROOF MEMBRANE PER SPECS
- 4 SHEAR PANEL PER STRUCTURAL
- 5 2x FRAMING PER PLAN
- 6 HEADER PER STRUCTURAL
- 7 3/4" PLYWOOD SHIM
- 8 5/8" DRYWALL W/ PLASTER SKIM COAT
- 9 R-19 WALL INSULATION PER ENERGY CALCS. R-30 ICYNENE ATTIC INSULATION PER ENERGY CALCS.
- 10 TRUSS TO BE DETERMINED
- 11 6"x6" DRAPE POCKET
- 12 LIGHT COVE PER LIGHTING SCHEDULE
- 13 ELDORADO STONE SURROUND (SEE MATERIALS LEGEND FOR DETAILS)
- 14 ELDORADO STONE VENER (SEE MATERIALS LEGEND FOR DETAILS)
- 15 METAL WINDOW PER PLAN
- 16 DOOR PER PLAN
- 17 SILICONE SEALANT w/ BACKING ROD
- 18 SHEET METAL PAN @ SILL LAP OVER W.P. MEMBRANE AS OCCURS
- 19 SLOPE @ SILL ONLY
- 20 CEILING AS OCCURS
- 21 TILE PER OWNER OVER MORTAR BASE OVER WATERPROOF MEMBRANE.
- 22 8"x8"x1/2" METAL "L" BRACKET
- 23 FINISH COUNTER TOP AS OCCURS
- 24 MDF BASE BOARD
- 25 FINISH FLOOR PER PLAN
- 26 2x NAILER
- 27 CARVED SLOPED STONE SILL OVER MORTAR
- 28 STONE BASE
- 29 EXTERIOR STONE FLOOR
- 30 FOUNDATION PER STRUCTURAL
- 31 EXPANSION JOINT MATERIAL
- 32 PLYWOOD GUSSET
- 33 PTDF PLATE PER PLAN
- 34 PROVIDE PLASTER SCREED NEAR TOP OF FOUNDATION
- 35 PAVED SURFACE AS OCCURS (2% MIN. SLOPE)
- 36 PLASTER FINISH COAT ON ALL EXPOSED FOUNDATION WALLS
- 37 FINISH GRADE. SLOPE 5% MINIMUM FROM BLD'G FOR 10'-0" MIN.
- 38 MILLED 2x K.D.V.G.D.F. JAMB
- 39 ZURN Z880 SLOT DRAIN w/ REMOVABLE DECORATIVE GRATE OR EQ. INSTALLED OVER MTL. FLASHING
- 40 3x6 HSS VERTICAL COLUMN PER STRUCT.
- 41 STEEL DOOR FRAME PER PLAN
- 42 SLOPED STONE SILL OVER MTL. PLAN
- 43 90# CAP SHEET @ RIDGE
- 44 CAP TILE SET w/ MORTAR OVER WP
- 45 PLYWD SH'T.G. PER STRUCTURAL
- 46 RAFTERS/TRUSS PER STRUCTURAL
- 47 2 PIECE MISSION TILE PER PLAN OVER WATERPROOF MEMBRANE PER SPEC.
- 48 "Z" BAR G.I. FLASH & COUNTERFLASH'G. LAP G.I. FLASHING w/ BUILDING PAPER
- 49 2x SOLID BLOCKING
- 50 G.I. CHANNEL FLASHING
- 51 COPPER GUTTER w/ LEAF GUARD
- 52 DBL. STARTER TILE w/ BIRD BLOCK MORTAR
- 53 ELASTA-TUFF 5000/6000 PED. WATERPROOFING SYSTEM BY TUFFLEX AT EXT. STAIRS AND ROOFDECK.

MISSION GROUP ARCHITECTS
 R. E. JOHNSON AIA

ARCHITECTURAL
 DETAILS

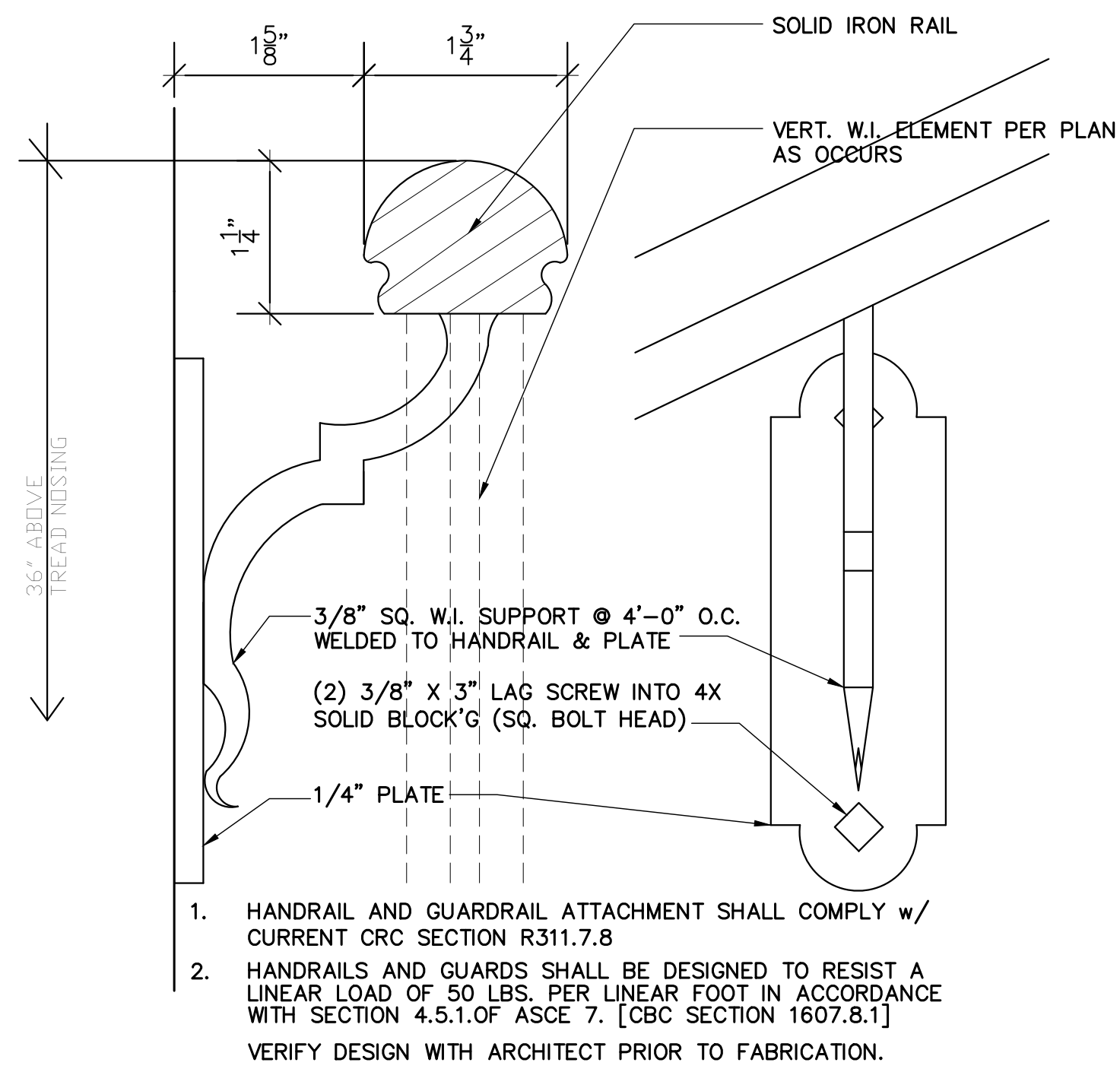
PROPOSED ADV FOR:
KISLAK RESIDENCE
 3627 CAMPANIL DRIVE
 SANTA BARBARA, CALIFORNIA 93109

SHEET
A6.1

DATE
 06/30/2025

REVISIONS

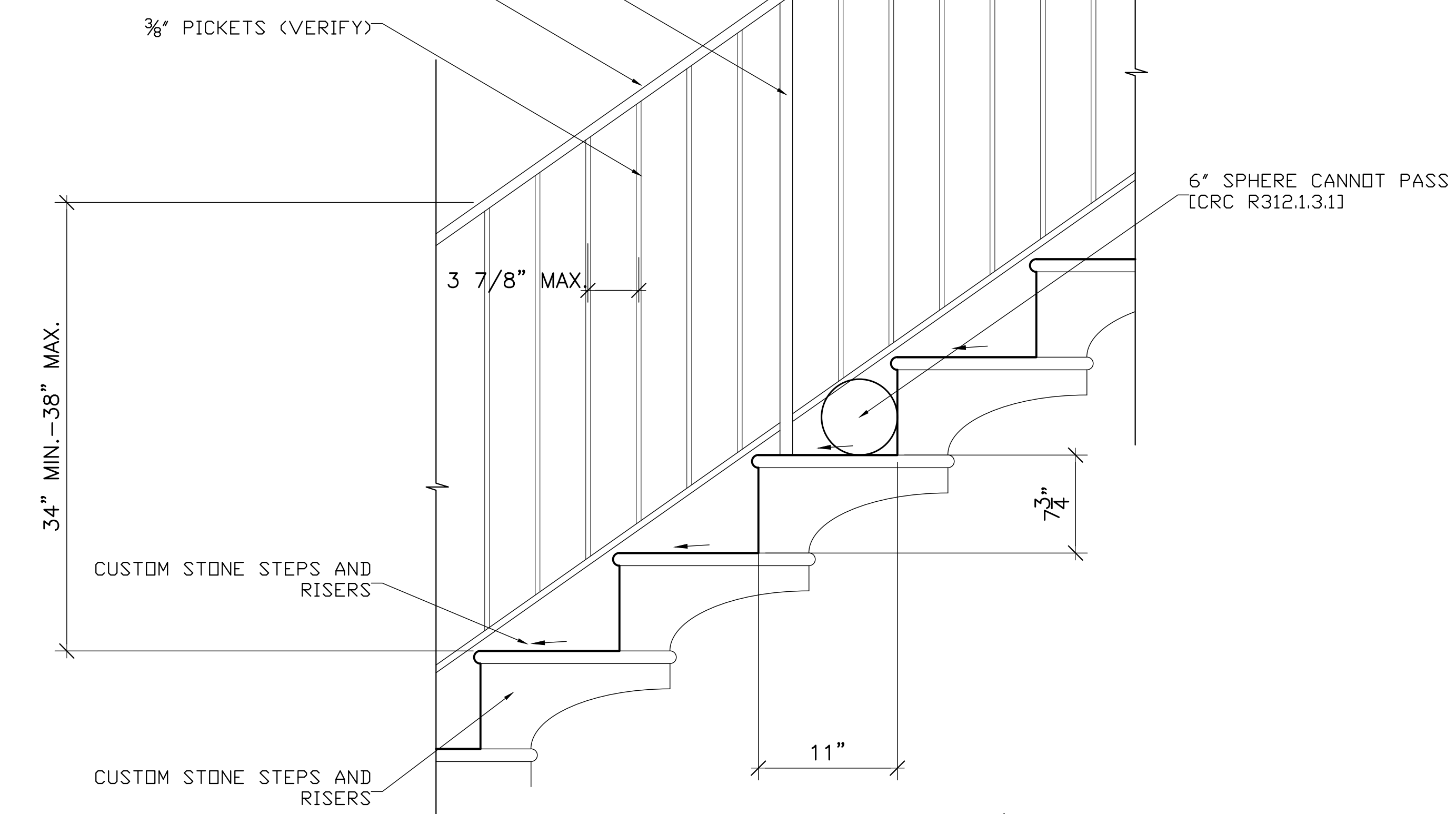
1230 COAST VILLAGE CIRCLE STE 110 SANTA BARBARA, CA 93108 805-969-5910 © COPYRIGHT MISSION GROUP ARCHITECTS



- HANDRAIL AND GUARDRAIL ATTACHMENT SHALL COMPLY w/ CURRENT CRC SECTION R311.7.8
- HANDRAILS AND GUARDS SHALL BE DESIGNED TO RESIST A LINEAR LOAD OF 50 LBS. PER LINEAR FOOT IN ACCORDANCE WITH SECTION 4.5.1 OF ASCE 7. [CBC SECTION 1607.8.1] VERIFY DESIGN WITH ARCHITECT PRIOR TO FABRICATION.

GUARDRAIL SHALL BE CAPABLE TO WITHSTAND LATERAL FORCE OF 20 PLF APPLIED TOP OF RAIL OR 25 PSF LATERAL FORCE @ INTERMEDIATE RAILS.(1611.1 & TABLE 16-B)

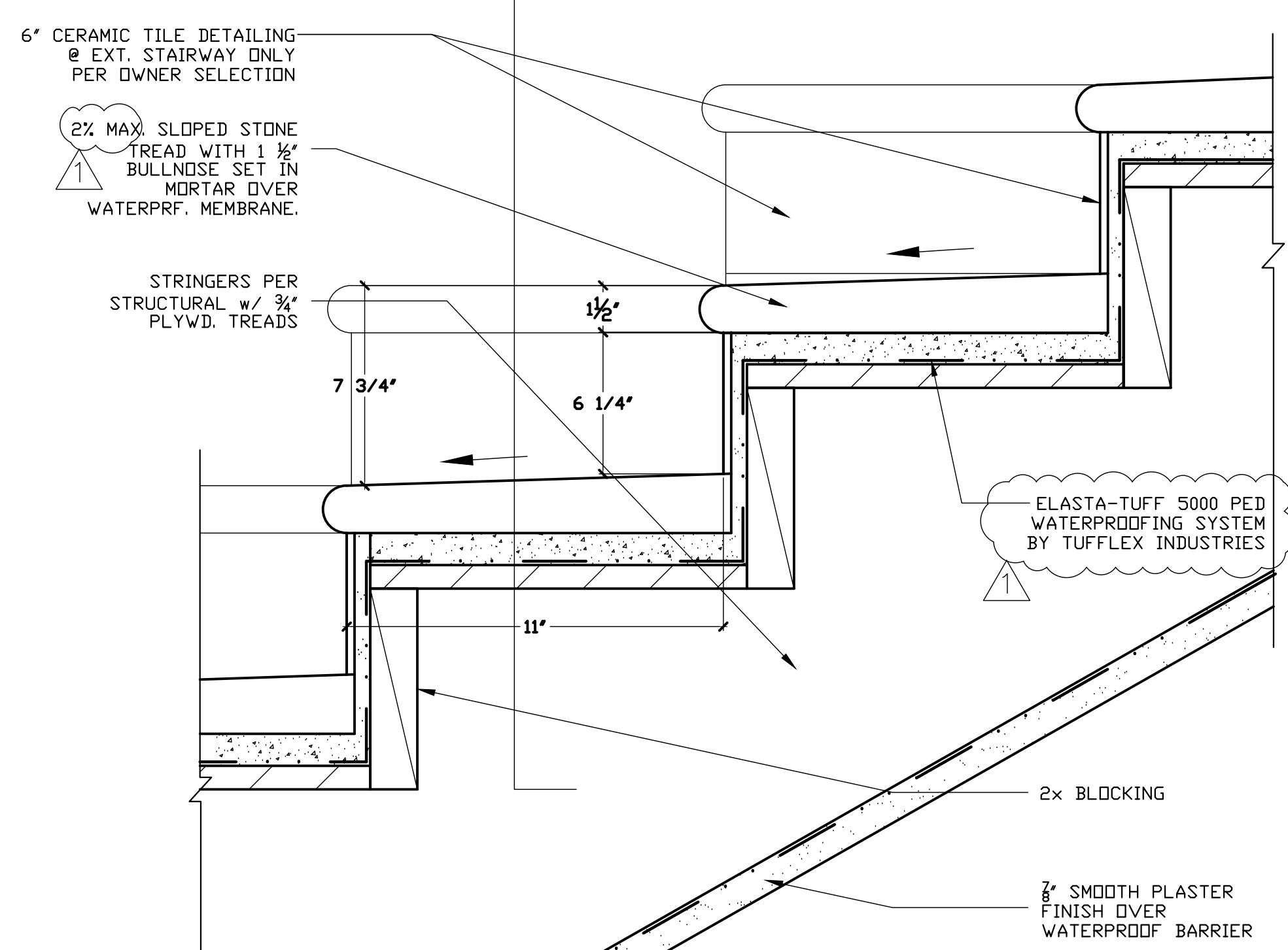
1" SUPPORTS SEE NOTES BELOW & DETAIL 4/A6.3
W.I. RAILING PER DETAIL 2/A6.2



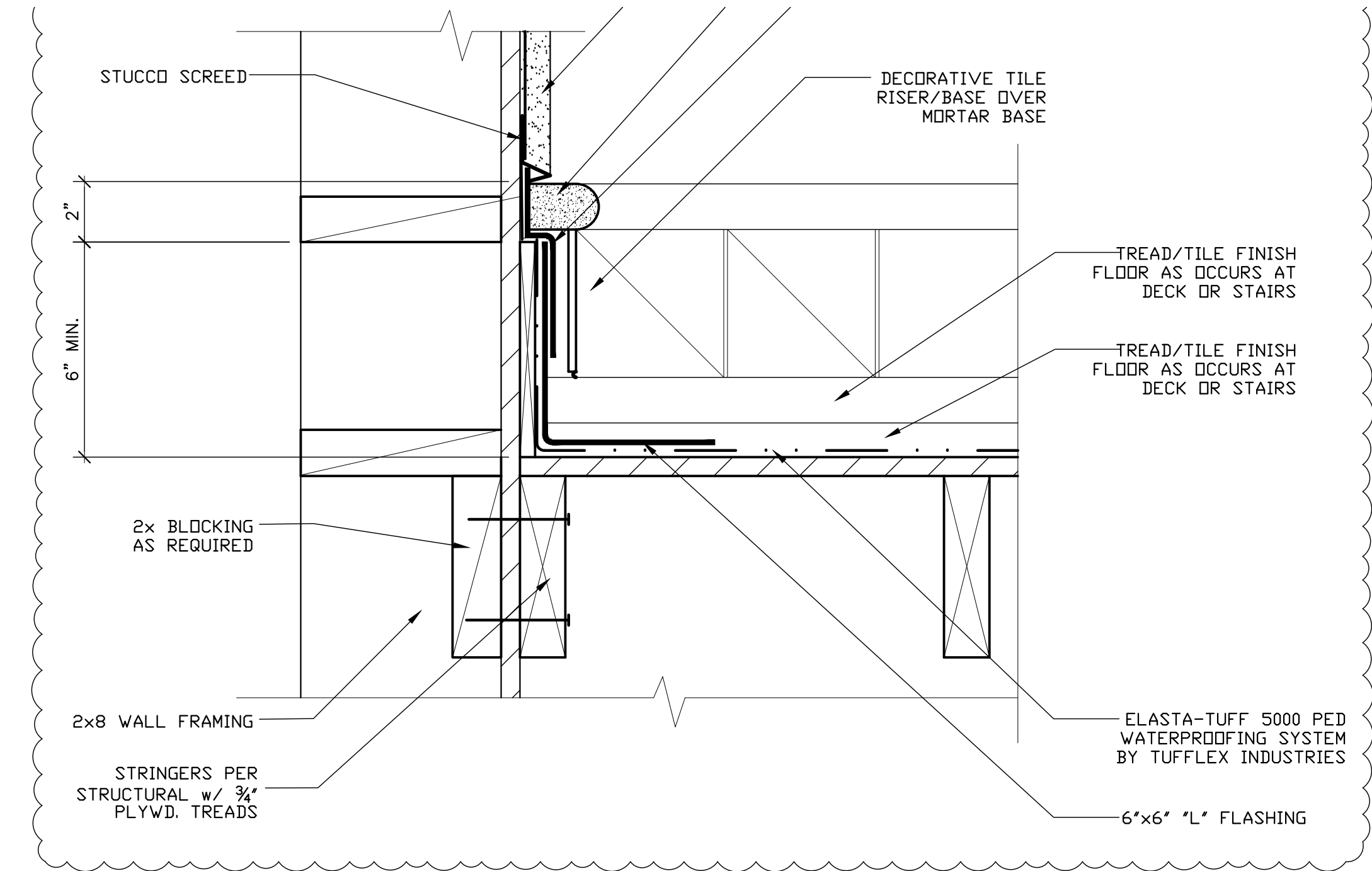
- HANDRAIL AND GUARDRAIL ATTACHMENT SHALL COMPLY w/ MOST CURRENT C.R.C. SECTION R312.
- HANDRAILS & GUARDRAILS SHALL BE DESIGNED TO RESIST A CONCENTRATED LOAD OF 200 LBS IN ACCORDANCE WITH SECTION 4.5.1 OF ASCE 7. [C.B.C. 1607.8.1.1]

NOTE:
VERIFY DESIGN WITH ARCHITECT PRIOR TO FABRICATION.

2 TYPICAL HANDRAIL STAIR DETAIL
SCALE: NO SCALE



1 TYPICAL STAIR DETAIL/ROOFDECK BASE WATERPROOFING DETAIL
SCALE: 1/2" = 1'-0"



KEY NOTES:

- STYROFOAM CORBEL PER ARCHITECT/OWNER
- 8" EXTERIOR MASONRY PLASTER OVER WATERPROOF MEMBRANE PER SPECS. OR MIN. 2 LAYERS GRADE "D" OR BETTER
- WATERPROOF MEMBRANE PER SPECS
- SHEAR PANEL PER STRUCTURAL
- 2x FRAMING PER PLAN
- HEADER PER STRUCTURAL
- 3/4" PLYWOOD SHIM
- 5/8" DRYWALL W/ PLASTER SKIM COAT
- R-19 WALL INSULATION PER ENERGY CALCS. R-30 ICYNENE ATTIC INSULATION PER ENERGY CALCS.
- TRUSS TO BE DETERMINED
- 6"x6" DRAPE POCKET
- LIGHT COVE PER LIGHTING SCHEDULE
- ELDORADO STONE SURROUND (SEE MATERIALS LEGEND FOR DETAILS)
- ELDORADO STONE VENEER (SEE MATERIALS LEGEND FOR DETAILS)
- METAL WINDOW PER PLAN
- DOOR PER PLAN
- SILICONE SEALANT w/BACKING ROD
- SHEET METAL PAN @ SILL LAP OVER W.P. MEMBRANE AS OCCURS
- SLOPE @ SILL ONLY
- CEILING AS OCCURS
- TILE PER OWNER OVER MORTAR BASE OVER WATERPROOF MEMBRANE.
- 8"x8"x1/2" METAL "L" BRACKET
- FINISH COUNTER TOP AS OCCURS
- MDF BASE BOARD
- FINISH FLOOR PER PLAN
- 2x NAILER
- CARVED SLOPED STONE SILL OVER MORTAR
- STONE BASE
- EXTERIOR STONE FLOOR
- FOUNDATION PER STRUCTURAL
- EXPANSION JOINT MATERIAL
- PLYWOOD GUSSET
- PTDF PLATE PER PLAN
- PROVIDE PLASTER SCREED NEAR TOP OF FOUNDATION
- PAVED SURFACE AS OCCURS (2% MIN. SLOPE)
- PLASTER FINISH COAT ON ALL EXPOSED FOUNDATION WALLS
- FINISH GRADE. SLOPE 5% MINIMUM FROM BLD'G FOR 10'-0" MIN.
- MILLED 2x K.D.V.G.D.F. JAMB
- ZURN Z880 SLOT DRAIN w/ REMOVABLE DECORATIVE GRATE OR EQ. INSTALLED OVER MTL. FLASHING
- 3x6 HSS VERTICAL COLUMN PER STRUCT.
- STEEL DOOR FRAME PER PLAN
- SLOPED STONE SILL OVER MTL. PAN
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- CAP TILE SET w/MORTAR OVER WP
- PLYWD SHTG. PER STRUCTURAL
- RAFTERS/TRUSS PER STRUCTURAL
- 2 PIECE MISSION TILE PER PLAN OVER WATERPROOF MEMBRANE PER SPEC.
- "Z" BAR G.I. FLASH & COUNTERFLASH'G. LAP G.I. FLASHING w/ BUILDING PAPER
- 2x SOLID BLOCKING
- G.I. CHANNEL FLASHING
- COPPER GUTTER w/ LEAF GUARD
- DBL. STARTER TILE w/ BIRD BLOCK MORTAR
- ELASTA-TUFF 5000/6000 PED WATERPROOFING SYSTEM BY TUFFLEX AT EXT. STAIRS AND ROOFDECK

PROPOSED ADV FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

DATE: 06/30/2025

SHEET: A6.2

REVISIONS

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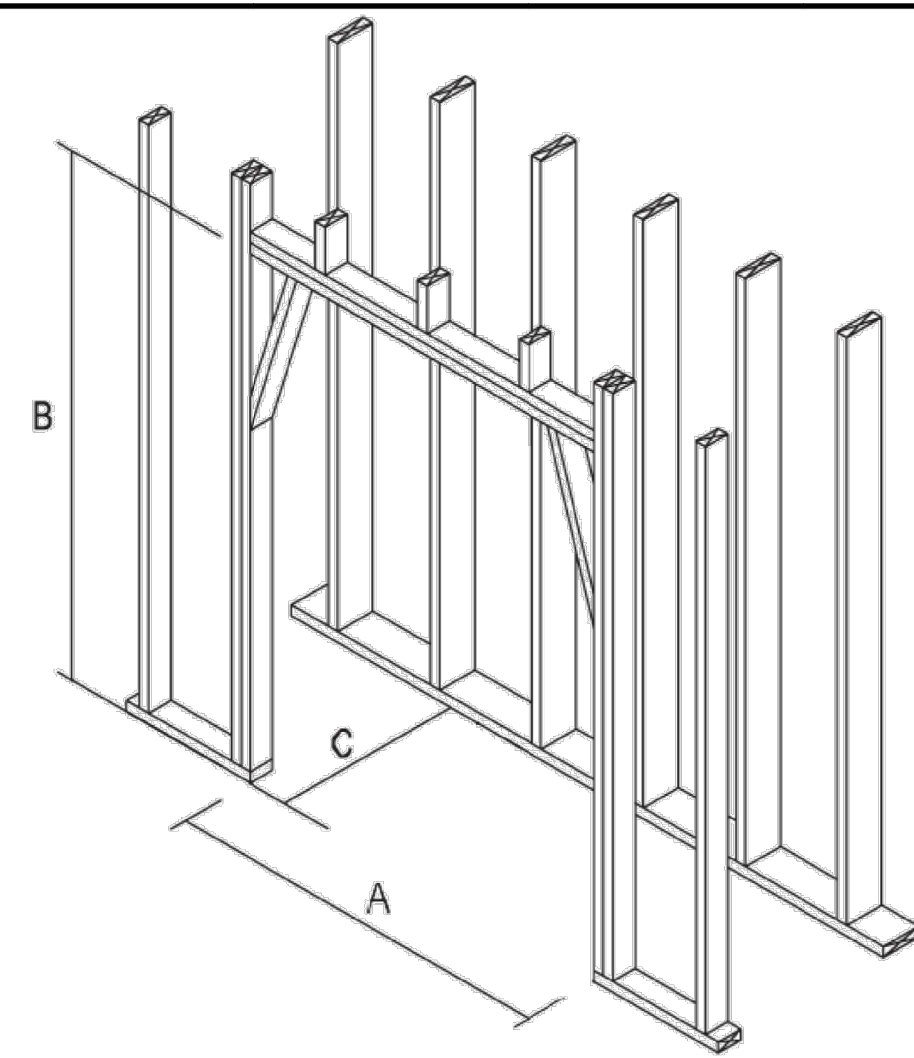
IF REVISION AIA OR OTHER PROFESSIONAL REGISTERED ARCHITECT HAS BEEN PROVIDED BY ARCHITECT OR OWNER, ARCHITECT SHALL BE RESPONSIBLE FOR VERIFYING THAT THE REVISIONS DO NOT VIOLATE ANY APPLICABLE CODES, ORDINANCES, REGULATIONS, OR STANDARDS. ARCHITECT SHALL BE RESPONSIBLE FOR VERIFYING THAT THE REVISIONS DO NOT VIOLATE ANY APPLICABLE CODES, ORDINANCES, REGULATIONS, OR STANDARDS. ARCHITECT SHALL BE RESPONSIBLE FOR VERIFYING THAT THE REVISIONS DO NOT VIOLATE ANY APPLICABLE CODES, ORDINANCES, REGULATIONS, OR STANDARDS.

Typical Installation Framing Dimensions

STANDARD	Width - A	Height - B	Depth - C
Model 36	46"	65"	26 3/4"
Model 42	52"	65"	26 3/4"
Model 46	56"	65"	26 3/4"

Notes:

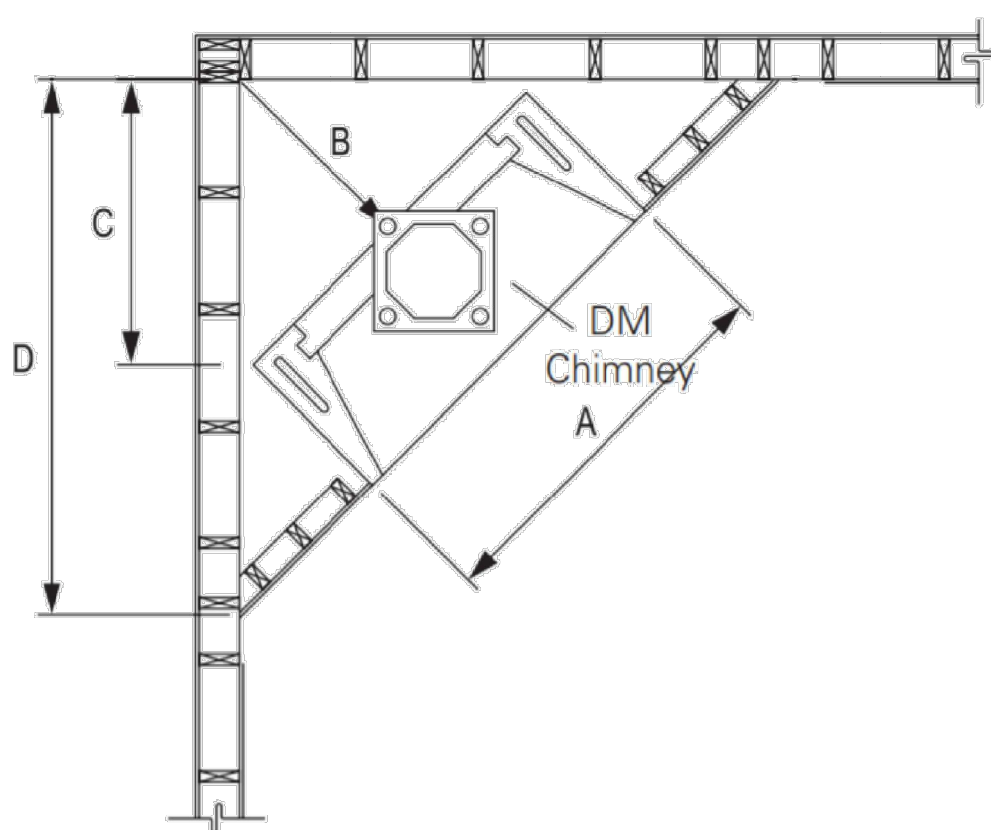
- "B" includes the 3" thick base plate.
- "Raised hearth" requires additional rough opening height at "B" equal to the height of the raised hearth detail.
- Rough framing dimension for width "A" allows for the required 1 1/2" clearance at the sides of the Fireplace.
- Rough framing dimension for Height "B" will need to be increased by 4" if the Firelite application is utilized.
- Rough framing dimension for depth "C" allows for the required 1 1/2" clearance at the back of the Fireplace. 26 3/4" is only for an interior wall as most exterior wall framing have insulation, even if the wall is 2x6, the foam they spray expands so typically 31" is allowed on an exterior wall



Corner Installation Framing Dimensions

The following chart of dimensions detail the positioning of an STANDARD Series fireplace in a corner. It also details the positioning of DM chimney where it must turn 45° degrees, if alignment is needed to overhead framing.

STANDARD	A	B	C	D
Model 36	46"	23 1/2"	32"	69"
Model 42	52"	26 1/2"	36"	73"
Model 46	56"	28 1/2"	39"	76"



To turn flue 45°, first set starting inner liner onto the top plate of the fireplace.

Next, set a DM outer casing onto the inner liner so that the outer casing is at 45° to the firebox and square to the overhead framing system. Run the vertical DM chimney through the overhead framing.

Offset blocks can be used, if necessary, to align with overhead framing before running the vertical DM chimney outer casing and liner.

NOTE: Support the third offset down to footings and at each third offset block thereafter.

MODULAR REFRACTORY FIREPLACE

STANDARD: 36" 42" 46"
80S36 80S42 80S46

MADE IN USA
Chesapeake, VA 23323

Headquarters
Jacksonville, FL 32256

CERTIFIED TO: UL 127, ULC S610

WHI

CLEARANCE TO COMBUSTIBLES:

SMOKE DOME FRONT AND ISOKERN CHIMNEY UNIT SIDES AND REAR	= 0 in.
COMBUSTIBLE SHEATHING ABOVE OPENING	= 1.5 in. (38mm)
TOP SHEATHING OR TRIM TO OPENING SIDES	= 8 in. (203mm)
MANTEL ABOVE OPENING	= 8 in. (203mm)
OPENING TO SIDEWALL	= 14 in. (356mm)
HEARTH EXTENSION BEYOND FRONT	= 26 in. (660mm)
HEARTH EXTENSION BEYOND SIDES	= 20 in. (508mm)
COMBUSTIBLE FLOOR (MUST USE FIRE-LITE APPLICATION)	= 12 in. (305mm)
INSULATION FROM FIREBOX	= 4 in. (102mm)
	= 3 in. (76mm)

USE SOLID WOOD FUEL OR LISTED DECORATIVE GAS VENTED OR UNVENTED APPLIANCE. ALSO FOR USE WITH LISTED METAL CHIMNEY. FIRE-LITE APPLICATION TO USE ECO-STEEL CHIMNEY ONLY.

DO NOT USE A FIREPLACE INSERT OR OTHER PRODUCTS NOT SPECIFIED FOR USE WITH THIS PRODUCT. "WARNING" THIS FIREPLACE HAS NOT BEEN TESTED FOR USE WITH GLASS DOORS. TO REDUCE THE RISK OF FIRE OR INJURY, DO NOT INSTALL GLASS DOORS. IF DOORS ARE USED, OPERATE FIREPLACE WITH DOORS FULLY OPEN, WHEN BURNING A DECORATIVE GAS APPLIANCE IN THE FIREPLACE, LOCK THE DAMPER TO THE FULLY OPEN POSITION. DO NOT OPERATE AN UNVENTED GAS LOG SET IN THIS FIREPLACE WITH THE CHIMNEY REMOVED.

SEE INSTALLATION AND OPERATING INSTRUCTIONS FOR THIS MODEL AND ICC # ESR-2316.

CONTACT BUILDING OFFICIAL PRIOR TO INSTALLATION
REFER TO INTERTEK'S DIRECTORY OF BUILDING PRODUCTS FOR DETAILED INFORMATION.

Clearance to Combustible Trim - 36, 42 & 46 Models

Hearth Extensions

All STANDARD 36, 42 and 46 Fireplaces shall have hearth extensions of brick, concrete, stone, tile or other code approved noncombustible material. Suitable hearth extension material for the fireplaces shall be placed on the hearth extension's noncombustible substrate and must extend to at least twenty inches (20") in front of the fireplace's finished opening and must extend to at least twelve inches (12") beyond the sides of the finished fireplace opening. (Figure 68)

WARNING: The noncombustible hearth extension, by code, must sit on noncombustible substrate which shall have no wood underpinnings.

This means that off-grade wood floor systems shall be constructed in such a way that all wood floor joists and sub-flooring shall stop twenty inches (20") out from the front of the firebox. (Figure 69)

Mantle and Mantle Shelf Clearances

Fireplaces are subject to the same building code safety clearances to combustible trim as with any radiant heat fireplace.

All combustible trim shall be kept at least eight inches (8") from the finished fireplace opening.

Combustible trim located along the sides of the fireplace opening, which project more than one and one-half inches (1 1/2") from the face of the fireplace, shall have additional clearance from the eight inches (8") equal to the projection.

Combustible projecting mantles - up to twelve inches (12") of projection - shall not be placed less than fourteen inches (14") from the top of the fireplace opening. Combustible mantles which project more than twelve inches (12") from the face of the fireplace, shall have additional clearance from the fourteen inches (14") equal to the projection.

Note: The local authority having jurisdiction may require greater clearances for projecting combustible mantle shelves. Be sure to check local building codes regarding required clearances to projecting combustible mantles.

Adjoining Walls. Side walls and walls to rooms adjoining fireplace installations cannot be closer than twenty-six inches (26") to the finished fireplace opening.

Note: "Clearance to Combustible Trim" are those distances required to ensure that a fireplace mantle or facing will not catch fire. In most cases the distances should also be adequate to prevent any discoloration or warping due to heat. However each installation presents a unique and completely different set of circumstances involving many variables.

These include paint or finish composition, previous exposure to heat, methods and quality of construction, air flow patterns, etc. Because of these variables, the manufacturer does not guarantee that heat warping or discoloration will never occur.

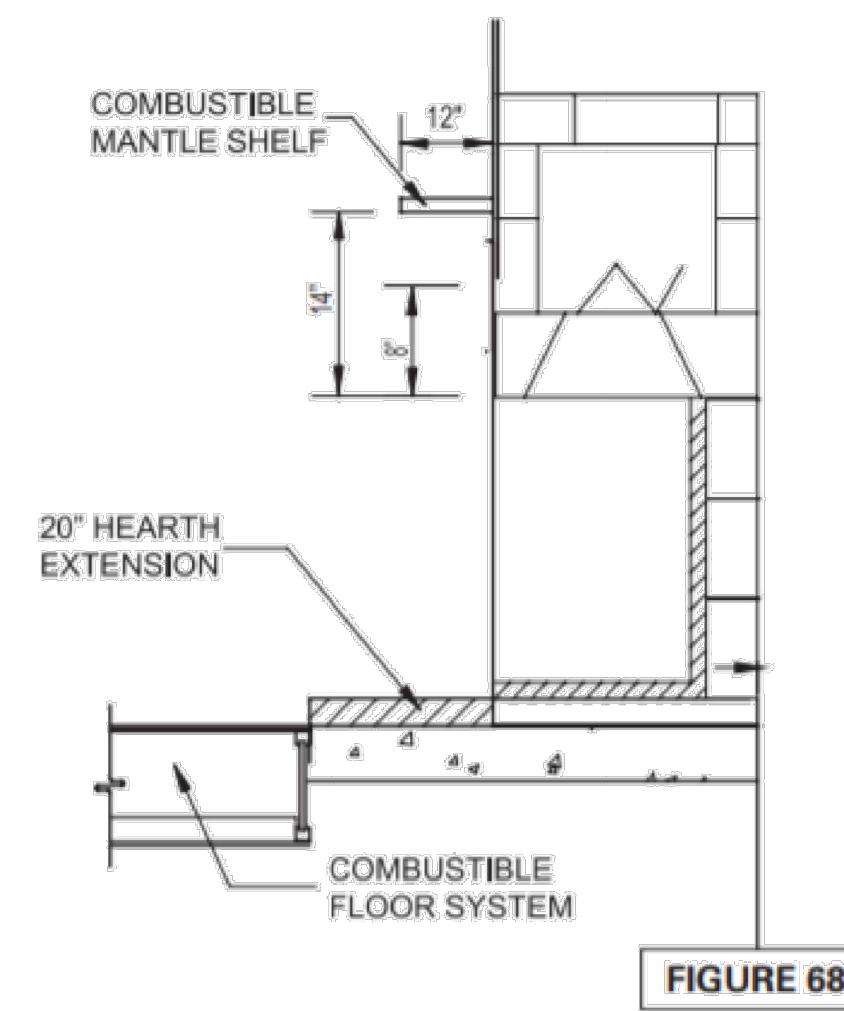


FIGURE 68

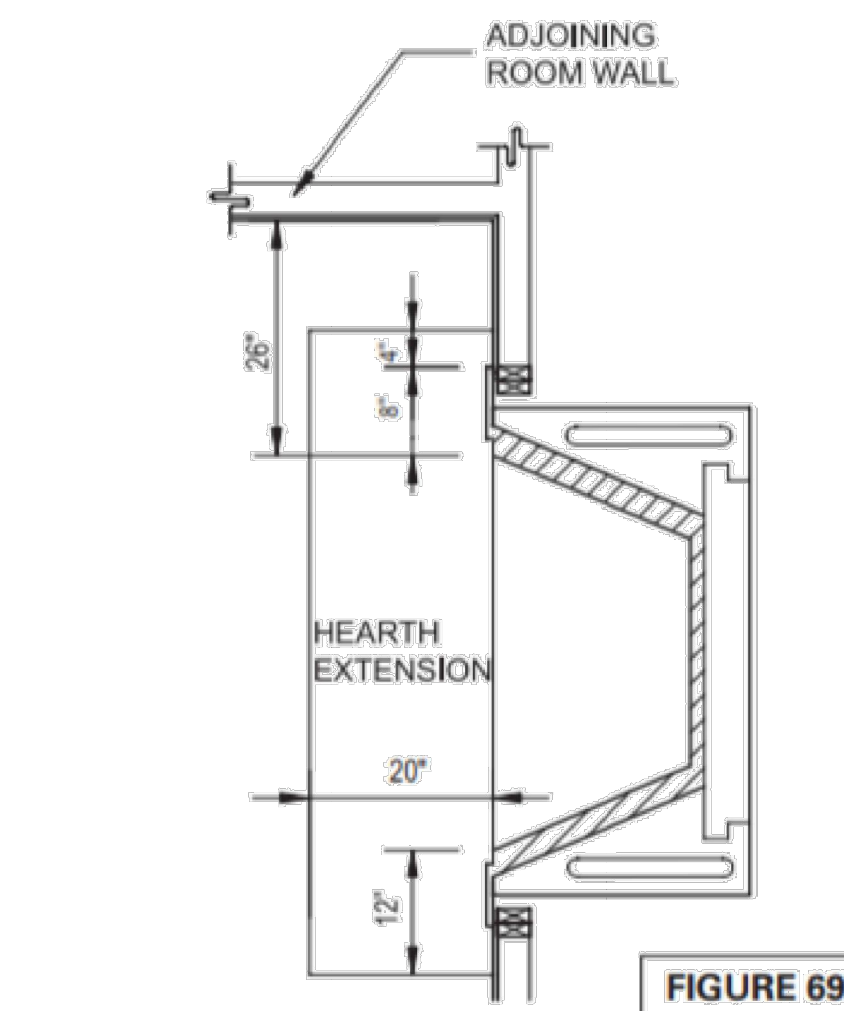


FIGURE 69

Standard Series (All Fuel) Specifications

MANUFACTURER:

ISOKERN Fireplace and Chimney Systems, North America distribution by Earthcore Industries, LLC, Jacksonville, Florida (Telephone 800-642-2920)

PRODUCT DESCRIPTION:

Modular refractory masonry precast fireplace and chimney components.

- Designed for field assembly as a fireplace and chimney unit.
- All interlocking parts necessary for assembly of a complete firebox and smoke dome.
- Interlocking double module chimney components (DM).

MATERIALS:

- Light weight concrete of a proprietary mixture of Icelandic volcanic aggregate and cement for precast firebox, chimney block and flue liner components.
 - Compressive Strength: Firebox and Chimney Block: 972 psi.
 - Compressive Strength: Flue Liner: 1175 psi.
- Premixed (dry) EARTHCORE ADHESIVE.
 - Tensile strength: 807 psi; Compressive strength: 2460 psi.
 - Tested per ASTM C109, ASTM C307, and ANSI 118.4.
- Standard 1-1/8" thick high temperature refractory brick to line firebox interior
- Standard cast iron poker-style damper (optional top mount or in-line damper available).

INSTALLATION:

- Reference manufacturer's installation instructions for standard configurations, weights, sizes and installation details.
- Suitable masonry foundation and noncombustible hearth extensions must be provided.
- Unit to be assembled on site per manufacturer's illustrated instructions.
 - Premixed EARTHCORE ADHESIVE is used at all joints between components.
 - Firebox to be lined with a minimum 1-1/8" thick rated firebrick.
- A 1-1/2" minimum clearance to combustible materials is required.
- A 3" minimum clearance to insulation required.

CERTIFICATION:

- Warnock Hersey/Intertek Testing Services Report No. 632-912500; 3159656MID008; 3082504-T1
- Meets or exceeds UL 103HT; UL 103; UL 127; UL 1777; ULC 5610; ANSI 223.1.

CODE COMPLIANCE:

- SBCCI NO. 9626
- LARR NO. 25483
- NYC-MEA 241-90-E
- ICC REPORT NO. ESR 2316
- IBC 2006, IRC 2006, IMC 2006

NOTE: Isokern components are a natural material and slight variations in dimensions may occur. These should be no more than 1/8".

SCAN QR CODE FOR LATEST INSTALLATION MANUALS



		PROJECT: 36" STANDARD (80s36)	
UNLESS OTHERWISE STATED, ALL DIMENSIONS ARE IN MILLIMETERS		DESCRIPTION: GENERAL INFORMATION AND SPECIFICATIONS	
UNDEFINED TOLERANCES:	THIRD ANGLE PROJECTION:	MATERIAL:	
LIN. X ± X ± X ± XX ±		DRAWN: K.B.H.	DATE: 4/21/22
APPROVED:		SIZE: D	SCALE: N.T.S.
8950 Philips Industrial Blvd. Jacksonville, FL 32256		DRAWING #: 80s36	REV: D

PROPOSED ADV FOR: KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

DATE: 06/30/2025

SHEET: A6.5

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MISSION GROUP ARCHITECTS
R. E. JOHNSON AIA

REVISIONS

LICENSED ARCHITECT
STATE OF CALIFORNIA
EX. 475

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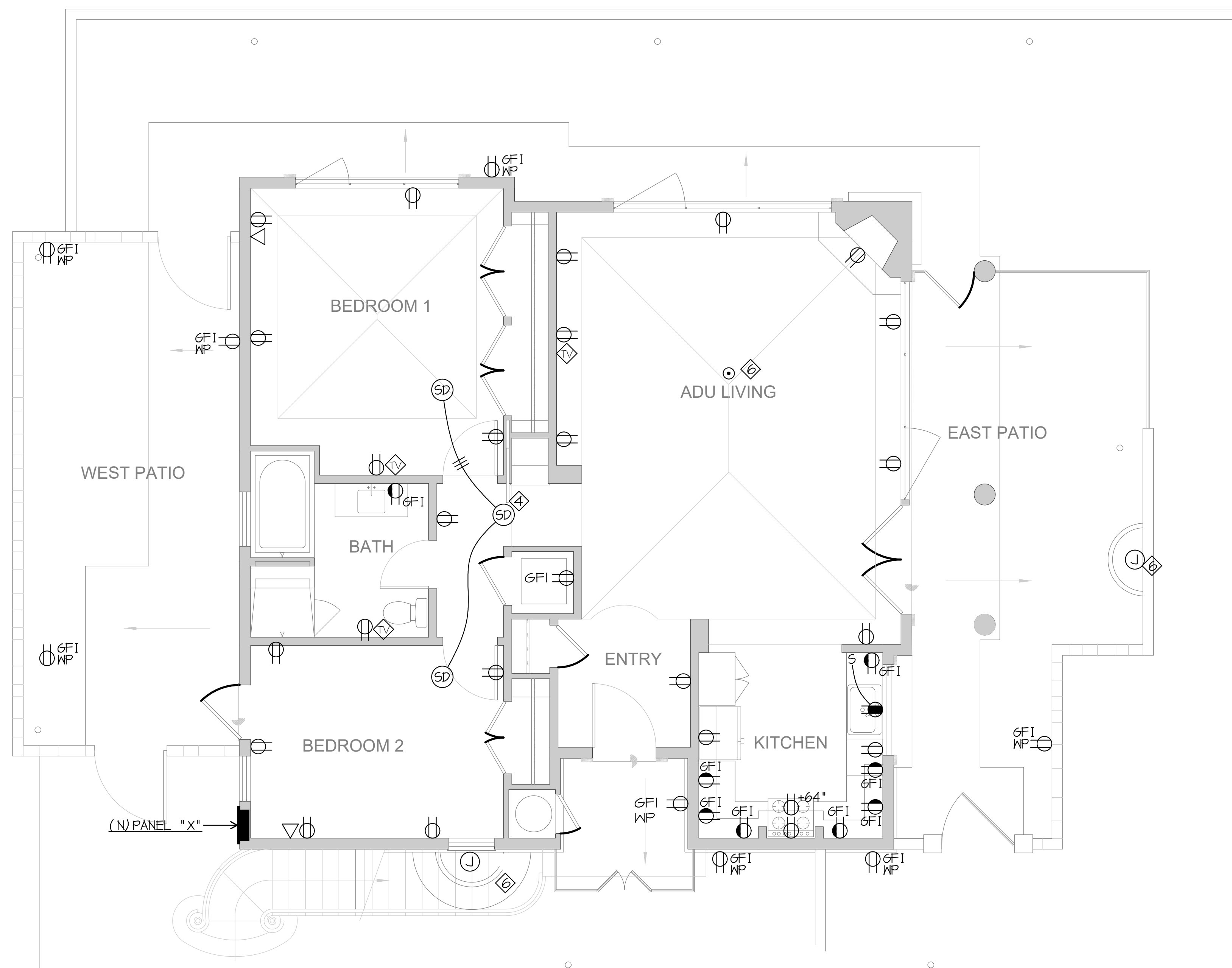
REVISIONS	BY

JMPE
 ELECTRICAL ENGINEERING
 LIGHTING DESIGN
 CA REGISTRATION NO. E13083
 22311
 627 OLIVE STREET
 SANTA BARBARA, CA 93105
 (805) 569-8216
 FAX (805) 569-2405
 e-mail: jmaloney@jmpe.net
 www.jmpe.net



ELECTRICAL NOTES

- ALL OUTLETS IN DWELLING UNITS SHALL BE PROTECTED BY A LISTED ARC FAULT CIRCUIT INTERRUPTER, COMBINATION TYPE PER CEC 210.12
- TAMPER RESISTANCE RECEPTACLE SHALL BE INSTALLED IN DWELLING UNITS PER CEC 406.11
- SMOKE DETECTORS SHALL BE 120V, INTERCONNECTED, PHOTOELECTRIC/ION UNITS WITH BATTERY BACK UP
- HALLWAY DETECTORS SHALL BE COMBINATION SMOKE AND CARBON MONOXIDE DETECTOR, PART #KIDDE KN-COSM-BA
- VERIFY LOCATION OF FLUSH FLOOR RECEPTACLE PRIOR TO ROUGH IN
- CONNECT FOUNTAIN VIA GFCI



POWER PLAN

SCALE: 1/4" = 1'-0"



PROPOSED ADU FOR:
 KISLAK RESIDENCE
 3627 CAMPANIL DRIVE
 SANTA BARBARA, CA 93104

POWER PLAN

DATE 05-08-2023
 SCALE 1/4" = 1'-0"
 DRAWN ARRED

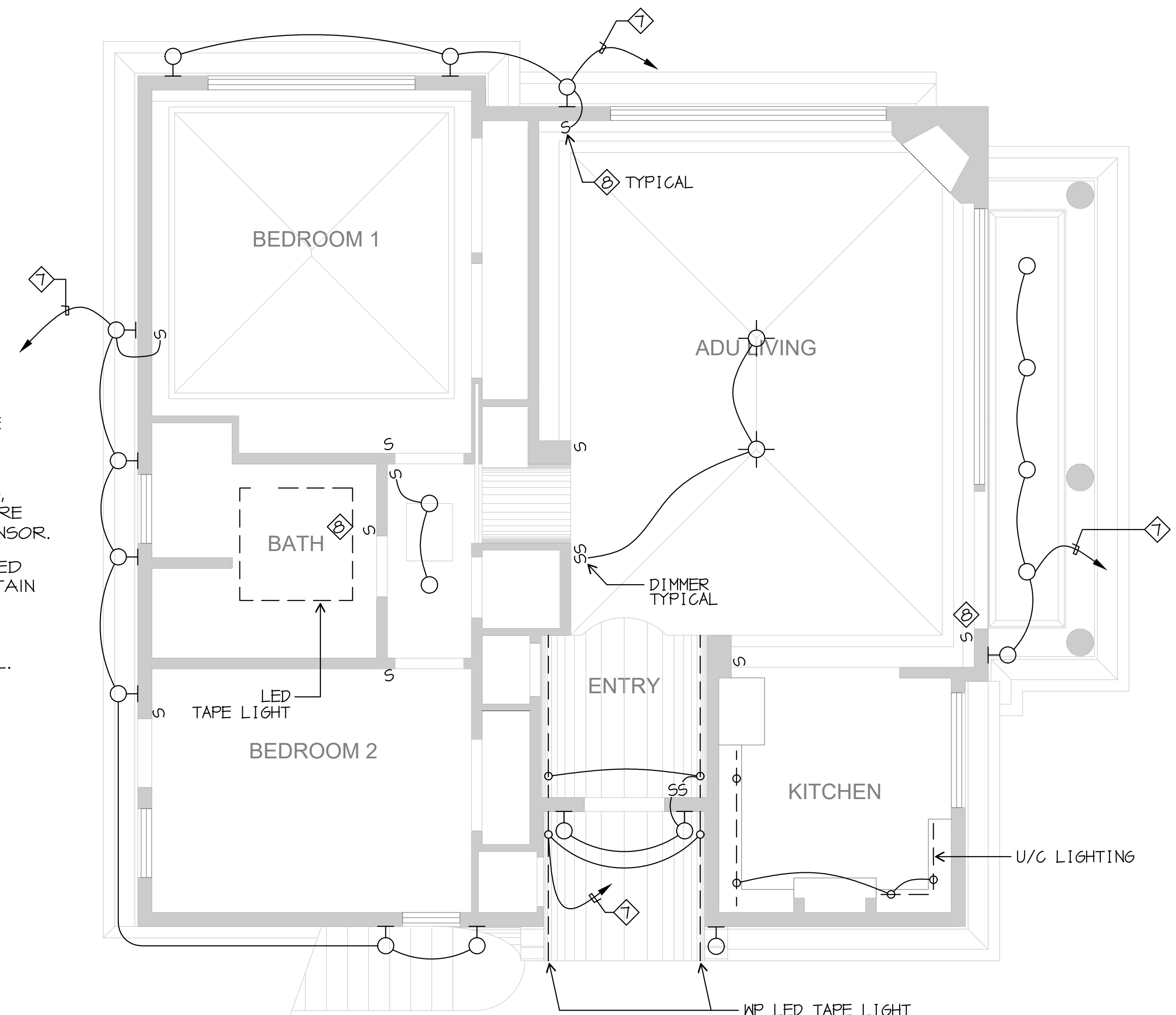
SHEET E2

SEE SHEET ID-601 FOR
LIGHT FIXTURE LOCATIONS
+ SPECIFICATIONS

LIGHTING NOTES

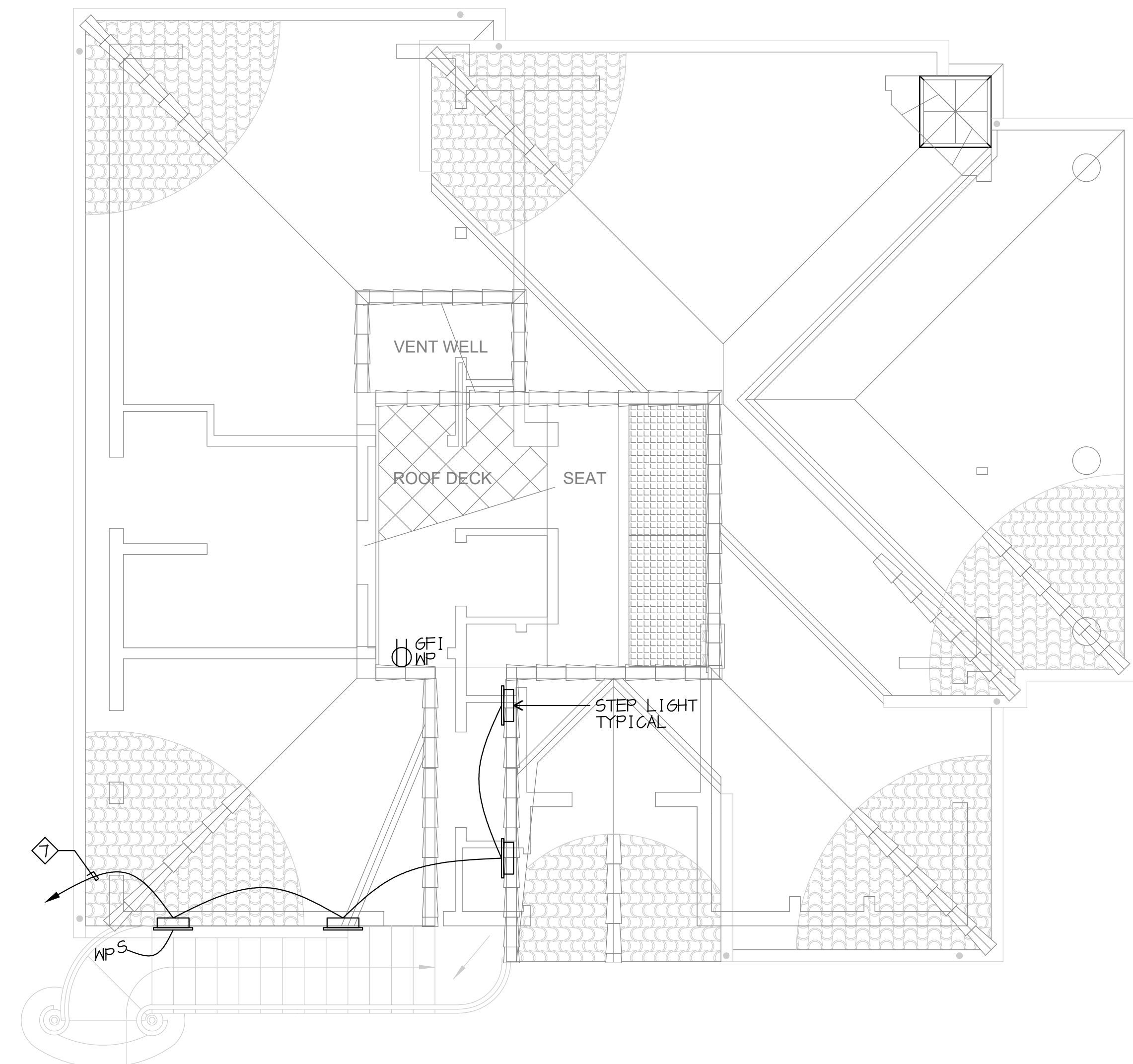
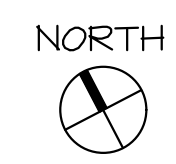
1. ALL INSTALLED LIGHTING TO BE HIGH EFFICACY PER REQUIREMENTS OF 2019 CEC SECTION 150.0(k) AND JOINT APPENDIX JAB.
2. BUILDER SHALL PROVIDE THE HOMEOWNER WITH A LUMINAIRE SCHEDULE THAT INCLUDES A LIST OF ALL INSTALLED LAMPS AND LUMINAIRES.
3. ANY JAB COMPLIANT LAMP MUST BE CONTROLLED BY A VACANCY SENSOR OR DIMMER BASED ON TYPE OF LUMINAIRE OR LAMP INSTALLED.
4. RECESSED FIXTURES SHALL BE IC RATED AND ASTM E283 CERTIFIED. RECESSED FIXTURES SHALL NOT CONTAIN A SCREW BASE SOCKET AND MUST BE INSTALLED WITH A LIGHT SOURCE THAT IS JAB CERTIFIED. SHALL NOT CONTAIN LIGHT SOURCES THAT ARE LABELED "NOT FOR USE IN ENCLOSED FIXTURES" OR "NOT FOR USE IN RECESSED FIXTURES."
5. IN BATHROOMS, LAUNDRY ROOMS, GARAGES, AND UTILITY ROOMS, AT LEAST ONE LUMINAIRE SHALL BE CONTROLLED BY A VACANCY SENSOR.
6. THE NUMBER OF ELECTRICAL BOXES LOCATED ABOVE FINISHED FLOOR THAT DO NOT CONTAIN A LUMINAIRE OR OTHER DEVICE SHALL NOT EXCEED THE NUMBER OF BEDROOMS. THESE BOXES MUST BE SERVED BY A DIMMER, VACANCY SENSOR, OR FAN SPEED CONTROL.

- ◊ HOMERUN VIA LIGHTING CONTROL SYSTEM
- ⊞ LIGHTING CONTROL KEY PAD



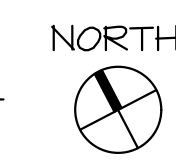
LIGHTING PLAN

SCALE: 1/4" = 1'-0"



ELECTRICAL ROOF PLAN

SCALE: 1/4" = 1'-0"



PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CA 93109

LIGHTING PLAN
ELECTRICAL ROOF PLAN

DATE 05-08-2023

SCALE 1/4" = 1'-0"

DRAWN ARRED

SHEET

E3

6. Structural Wood

- A. General: All structural wood work shall be done in accordance with the National Design Specification for Wood Construction (NDS-2018 & SDPWS 2018) and the CBC 2019 edition, chapter 23.
- B. Structural members shall not be cut or drilled unless specifically noted or detailed.
- C. All lumber materials, unless otherwise noted or shown, shall be as follows:
 - 1. Foundation sill, nailers, and ledgers embedded in or in direct contact with concrete or masonry shall be pressure treated D.F.
 - 2. 6x Beam: No. 1 D.F.
 - 3. Framing Members:
 - a. Joists, headers, plates, subpurlins, nailers, and blocking shall be D.F. #2 or better unless noted otherwise.
 - b. 2x4 studs shall be D.F. construction grade or better.
 - c. All 2x joists and rafters are to be solid blocked at all points of bearing. Solid blocking is required at 8'-0"oc unless continuously braced on bottom edge. Blocking may be omitted for ceiling joists and roof rafters eight inches and less in depth, unless noted otherwise on plans.
 - 4. Glue-Laminated Beams:
 - a. Glulam beams shall conform to industrial grade 24F-V8 unless noted otherwise. Use architectural grade where noted on architectural plans.
 - b. All glulam beams shall be fabricated by a licensed fabricator and shall have AITC certifications.
 - c. Adhesives and laminations shall conform to AITC Standard AI90.1.
- 5. Prefabricated Joist Notes:
 - a. TJI joists shall be as manufactured by Trus-Joist (by Meyerhaeuser) and shall have solid plywood or Oriented Strand Board webs as recommended by the manufacturer for the loads and spans indicated.
 - b. Microlam beams shall be as manufactured by Trus-Joist (by Meyerhaeuser). Multiple pieces shall be joined as follows:
 - 2 pieces (3) rows 16d nails @ 12"oc
 - 3 or more pieces (2) rows 1/2" M.B. @ 12"oc
 - c. The joists indicated on the plans are based on the manufacturer's load tables and must be verified by the manufacturer. Submit shop drawings and calculations for each different joist type, span, and loading condition.
 - d. Install all joists and beams per the manufacturer's recommendations including all necessary stiffeners, bridging, blocking, and hangers.
 - e. Flanges shall be designed to accommodate closely spaced plywood roof or floor diaphragm nailing where occurs on plans.
 - f. Design loads shall be as indicated on plans.
 - g. Parollam beams shall be as manufactured by Trus-Joist (by Meyerhaeuser) and shall have an E minimum = 2000 ksi.
- 6. Plywood Sheathing:
 - a. Roof Sheathing shall be USDOC PSI or PS2 rated plywood or OSB as indicated on the plans. Plywood shall be fabricated using exterior glue.
 - b. Floor Sheathing shall be USDOC PSI or PS2 rated plywood or OSB as indicated on the plans. Glue all contact surfaces. Nail with ring shank or screw shank nails.
 - c. Wall Sheathing shall be USDOC PSI or PS2 rated plywood or OSB as scheduled on the plans. Block all panel edges.
 - d. Plywood machine nailing will be subject to satisfactory job site demonstration and approval by the structural engineer. The approval is subject to continued satisfactory performance. If nail heads penetrate the outer ply more than would be normal for a hand hammer or if minimum allowable edge distances are not maintained, the performance will be deemed unsatisfactory.
 - e. At all unblocked plywood roof sheathing edges, provide one ply clip between each joist unless T & G plywood is used.
 - f. Plywood sheet minimum dimensions shall be 24". Minimum area shall be 8 sq. ft.
 - g. Face nail 2x6 T & G decking with (2) 16d to each support.
- 7. Exposed Beams and Joists:
 - a. All exposed beams and joists shall be Select Structural D.F., Free of Heart Center and Void of Defects.

D. Bolted Joints

- 1. Bolts shall be ASTM A-307, unless noted otherwise on plans.
 - 2. Bolt Holes are to be 1/32" to 1/16" larger than bolt diameter. Locate accurately.
 - 3. Washers are to be provided at each bolt head and nut. Place washer next to wood.
 - 4. Lag or Wood Screws shall be screwed and not driven into place.
 - 5. Tighten bolts up snug and re-tighten at the latest practical time during construction.
- E. Nailed Joints**
- 1. Size and spacing shall be as shown on the drawings and nailing schedule, Sheet S1.3.
 - 2. Sub-bore when nails tend to split wood. Sub-bore for 20d and larger nails. Drill diameter shall be 0.75 x nail diameter.
 - 3. Nails shall be common or ICC approved plywood nails unless noted otherwise. Common nails shall have the following minimum shank diameters:
 - 8d nail - .131" diameter
 - 10d nail - .148" diameter
 - 16d nail - .162" diameter

F. Joist Hangers and Framing Connectors

- 1. By Simpson Strong Tie Co. ICC-ES # 209, #393, #413, #1211
- G. Fasteners
 - 1. Expansion Anchors to be Hilti Kwik-Bolt TZ ICC-ES-ESR #1917.
 - 2. Epoxy shall be two part epoxy. Use Simpson SET-XP Epoxy ICC-ESR #2508.
 - 3. Powder driven "shot pins" to be Hilti NK72 or approved equal. For use on interior, non-bearing sill plates on slab only.

K. Dry Pack: 1 Part Portland cement to 2 1/2 parts sand; or use non-shrink metallic grout (submit proposal to structural engineer for review).

- L. Cover to Bars: Cover to reinforcing bars shall be as follows, unless otherwise shown or noted: Cover to reinforcing bars shall be increased by 50% (or epoxy coated reinforcing used) in corrosive environments or other severe exposure conditions.
 - 1. When concrete is placed against ground3"
 - 2. When concrete is placed against forms, but after form removal will be in contact with ground2"
 - 3. Inside face of walls not exposed to the elements.....3/4"
 - 4. All others1 1/2"

M. Curing: Keep concrete slab on grade wet for 7 days, or cover with approved curing compound in strict accordance with manufacturer's installation recommendations.

N. Vibrate all concrete in place with a mechanical vibrator used by experienced personnel.

O. Slab Finishes: Interior slab finish shall be troweled smooth. Exterior slab finish shall be with a light broom perpendicular to travel. Unless otherwise noted.

P. Slab tolerance to be planar to within 1/8" in 10 feet when checked with a 10' long rod in any direction.

Q. Sill Plate Anchor Bolts: Provide minimum 3/8" diameter x 10' long anchor bolts spaced at 48" on center and within 12" of all sill plate ends or breaks. Provide additional anchor bolts as required by foundation plan and shear wall schedule. Anchor bolts shall be embedded 7" minimum into concrete.

4. Masonry

- A. General: All masonry work shall be done in accordance with the Masonry Standards Joint Committee (MSJC) Building Code requirements for masonry structures (TMS 402-13 / ACI 530-13 / ASCE 5-13), and the CBC 2019 edition, chapter 21.
- B. f'm = 1500 psi, (TMS 602 S-16 Table 2) Periodic Special Inspection is required.
- C. Concrete masonry units shall be of a quality at least equal to the requirements as set forth in ASTM C-90, medium weight units. Use open ended units with fully mortared joints. F'c = 1900 psi
- D. Mortar shall conform to ASTM C-270 type M or S with a compressive strength of 1250 psi at 7 days and 2500 psi at 28 days. Use mix ratios of 3 parts sand and 1 part type 2 Portland cement. Do not use masonry cement.
- E. Grout (fine grout) shall conform to ASTM C-476 with a compressive strength of 1250 psi at 7 days and 2000 psi at 28 days. Slump limits shall be a minimum of 8" and a maximum of 11". All masonry cells are to be solid grouted unless otherwise noted. Provide "Grout Aid" admixture in all grout. Follow manufacturer's recommendations for quantities and procedures.
- F. Reinforcing shall conform to ASTM A-615 Grade 40 or 60 for #4 and smaller and ASTM A-615 Grade 60 for #5 and larger. Epoxy coated reinforcing shall conform to ASTM A-775. See Sheet S1.3 for all required laps, bends & splices.
- G. All Masonry shall be laid true, level, plumb, and neatly in accordance with the plans. Use running bond, unless otherwise noted.
- H. No chipped, cracked, soiled, or otherwise imperfect block shall be used in the work.
- I. Consolidate all grout using an electric mechanical vibrator of suitable size for masonry work. Vibrate 3 minutes after grout is deposited.
- J. Maximum grout lifts shall be 5'-4" maximum. Cleanouts are not required.

5. Structural Steel and Miscellaneous Metals

- A. General: All structural steel work shall be done in accordance with the American Institute of Steel Construction Steel Construction Manual (AISC 360), Seismic Design Manual (AISC 341), and the CBC 2019 edition, chapter 22.
- B. Materials: All material specifications are to be retained by contractor and to be available to structural engineer at his request.
 - 1. W Shapes: ASTM A-992 (50ksi)
 - 2. Pipe Columns: ASTM A-53 Grade B (35ksi)
 - 3. Hollow Structural Sections (HSS): ASTM A500 Grade B (46ksi)
 - 4. Channels, Angles: ASTM A-36 (36ksi)
 - 5. High Strength Bolts: ASTM A-325 (120ksi)
 - 6. Anchor Bolts: ASTM A-307 or A-36
 - 7. Welding Rod: Heavily coated, conforming to American Welding Society specifications for Arc welding electrodes of E70xxx classification. Use low hydrogen electrodes for welding reinforcing bars.
- C. Galvanizing: Galvanize all miscellaneous iron angles, clips and other elements exposed to weather or in processing areas.
- D. Shop Drawings: Submit a minimum of 2 sets of shop drawings to structural engineer for review (more sets shall be submitted if required by architect or general contractor).
- E. Shop Primer: Standard brand of rust inhibitive primer conforming to Federal Specification TT-P-86E, Type III. Galvanized metal primer shall conform to Federal Specification TT-P-64LD, Type II. Do not paint areas to be welded or embedded into masonry or concrete.
- F. Erection: All field welding shall be in accordance with American Welding Society Structural Welding Code, AWS D1.1, and all applicable revisions. All welds shall be made only by welders, tackers, and welding operators who have been previously qualified by test as prescribed by the AWS Structural Welding Code, AWS D1.1, and applicable revisions. Weld electrodes shall be as specified by the American Institute of Steel Construction, "Specification for the Design, Fabrication, and Erection of Steel for Buildings". Erection shall be in accordance with the American Institute of Steel Construction "Code of Standard Practices". Adequate temporary bracing and bracing shall be installed during erection where needed to secure the framing against wind & seismic forces, erection equipment, and erection operations. Do not use gas cutting torches for correcting fabrication errors in the structural framing. Report any errors to the structural engineer immediately. All bolts to be tightened to "snug tight" conditions, unless noted otherwise.
- G. Special Inspection is required for all field welding and high strength bolt installation.
- H. Fabrication: Fabricate structural steel in accordance with AISC specifications and as indicated on the shop drawings as reviewed by the architect, structural engineer, and general contractor. Fabrication shop is to be licensed and approved by the building official. If fabrication shop is not licensed and approved, all welding performed in the shop is to be Special Inspected by a licensed Special Inspector.

I. General

- A. The contractor shall inform the Structural Engineer 48 hours in advance of reaching the following stages of construction:
 - 1. Footing excavations completed.
 - 2. Footing reinforcing bars in place.
 - 3. Concrete placing operations.
 - 4. Wood framing completed but not covered in.
 - 5. Ply nailing completed but not closed.
 - 6. All structural work completed.
 - B. Details shown on the structural drawings are typical, similar details apply to similar conditions. Contractor shall verify existing conditions. Any existing conditions requiring construction different from that shown shall be reported to the architect immediately.
 - C. All drawings shall be read in conjunction with the specifications, architectural, mechanical, electrical, and all other contract drawings as applicable.
 - D. Dimensions shown shall take precedence over scale on plans, sections, and details. Notes and details on the drawings shall take precedence over general notes and typical details. Discrepancies shall be brought to the attention of the architect immediately.
 - E. The contract structural drawings and specifications represent the finished structure. Unless otherwise noted, they do not indicate the method of construction. The contractor shall provide all measures necessary to protect the structure, workmen, or other persons during construction. OSHA regulations are to be strictly adhered to in providing the protective measures as stated above to include, but not limited to, all shoring, bracing, and underpinning.
 - F. Unless otherwise noted on the structural drawings, use typical details on sheet: **S1.2 & S1.3** as applicable.
 - G. Dimensions are to be checked and verified by the contractor. Discrepancies are to be reported to the architect immediately. Discrepancies are to be resolved before proceeding with work.
 - H. All Special Inspections are to be performed by a licensed Special Inspector approved by the building official and the structural engineer. The Special Inspector is an Agent of the Owner and is paid by the Owner, per CBC 2019, chapter 17 "Structural Tests and Special Inspections"
- 2. Foundation Design**
- A. General: All foundation work shall be done in accordance with the CBC 2019 edition, chapter 18.
 - B. The Soils Report for this project was prepared by: **Pacific Materials Laboratory** File Number: **16-1153-2** Dated: **12/22/16**, Updated **10/25/17**
 - C. Foundation design is based on an allowable soil bearing pressure of: **N/A** psf (**Caissons**)
 - D. Provide 90% minimum soil compaction under all slabs and structural foundation work, unless noted otherwise in the soils report.
 - E. Earthwork shall be performed in accordance with the soils report and certified by a Special Inspector.
 - F. Prior to the contractor requesting a building department foundation inspection, the soils engineer (contractor, if soils report is not available) shall advise the building official in writing that:
 - 1) The building pad was prepared in accordance with the approved plans and the soils report (if available).
 - 2) The utility trenches have been properly backfilled and compacted.
 - 3) The foundation excavations, forming, and reinforcements comply with the approved plans and the soils report (if available).
 - G. Foundation excavations shall not disturb adjacent existing structures.

3. Concrete

- A. General: All concrete work shall be done in accordance with the ACI Building Code (ACI 318-14), the ACI manuals of concrete practice, and the CBC 2019 edition, chapter 19.
- B. Mix Design: Concrete shall have the following 28 day compressive strengths (unless specifically noted otherwise on plans):
 - Typical:
 - Slabs on Grade, Footings.....3000 psi #
 - Grade Beams, Caissons, Structural Slabs, Basement Walls.....3000 psi #
 - Retaining Walls.....3000 psi #
 *Indicates Special Inspection is required.
 - Residential: (Light Frame Construction, Two Stories or Less)
 - Slabs on Grade, Footings.....2500 psi
 - (Special Inspection is Not Required)
- Mix Designs shall be prepared by the concrete supplier or testing agency and reviewed by the structural engineer a minimum of 2 days prior to concrete pour.
- C. Cement: Shall Conform to ASTM C-150, Type 2, Low Alkali. Minimum 5 1/2 sacks per yard.
- D. Aggregate: Fine aggregate to conform to ASTM C-33. Coarse aggregate to conform to ASTM C-33. Maximum size to be 1" (maximum of 1 1/2" will be acceptable for footings only). Aggregate gradation shall conform to ASTM C-33. Pea Gravel shall not be used.
- E. Reinforcing Materials: #4 and smaller shall conform to ASTM A-706 or A-615 Grade 40 or 60. #5 and larger shall conform to ASTM A-706 or A-615 Grade 60. Steel welded wire reinforcement shall conform to ASTM A-1064. Epoxy coated reinforcing shall conform to ASTM A-775 or A-884. See Sheet S1.3 for all required laps, bends & splices. #6 bars and larger are to be shop fabricated. Make all bends cold.
- F. Concrete consistency: Slump limits shall be a minimum of 2 1/2" and a maximum of 4". Slabs on grade may be placed with a maximum slump of 5".
- G. Mix Designs shall be submitted as specified by the architect. If not specified, submit a minimum of two copies to the structural engineer a min. of 2 days prior to pour.
- H. Admixtures: Obtain structural engineers approval for all admixtures not noted: Flyash shall conform to ASTM C-618, type F; air entraining admixtures shall conform to ASTM C-260; water reducing admixtures shall conform to ASTM C-494 or ASTM C-1017.
- I. Check with all trades to insure proper placement of all inserts, sleeves, openings, conduits, etc. prior to pouring concrete. All penetrations through grade beams and all penetrations larger than 6" in diameter are to be approved by the structural engineer.
- J. All sleeves not specifically shown on the drawings shall be located by the trades involved and shall be reviewed by the structural engineer.

Building Code: CBC 2019 Edition (Based on IBC 2018 Edition)

Structural Observation Program (CBC 2019; 1704.6)

- A. The owner shall employ the Engineer of Record licensed in the State of California who is responsible for the structural design, to do structural observation.
- B. Engineer of Record: Kevin L. Vandervort, Registration/License Number: S 3734
- C. Designated Engineer to do Structural Observation: Kevin L. Vandervort, Registration/License Number: S 3734. EOR shall be contacted at least 48 hours in advance to schedule Structural Observations.
- D. The Engineer responsible for the Structural Observation, the Contractor, and appropriate Subcontractors shall hold a pre-construction meeting to review the details of the structural system to be structurally observed.
- E. Foundation: Structural elements to be observed prior to placement of concrete in the foundation. Footing excavations width and depth, reinforcing placement, holdown anchors and anchor bolts at shear walls, and utility line penetrations through structural elements.
- F. Floor Framing: Structural elements to be observed at each floor level. Shear wall construction, shear transfer, floor sheathing nailing.
- G. Roof Framing: Structural elements to be observed prior to roof covering. Shear wall construction, shear transfer, roof sheathing nailing, prefabricated roof truss installation (where applicable).
- H. FINAL OBSERVATION: Structural elements to be observed at the final observation visit. All drag straps and holdown straps installed, utility rough-ins, and all previous corrections completed.

Project Design Gravity Loads

Roof Loads
 Dead Loads: **32** psf (**Mission Tile**)
 Live Loads: **20** psf (**4:12 Slope**)

Floor Loads
 Dead Loads: **N/A** psf (**Structural Slab**)
 Live Loads: **40** psf (**Residential**)

Partition Loads N/A psf

Project Design Lateral Loads

Risk Category II
Wind Design Data
 (Directional Procedure, Simplified Method) (ASCE 7-16 27.4)
 Basic Wind Speed (Vw): **110** mph
 Exposure: **B**
 Height Above Ground Level (z): **15'**
 Maximum Horizontal Design Pressure: **21.89** psf

Seismic Design Data
 Equivalent Lateral Force Procedure (ASCE 7, 12.8)
 Site Latitude: **34.416**
 Site Longitude: **-117.752**
 Site Class: **D** (Plywood Shear Walls)
 R: **6.5**
 Ie: **1.0**
 Ss: **2.332**
 S1: **0.823**
 Sp: **1.866**
 Cs: **0.287**
 Seismic Design Category: **E**
 p: **1.3**
 V=pcSaIe= **0.373** Wd.

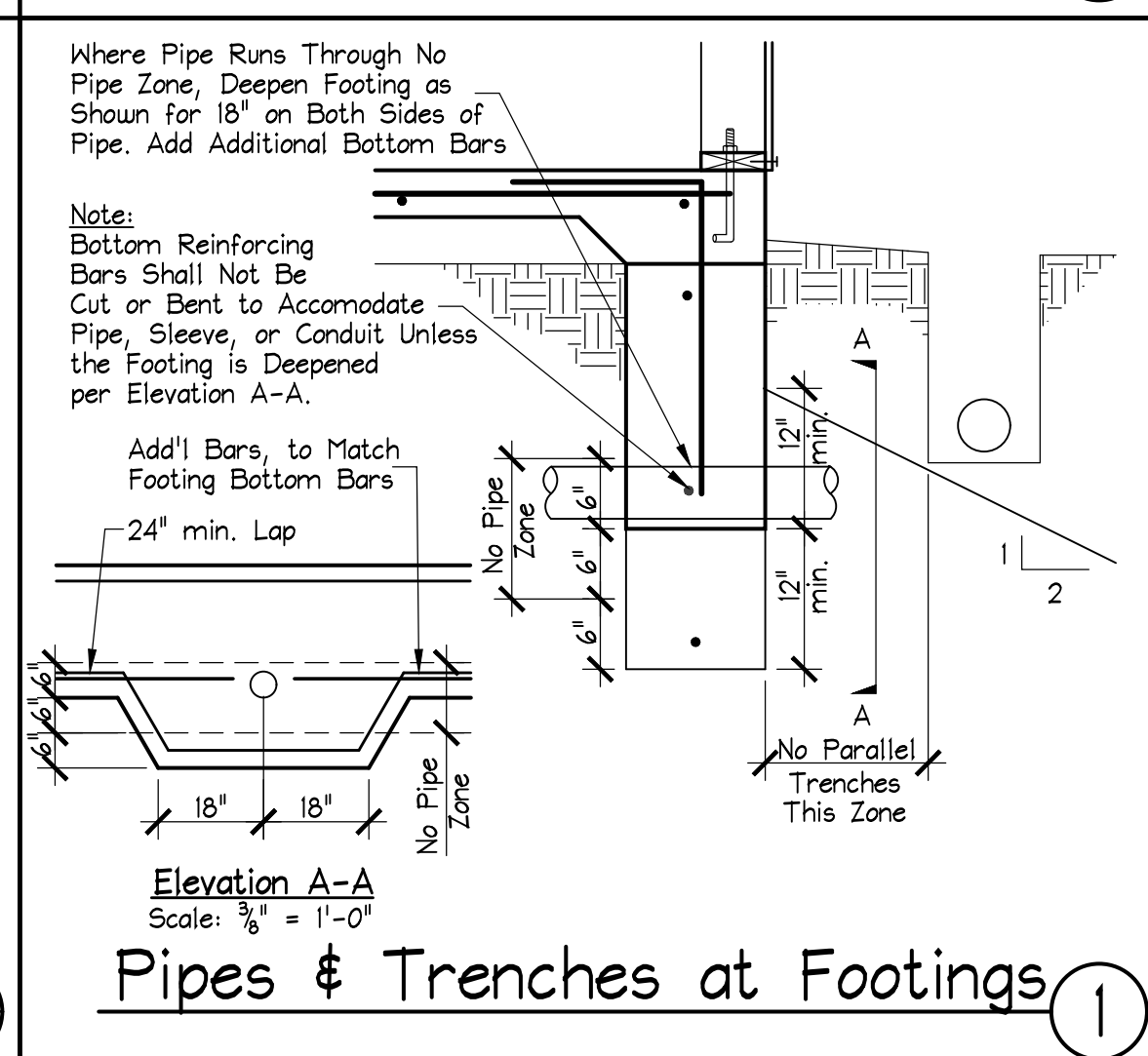
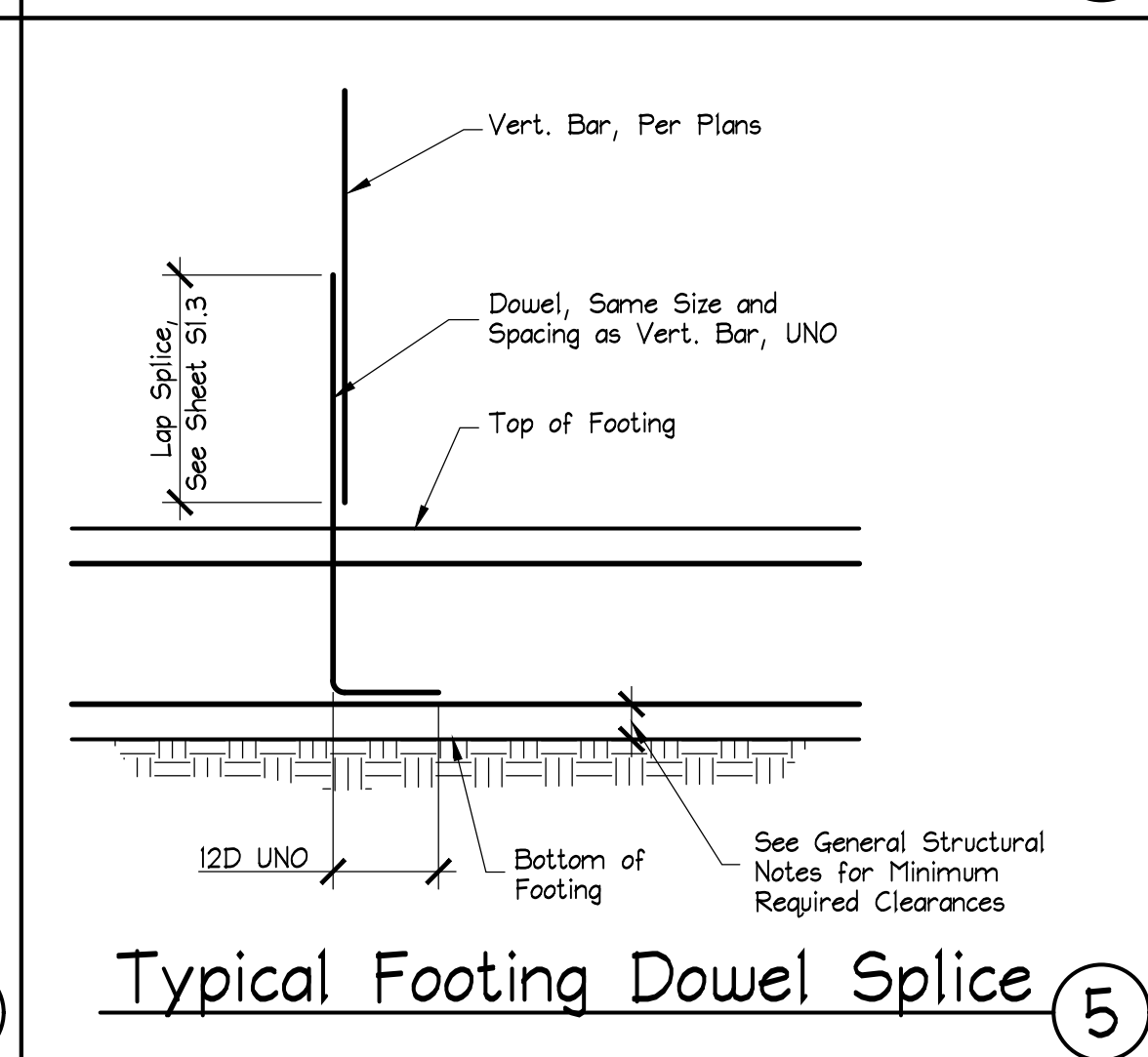
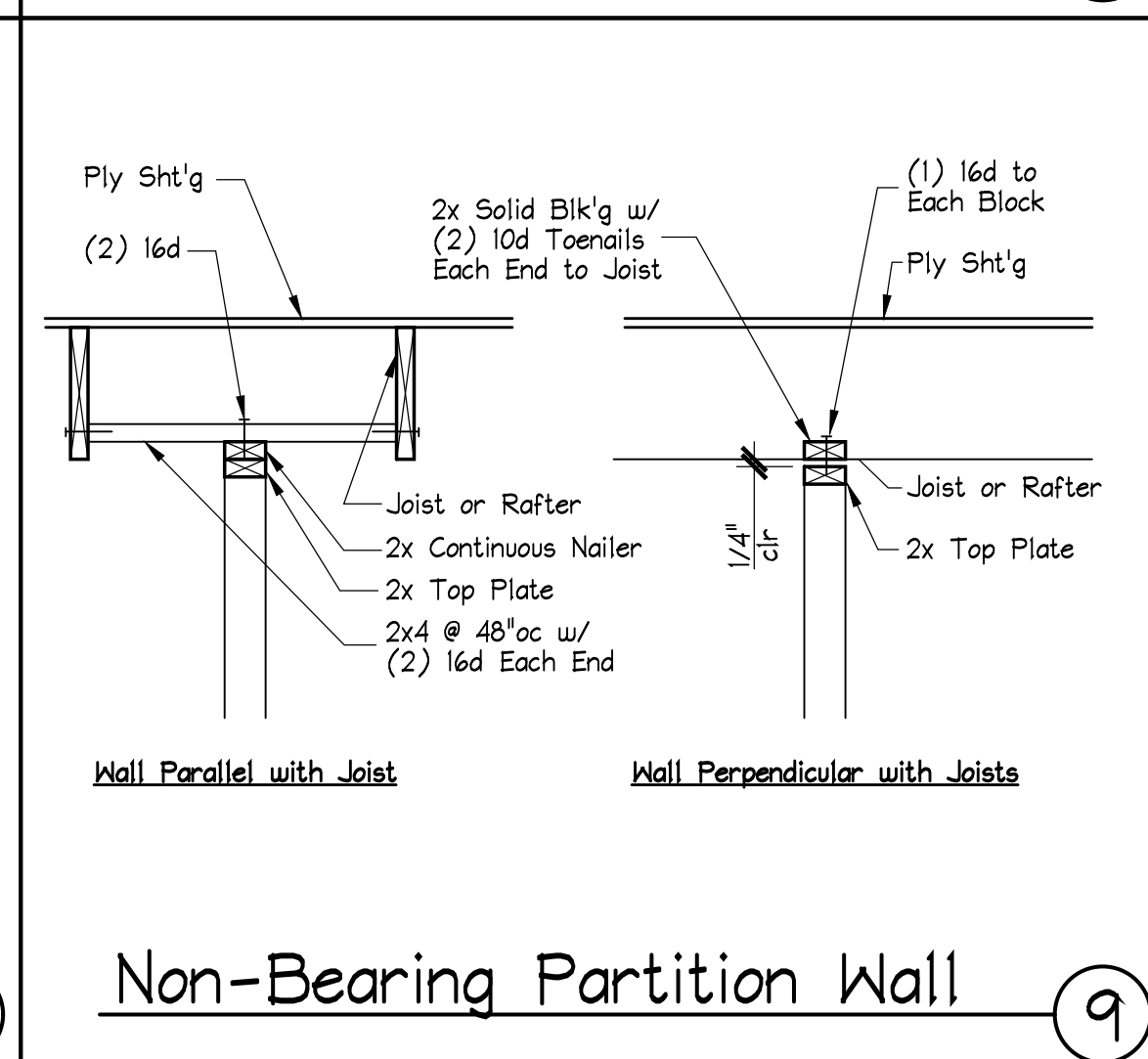
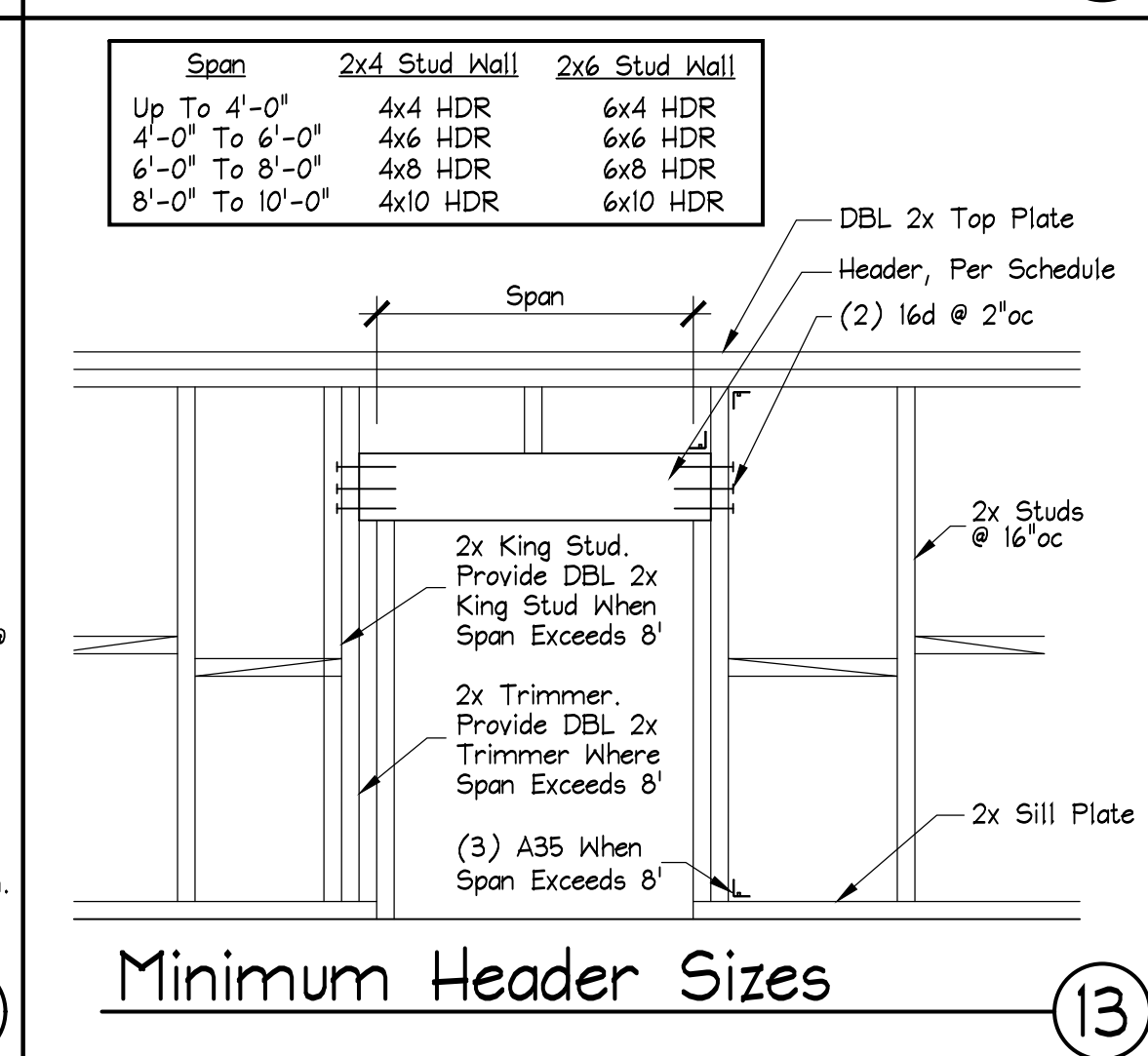
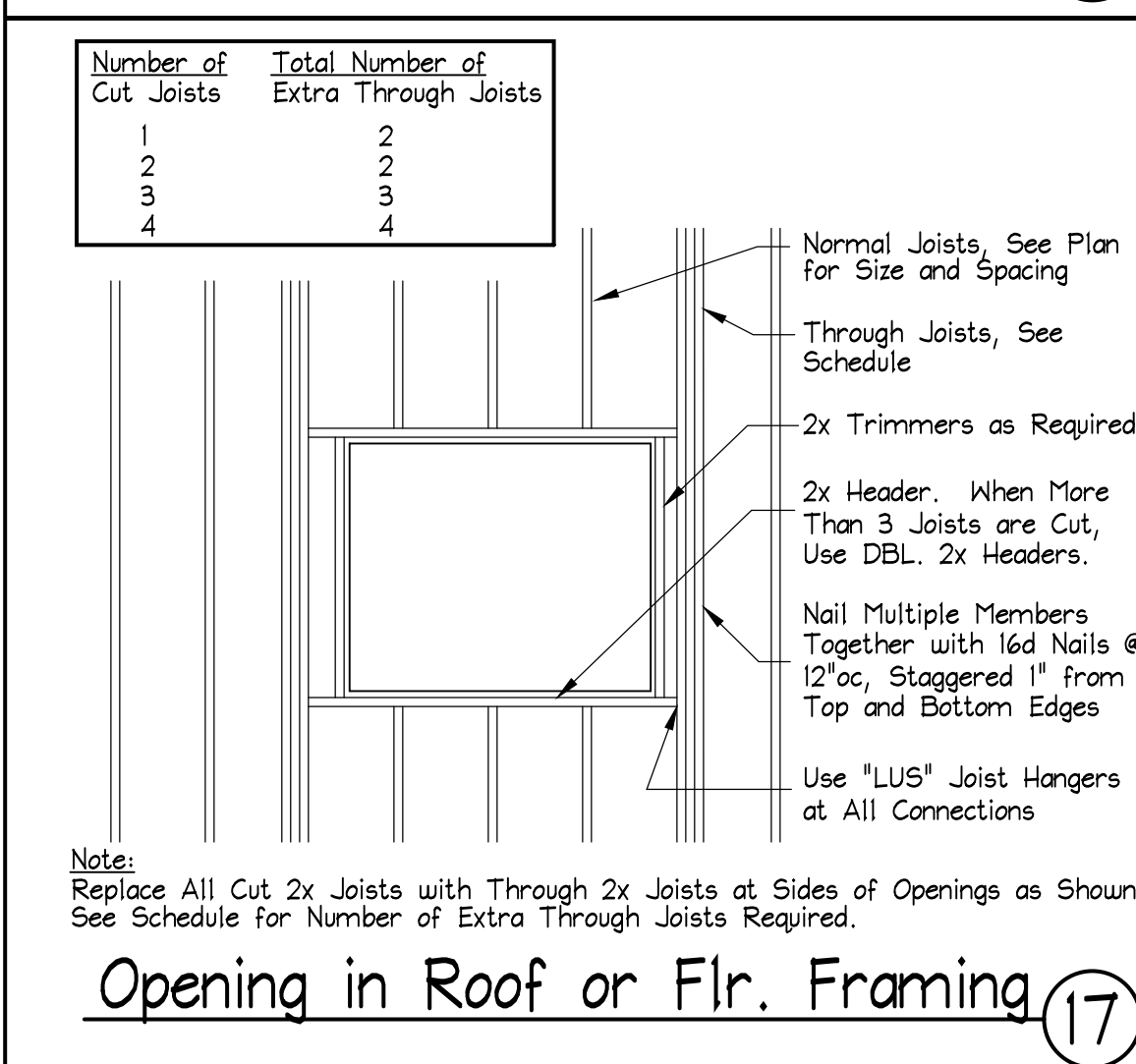
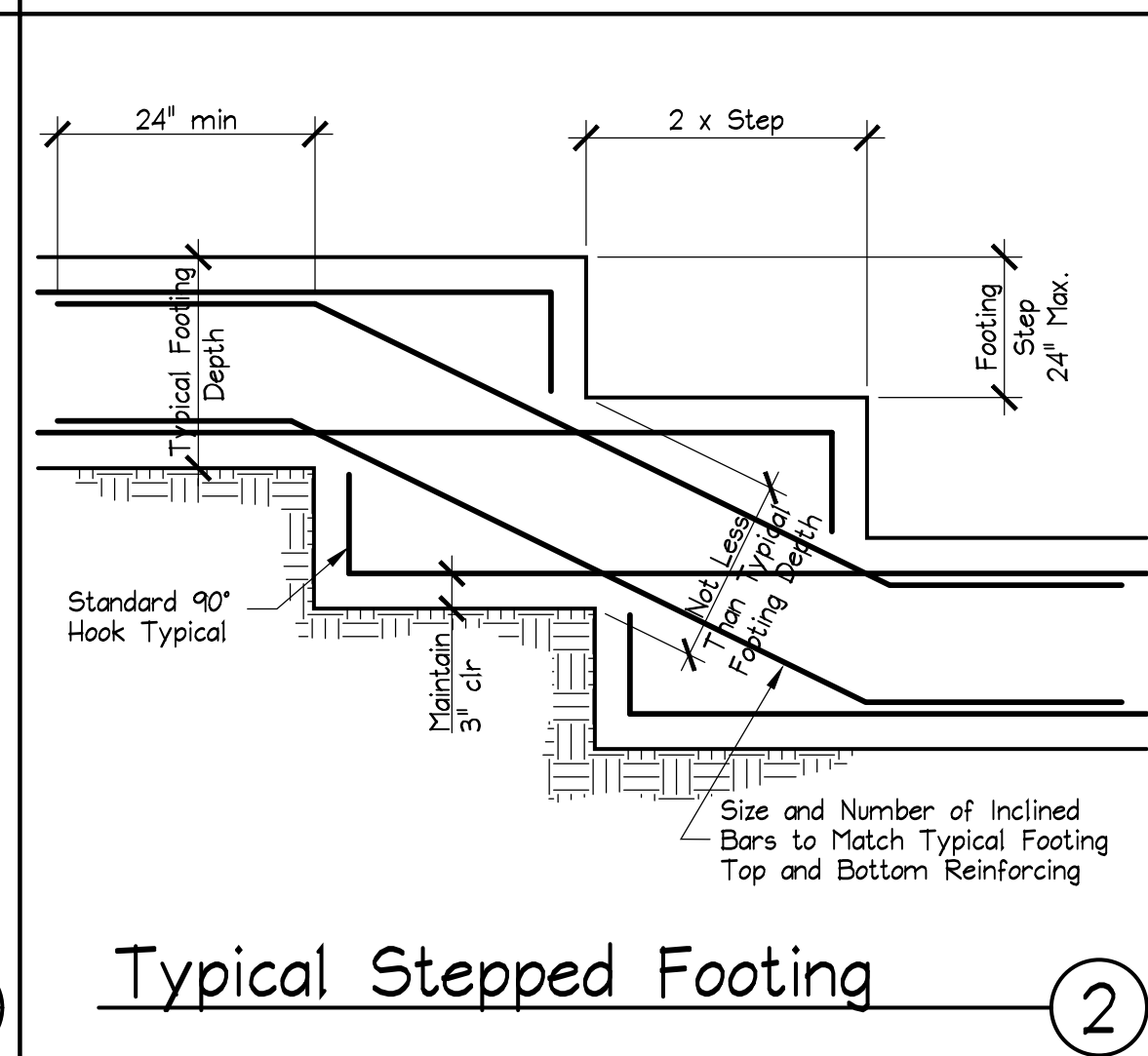
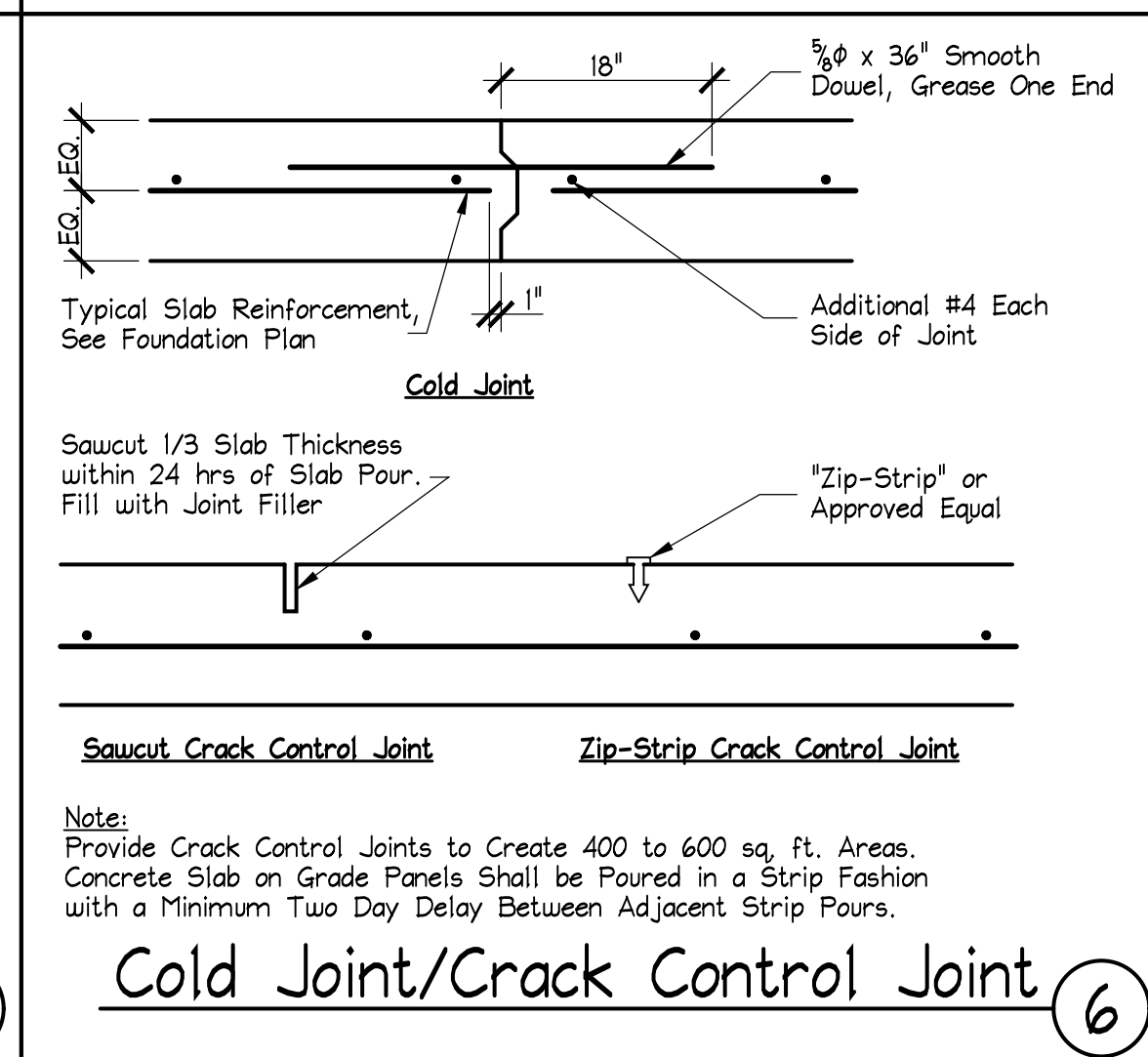
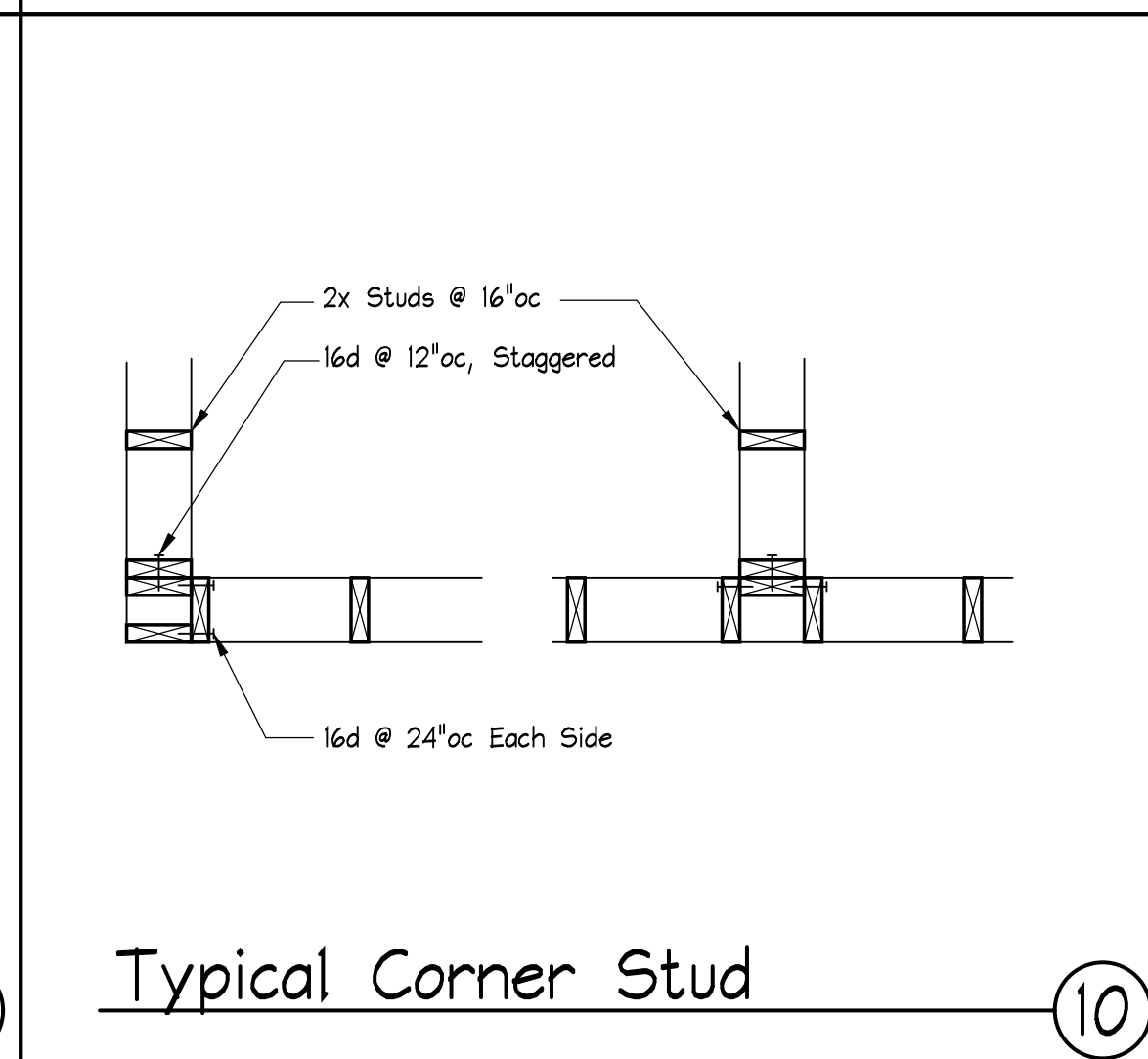
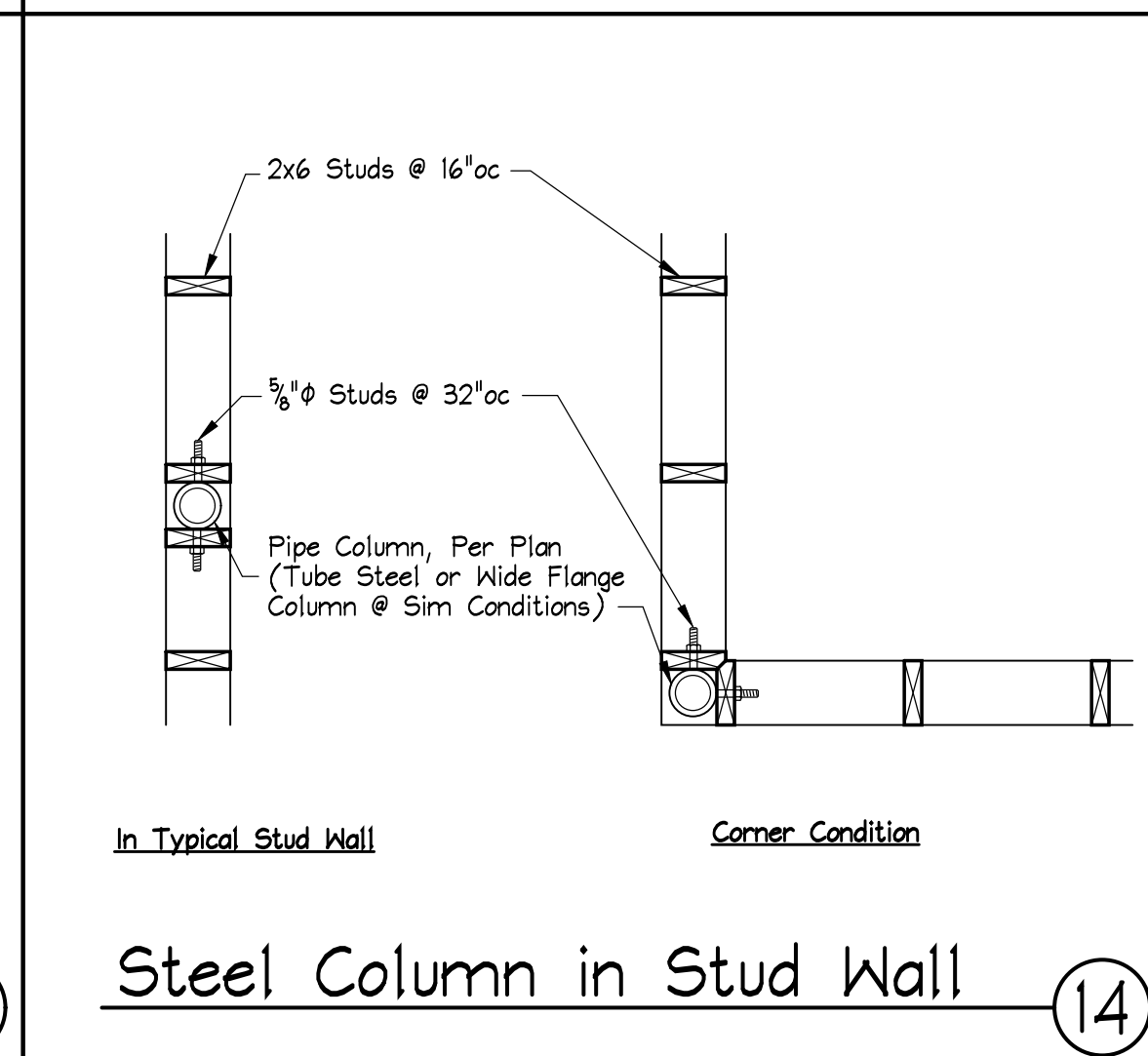
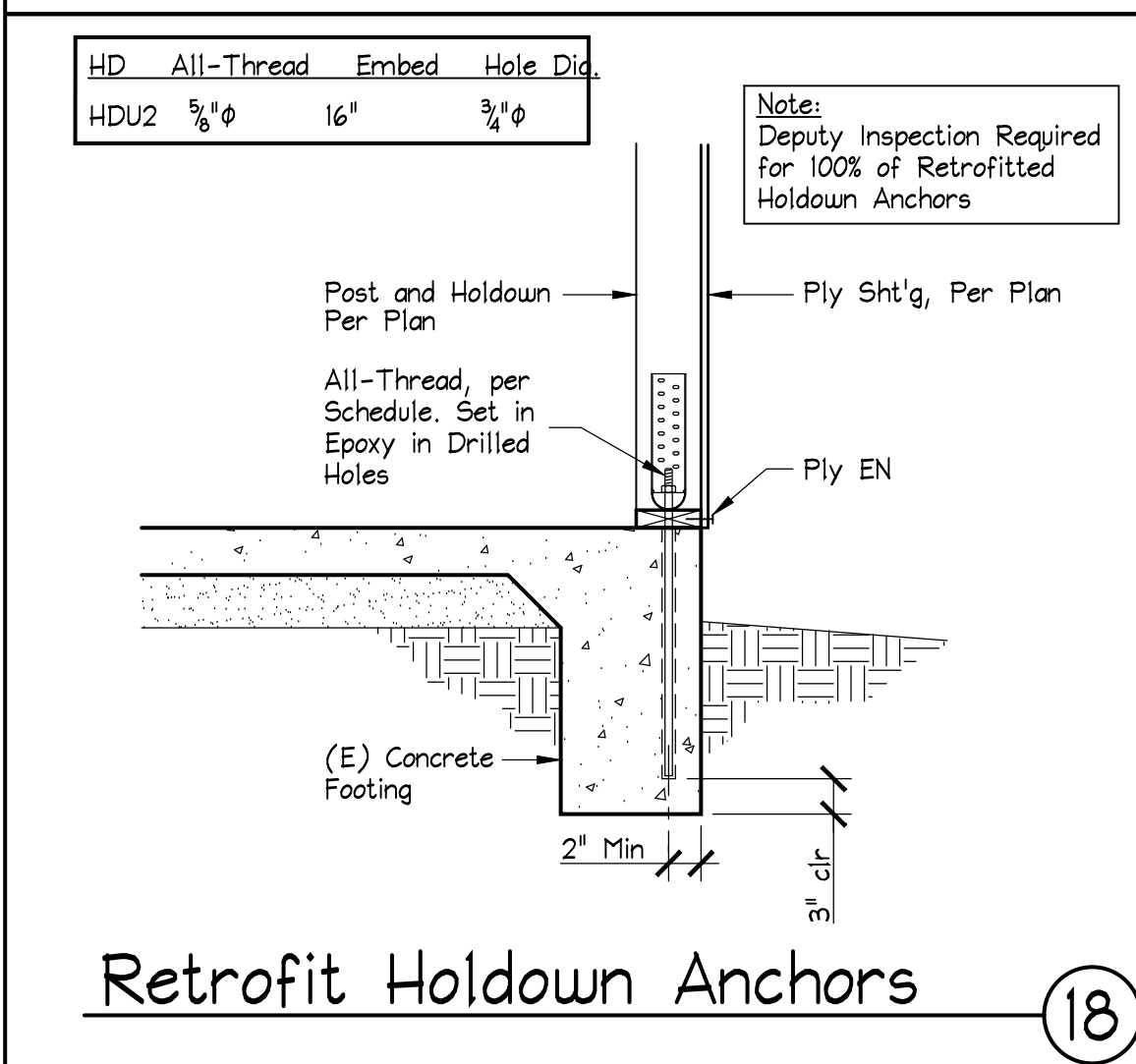
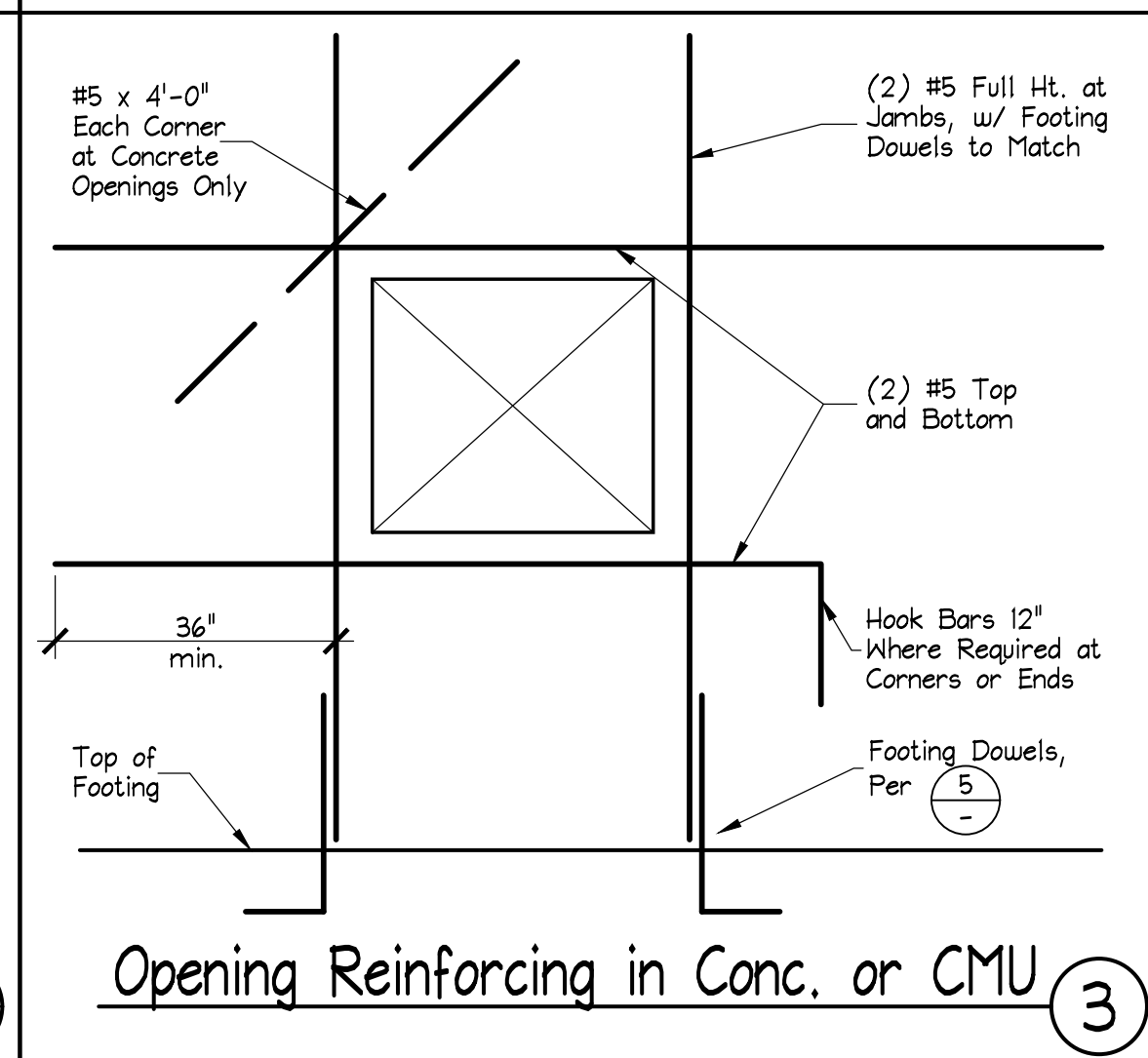
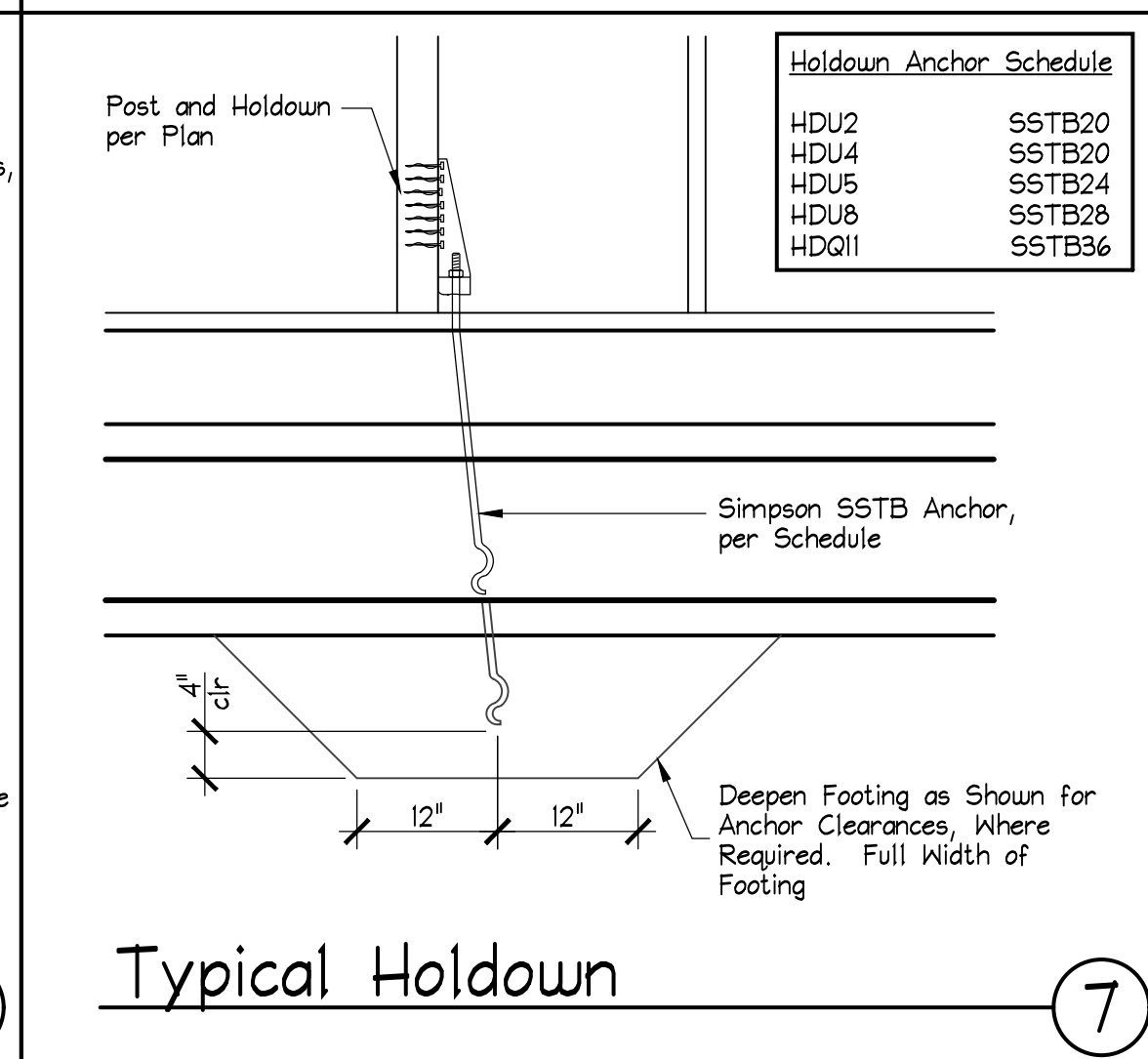
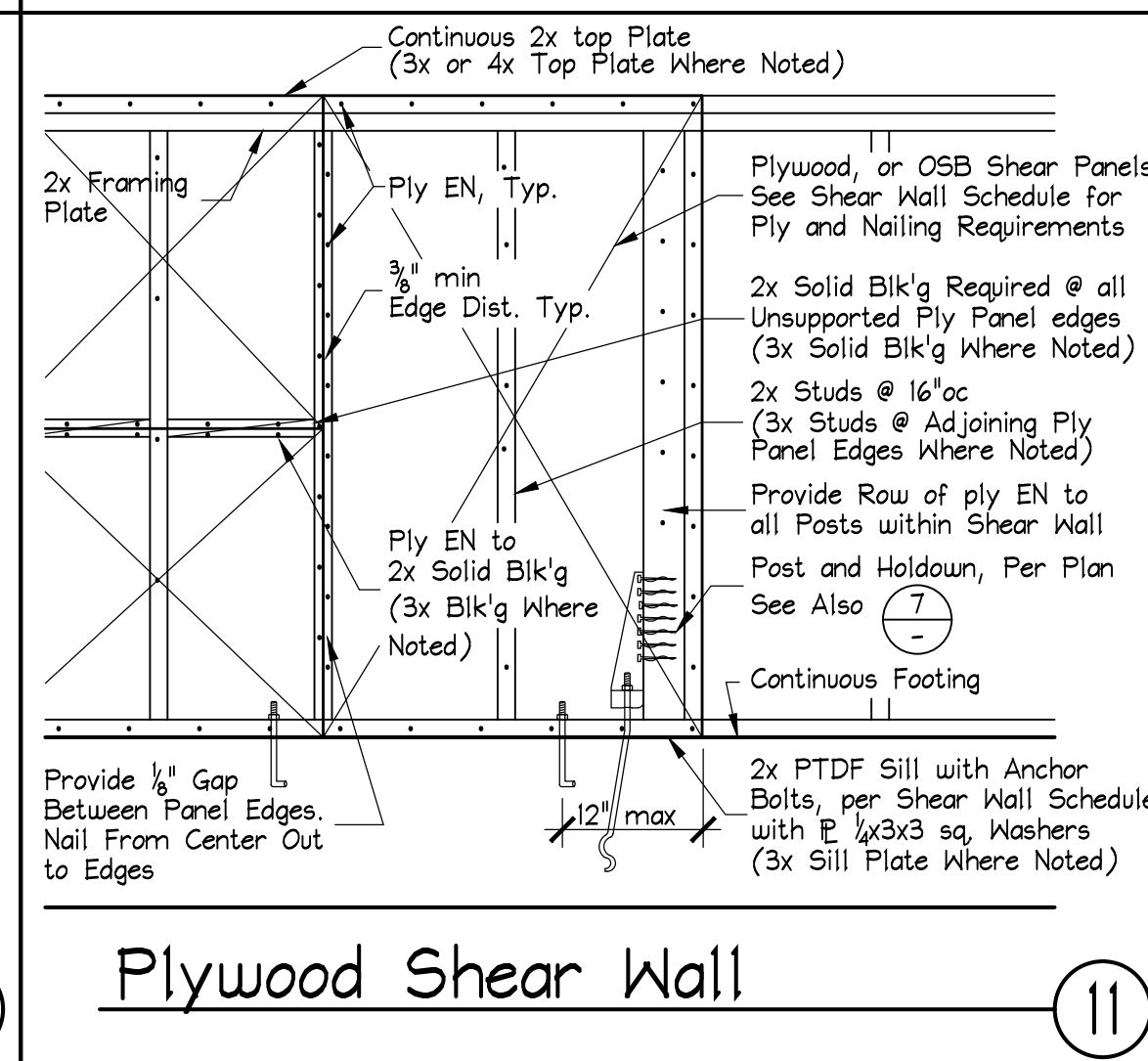
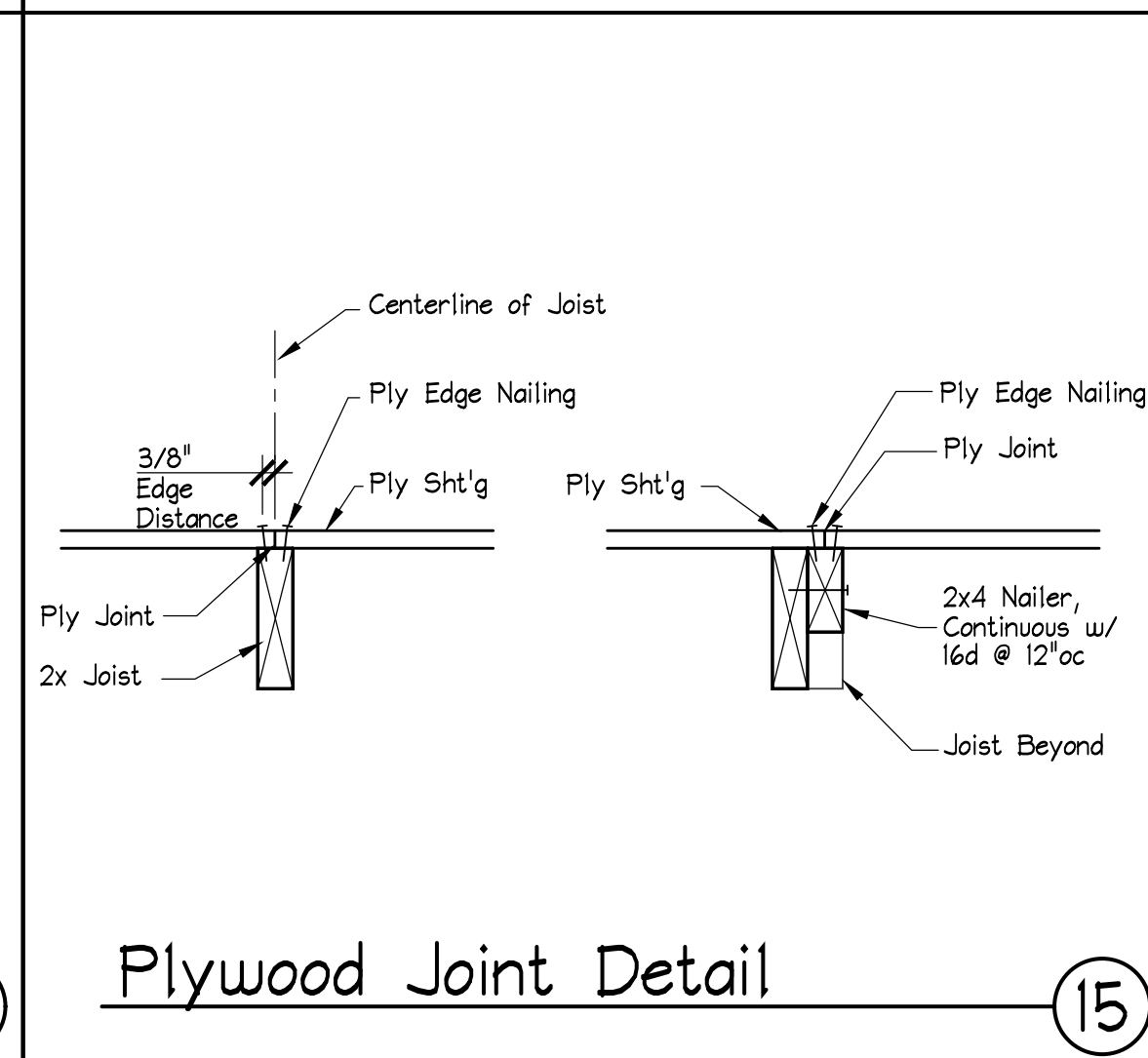
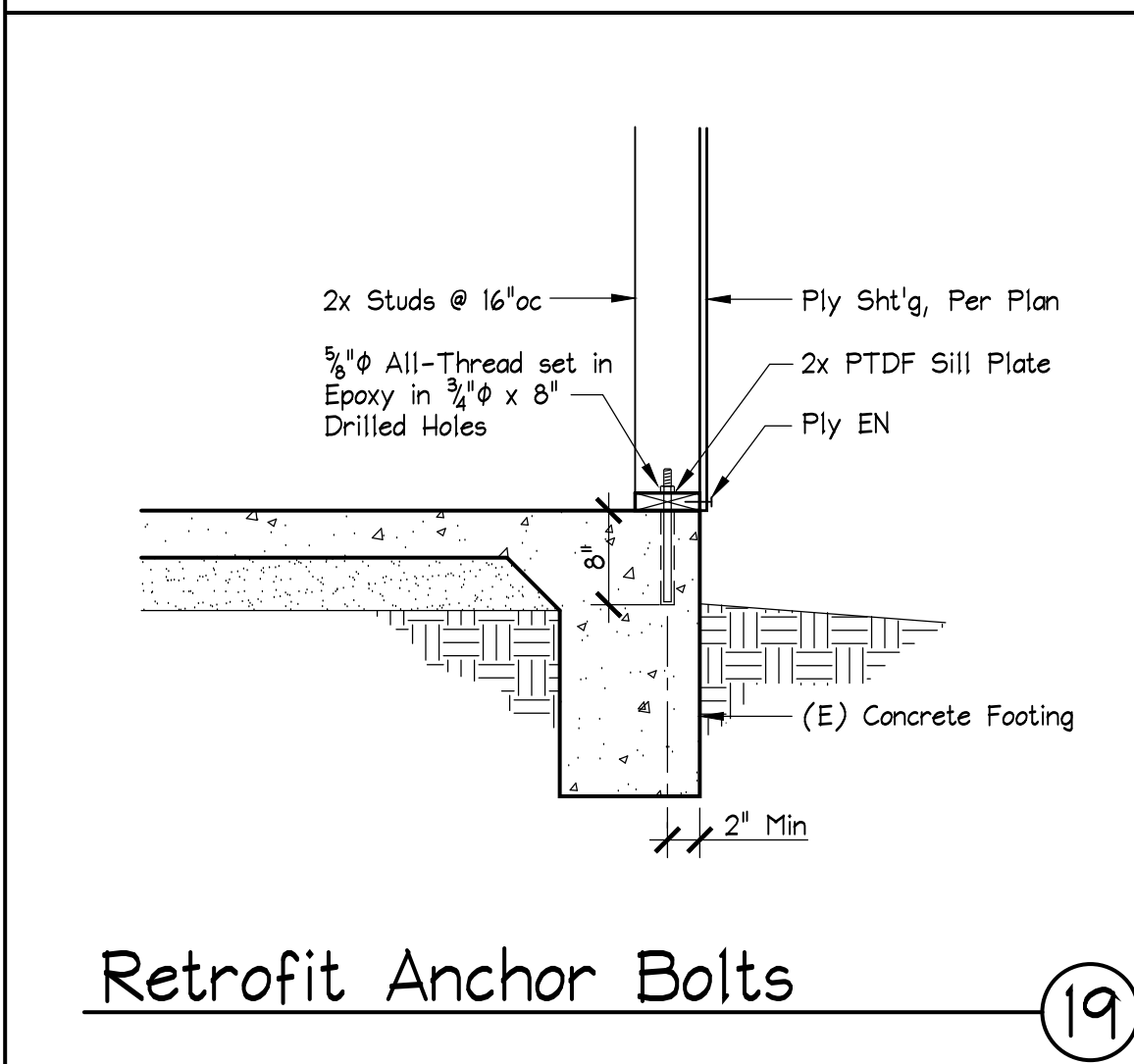
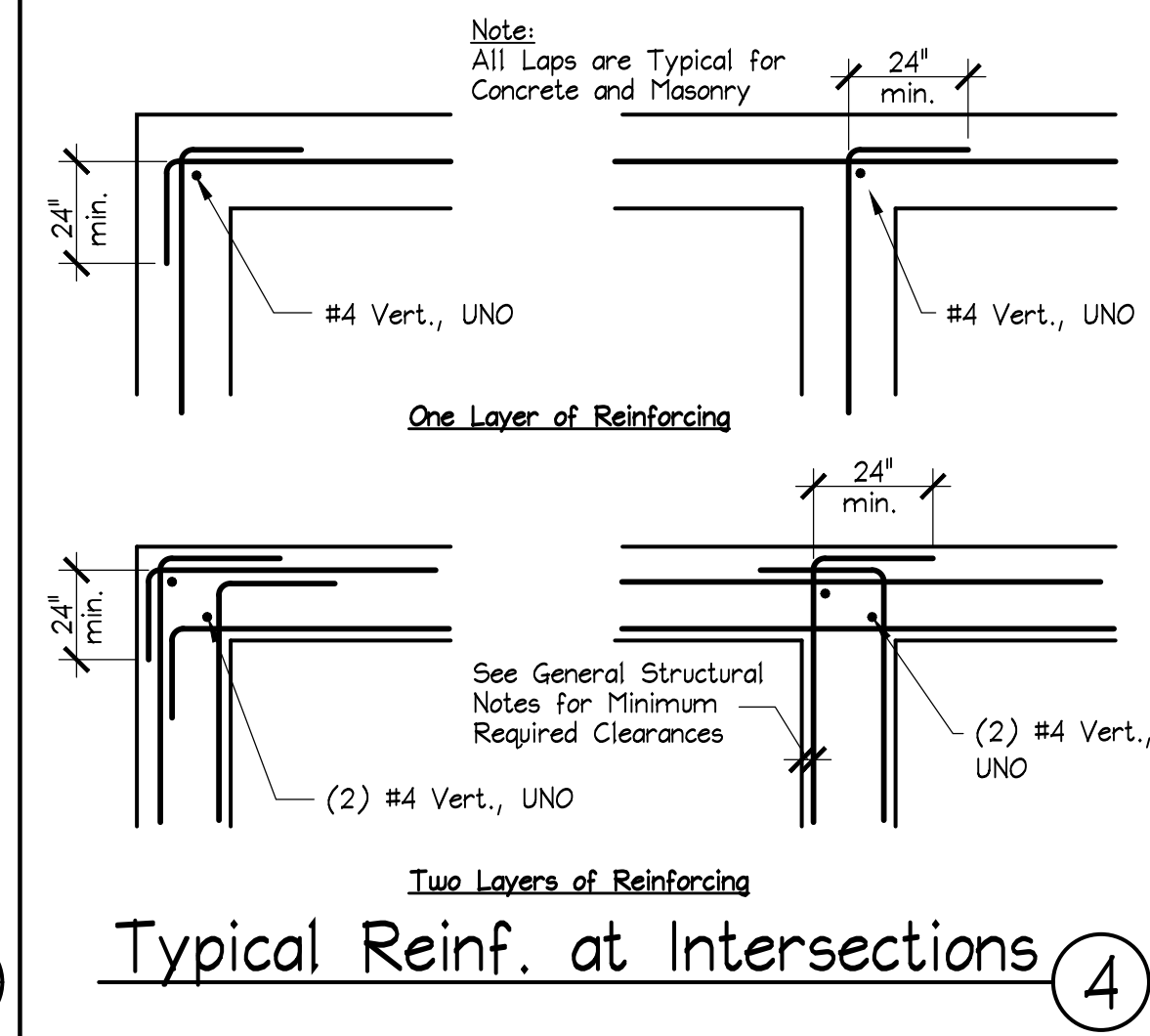
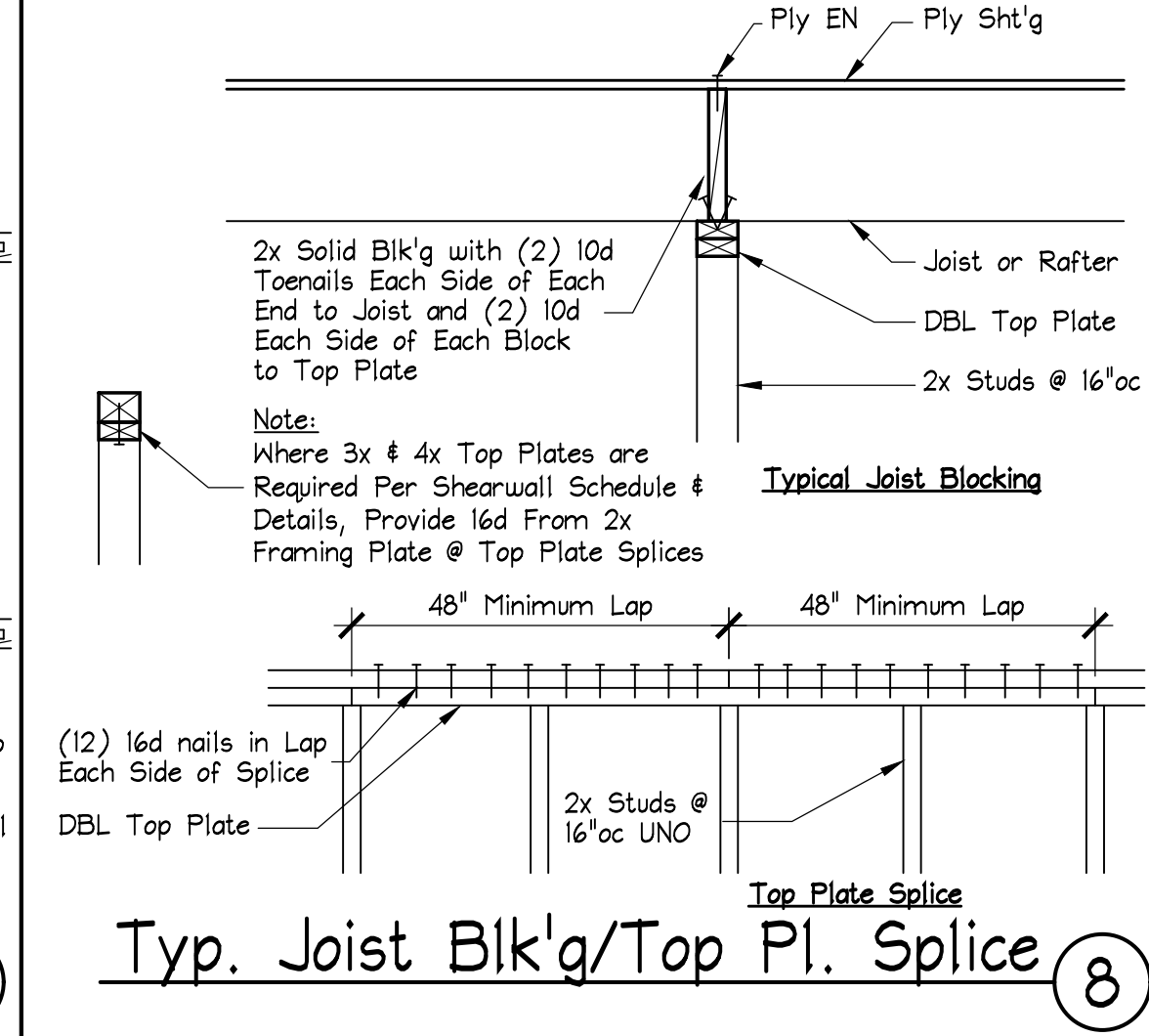
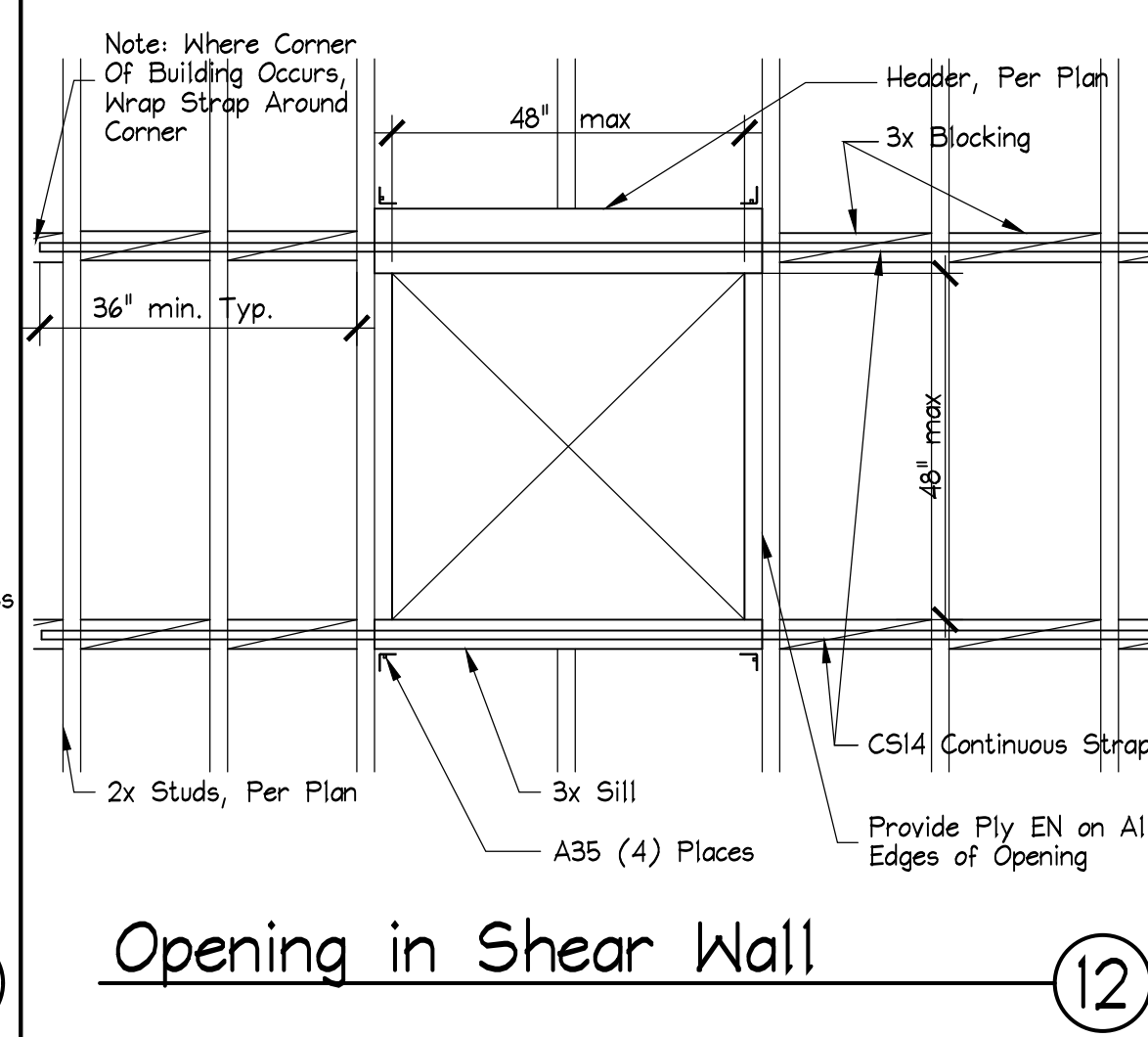
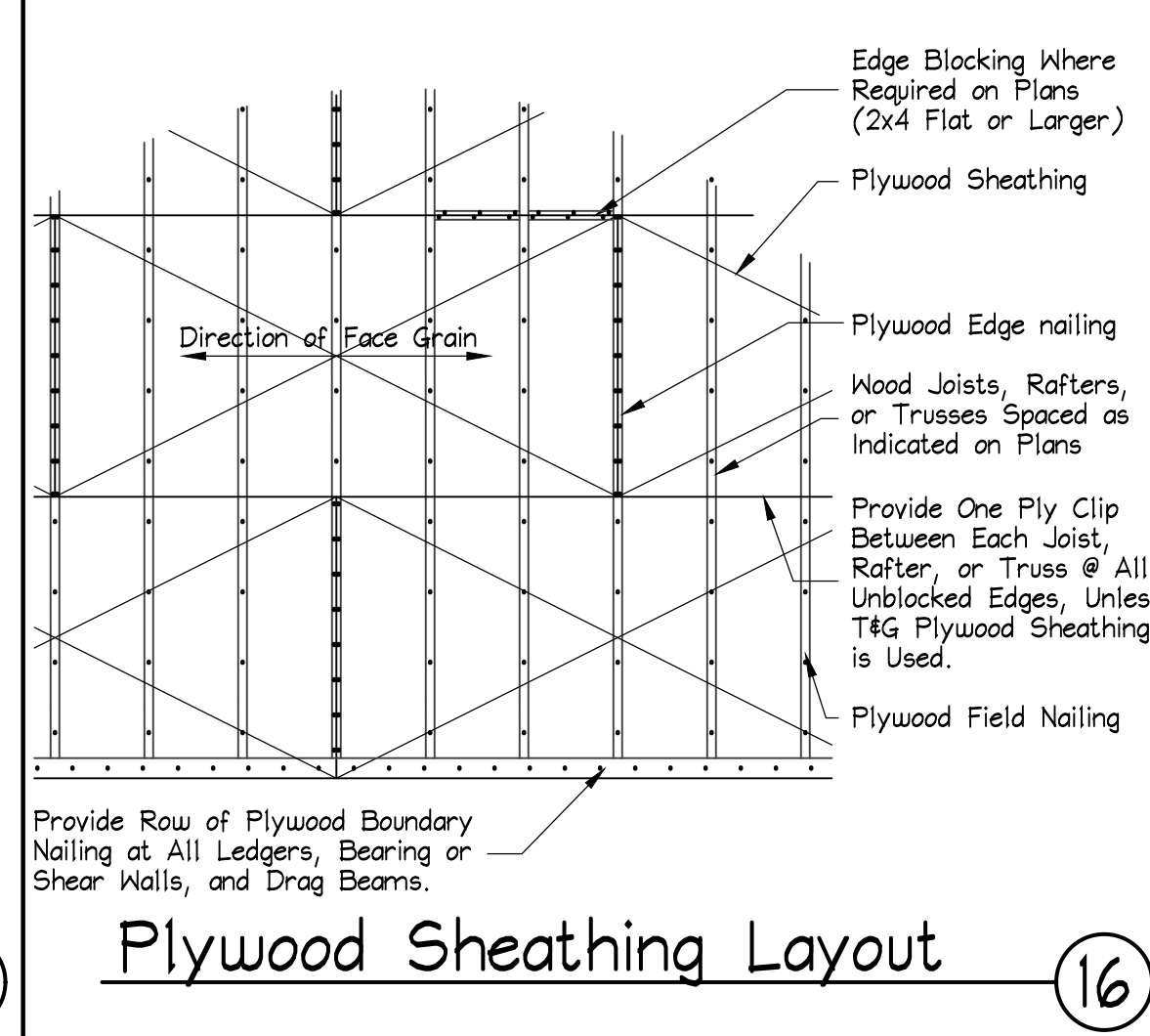
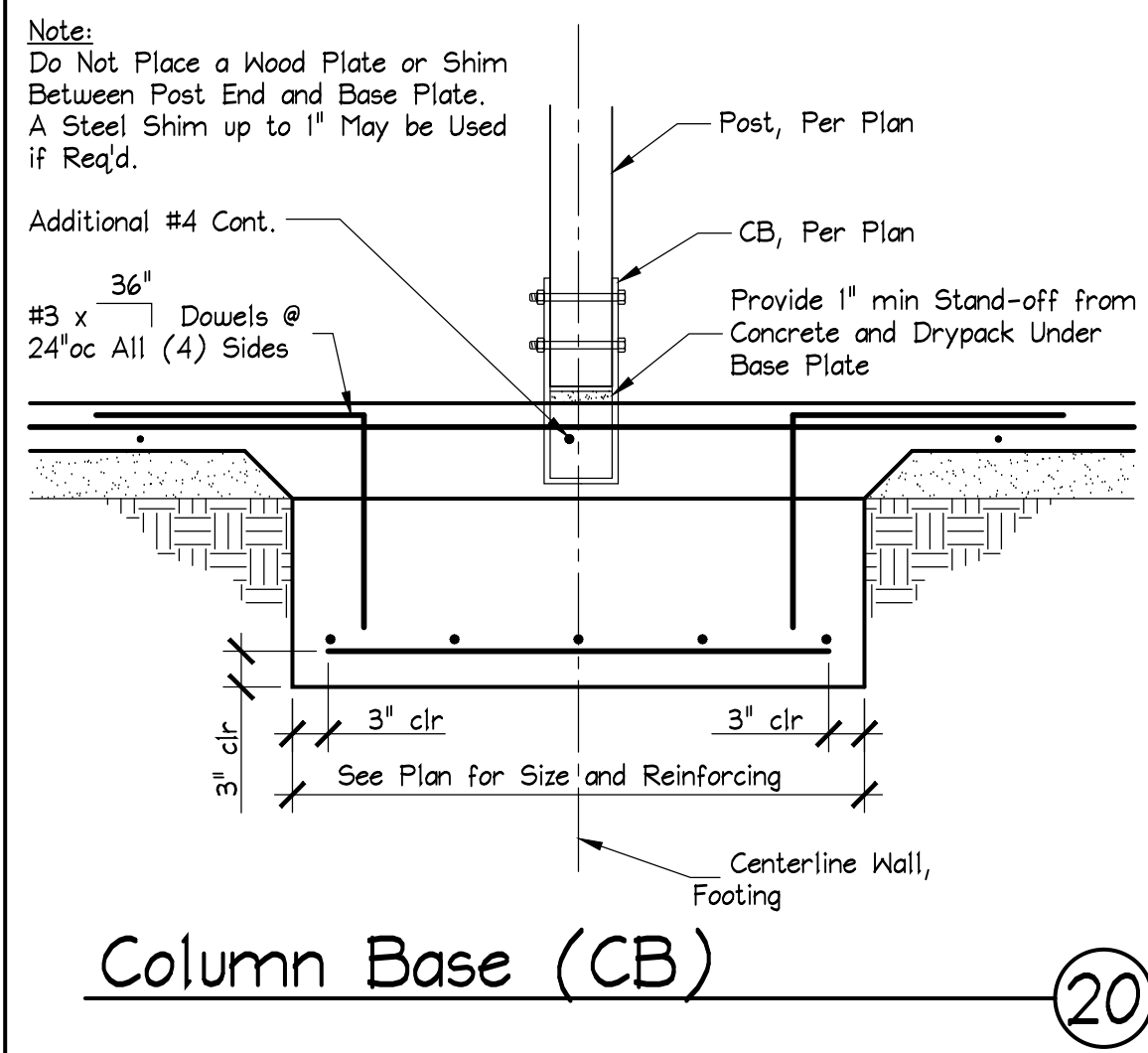
Tests and Inspections (X)

The following items require Special Inspection per CBC 2019 edition, chapter 17. See plans and details for specific locations.

STATEMENT OF SPECIAL INSPECTIONS

WORK REQUIRING SPECIAL INSPECTION AND STRUCTURAL OBSERVATION DURING: (TO BE SELECTED & CHECKED BY THE DESIGN PROFESSIONAL OF RECORD PER CBC 2019)	SPECIAL INSPECT.		STRUCT. OBSERV.	SOILS ENGR
	CONT.	PERIODIC		
GRADING & FOUNDATION				
PLACEMENT OF COMPACTED FILL, GRADING, AND EXCAVATIONS				X
HELICAL PIER INSTALLATION				
CAISSON EXCAVATION	X			X
OTHER				
CONCRETE			X	
MIX DESIGN			X	
REINFORCING PLACEMENT	X		X	
COMPRESSION TESTING				
PNEUMATICALLY PLACED CONCRETE				
STRUCTURAL - CONCRETE f _c ≥ 3,000 psi	X		X	
FOUNDATION / SLAB ON GRADE. CONC. f _c = 2,500 psi				
NON-SHRINK GROUT				
BOLTS INSTALLED IN CONCRETE				
OTHER				
DRILLED ANCHORS				
INSTALLATION OF ADHESIVE ANCHORS, RODS & DOWELS				
EXPANSION ANCHORS				
TITEN HD ANCHORS				
OTHER				
SHOP WELDING, NON-CERTIFIED & APPROVED SHOPS	X			
FIELD WELDING				
STRUCTURAL STEEL ERECTION			X	
HIGH STRENGTH BOLT CONNECTIONS				
REINFORCING WELDING				
METAL DECKING WELDING				
OTHER				
MASONRY				
GROUT MIX DESIGN				
REINFORCING PLACEMENT IN FOOTING				
REINFORCING PLACEMENT IN WALL				
SAMPLE & TEST MORTAR				
SAMPLE & TEST GROUT				
MASONRY UNIT PLACING & GROUTING				
PRISM TESTS				
BRICK PUSH TESTS				
OTHER				
WOOD CONSTR.				
STRUCTURAL FRAMING MEMBERS & CONNECTIONS				X
DIAPHRAGM NAILING				X
SHEAR WALLS, HOLDOWNS & SHEAR TRANSFER				X
DRAG BEAMS, STRAPS & CONNECTIONS				X
FINAL FRAMING				X
OTHER				
MISC. TESTS				
SPRAY APPLIED FIREPROOFING				
OTHER				

REVISIONS 05/22/2025
 MISSION GROUP ARCHITECTS
 R E JOHNSON AIA
 PROFESSIONAL ENGINEER IN THE STATE OF CALIFORNIA No. 53734 exp. 3/31/24
 GENERAL NOTES, SPECIFICATIONS & TYPICAL DETAILS
 PROPOSED ADU FOR: KISLAK RESIDENCE 3627 CAMPANIL DRIVE SANTA BARBARA, CALIFORNIA 93109
 DATE 11/21/2022
 SHEET S1.1 K-2233
 1280 COAST VILLAGE CIRCLE STE F SANTA BARBARA CALIFORNIA 93108 805-969-5910 © COPYHIGH MISSIONGROUPARCHITECTS



MISSION GROUP ARCHITECTS
RE JOHNSON
AIA
REVISIONS 05/22/2023
GENERAL NOTES, SPECIFICATIONS & TYPICAL DETAILS
PROPOSED ADV FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109
DATE 11/21/2022
SHEET S1.2
K-2233
1800 COAST VILLAGE CIRCLE STE 111 SANTA BARBARA CALIFORNIA 93108 805-969-5910 © COPYRIGHT MISSION GROUP ARCHITECTS

Description of Building Elements	Number and Type of Fastener	Spacing and Location	Description of Building Elements	Number and Type of Fastener	Spacing and Location
1. Blocking between ceiling joists, rafters or trusses to top plate or other framing below	3-8d common (2½"x0.131"); or 3-10d box (3"x 0.128"); or 3-3"x0.131" nails 3-3" 14 gage staples, 7/16" crown	Each end, toenail	19. 1" brace to each stud and plate	2-8d common (2½"x0.131"); or 2-10d box (3"x 0.128"); or 2-3"x0.131" nails; or 2-3" 14 gage staples, 7/16" crown	Face nail
Blocking between rafters or truss not at the wall top plate, to rafter or truss	2-8d common (2½"x0.131") 2-3"x0.131" nails 2-3" 14 gage staples	Each end, toenail	20. 1" x 6" sheathing to each bearing	2-8d common (2½"x0.131"); or 2-10d box (3"x 0.128")	Face nail
Flat blocking to truss and web filler	2-16d common (3½"x0.162) 2-3"x0.131" nails 2-3" 14 gage staples	End nail	21. 1" x 8" and wider sheathing to each bearing	3-8d common (2½"x0.131"); or 3-10d box (3"x 0.128")	Face nail
2. Ceiling joists to top plate	3-8d common (2½" x 0.131"); or 3-10d box (3"x0.128"); or 3-3"x0.131" nails 3-3" 14 gage staples, 7/16" crown	Each joist, toenail	Floor		
3. Ceiling joist not attached to parallel rafter, laps over partitions (no thrust) (see Section 2308.7.3.1, Table 2308.7.3.1)	3-16d common (3½"x0.162"); or 4-10d box (3"x0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 7/16" crown	Face nail	22. Joist to sill, top plate, or girder	3-8d common (2½"x0.131"); or 3-10d box (3"x 0.128"); or 3-3" 14 gage staples, 7/16" crown	Toenail
4. Ceiling joist attached to parallel rafter (heel joint) (see Section 2308.7.3.1, Table 2308.7.3.1)	Per Table 2308.7.3.1	Face nail	23. Rim joist, band joist, or blocking to top plate, sill or other framing member	8d common (2½"x0.131"); or 10d box (3"x 0.128"); or 3"x0.131" nails; or 3" 14 gage staples, 7/16" crown	6"oc, toenail
5. Collar tie to rafter	3-10d common (3"x0.148"); or 4-10d box (3"x0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 7/16" crown	Face nail	24. 1" x 6" subfloor or less to each joist	2-8d common (2½"x0.131"); or 2-10d box (3"x 0.128")	Face nail
6. Rafter or roof truss to top plate (See Section 2308.7.5, Table 2308.7.5)	3-10d common (3"x0.148"); or 3-16d box (3½"x0.155"); or 4-10d box (3"x 0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 7/16" crown	Toenail	25. 2" subfloor to joist or girder	2-16d common (3½"x0.162")	Face nail
7. Roof rafters to ridge valley or hip rafters; or roof rafter to 2-inch ridge beam	2-16d common (3½"x0.162"); or 3-10d box (3"x 0.128"); or 3-3"x0.131" nails 3-3" 14 gage staples, 7/16" crown; or 3-10d common (3"x0.148"); or 3-16d box (3½"x0.155"); or 4-10d box (3"x 0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 7/16" crown	End nail	26. 2" planks (plank & beam - floor & roof)	2-16d common (3½"x0.162")	Each bearing, face nail
Wall	8. Stud to stud (not at braced wall panels)	24"oc face nail 16"oc face nail	27. Built-up girders & beams, 2" lumber layers	20d common (4"x0.192") 10d box (3"x 0.128"); or 3"x0.131" nails; or 3" 14 gage staples, 7/16" crown And: 2-20d common (4"x0.192") 3-10d box (3"x 0.128"); or 3-3"x0.131" nails; or 3-3" 14 gage staples, 7/16" crown	32"oc, face nail at top and bottom staggered on opposite sides 24"oc, face nail at top and bottom staggered on opposite sides
	9. Stud to stud and abutting studs at intersecting wall corners (at braced wall panels)	16"oc face nail 12"oc face nail 12"oc face nail	28. Ledger strip supporting joists or rafters	3-16d box (3½"x0.155"); or 4-10d box (3"x 0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 7/16" crown	Each joist or rafter, face nail
	10. Built-up header (2" to 2") header)	16"oc each edge, face nail 12"oc each edge, face nail	29. Joist to band joist or rim joist	3-16d box (3½"x0.155"); or 4-10d box (3"x 0.128"); or 4-3"x0.131" nails 4-3" 14 gage staples, 7/16" crown	End nail
	11. Continuous header to stud	Toenail	30. Bridging or blocking to joist, rafter or truss	2-8d common (2½"x0.131"); or 2-10d box (3"x 0.128"); or 2-3"x0.131" nails 2-3" 14 gage staples, 7/16" crown	Each end, toenail
	12. Top plate to top plate	12"oc face nail	Wood structural panels (WSP), subfloor, roof and interior wall sheathing to framing and particleboard wall sheathing to framing		
	13. Top plate to top plate, at end joints	Each side of end joint, face nail (minimum 24" lap splice length each side of end joint)	31. ¾" - 1½"	6d common or deformed (2"x0.113") (subfloor and wall) 8d box or deformed (2½"x0.131)(roof) 2½"x0.113" nail (subfloor and wall) 1¾" 16 gage staple, 7/16" crown (subfloor and wall) 2½"x0.113" nail (roof) 1¾" 16 gage staple, 7/16" crown (roof)	Edges (inches) Intermediate supports (inches)
	14. Bottom plate to joist, rim joist, band joist or blocking (not at braced wall panels)	16"oc face nail	32. 1½" - ¾"	8d common (2½"x0.131); or 6d deformed (2"x0.113") 2½"x0.113" nail; or 2" 16 gage staple, 7/16" crown	6 12 6 12 4 8 3 6
	15. Bottom plate to joist, rim joist, band joist or blocking at braced wall panels	16"oc face nail	33. 7/8" - 1¼"	10d common (3"x0.148"); or 8d deformed (2½"x0.131)	6 12
	16. Stud to top or bottom plate	End nail	Other exterior wall sheathing		
	17. Top or bottom plate to stud	End nail	34. ½" fiberboard sheathing ³	1½" galvanized roofing nail (7/16" head diameter); or ¼" 16 gage staple with 7/16" or 1" crown	3 6
18. Top plates, laps at corners and intersections	Face nail	35. ¾" fiberboard sheathing ³	1½" galvanized roofing nail (7/16" head diameter); or ¼" 16 gage staple with 7/16" or 1" crown	3 6	
			Wood structural panels combinations subfloor underlayment to framing		
			Panel siding to framing		
			Wood structural panels (WSP), subfloor, roof and interior wall sheathing to framing and particleboard wall sheathing to framing		

Table 2304.10.1 Minimum Fastening Schedule (Wood)

Development l_d of Standard Hooks

Bar Size	Hook Development Lengths
#3	10"
#4	12"
#5	14"
#6	17"
#7	19"
#8	22"

Recommended End Hooks

Bar Size	D (in.)	180° Hook		90° Hook	
		A or G	J	A or G	
#3	2½"	5"	3"	6"	
#4	3"	6"	4"	8"	
#5	3¾"	7"	5"	10"	
#6	4½"	8"	6"	12"	
#7	5½"	10"	7"	14"	
#8	6"	11"	8"	16"	

Seismic Stirrups/Ties

Bar Size	D (in.)	135° Seismic Hook	
		A or G	H
#3	1½"	4¼"	3"
#4	2"	4½"	3"
#5	2½"	5½"	3¾"
#6	4½"	8"	4½"
#7	5½"	9"	5½"
#8	6"	10½"	6"

Tension Development & Lap Splice Lengths - Masonry

Bar Size	D (in.)	Development & Lap Splice Lengths		
		Edge	Center	40db or 12" min.
#2	40	17"	12"	12"
	60	26"	14"	12"
	26"	14"	15"	
#3	60	38"	21"	15"
	40	34"	18"	20"
#4	60	51"	27"	20"
	60	63"	34"	25"
#6	60	99"	53"	30"
#7	60	115"	62"	35"
#8	60	152"	81"	40"

Compression Development and Lap Splice Lengths - Concrete

Bar Size	Grade	Development Length		Lap Splice	
		2500psi	3000psi	2500psi	3000psi
#3	40	8"	8"	16"	12"
	60	10"	9"		
#4	40	9"	8"	20"	15"
	60	12"	11"		
#5	40	14"	14"	26"	19"
	60	16"	17"	31"	23"
#7	40	18"	19"	36"	27"
	60	20"	22"	40"	30"

Notes:

- Tabulated values are based on Grade 40 or 60 reinforcing bars and normal-weight concrete.
- Tension development lengths and lap splice lengths are based on ACI 318-14, Sections 25.4.3 and 25.4.4, respectively. Lengths are in inches.
- For epoxy coated hooks, multiply by 1.2.

Notes:

- Tabulated values are based on Grade 40 or 60 reinforcing bars and normal-weight concrete.
- Compression development lengths and lap splice lengths are based on ACI 318-14, Sections 25.4.9 and 25.5.5, respectively. Lengths are in inches.

Notes:

- Tabulated values are based on Grade 40 or 60 reinforcing bars and normal-weight concrete.
- Tension development lengths and lap splice lengths are based on ACI 318-14, Sections 25.4.2.2 and 25.5, respectively. Tabulated values for beams or columns are based on transverse reinforcement and concrete cover meeting minimum Code requirements. Lengths are in inches.
- Lap Class A values are the required tension development lengths, l_d ; lap splice lengths are multiples of tension development lengths; Class A splice = 1.0 l_d and Class B splice = 1.3 l_d (ACE 318-14, Section 25.1).
- For lightweight aggregate concrete, multiply the tabulated values by 1.3.
- For epoxy-coated bars, multiply the tabulated values by 1.20.

Reinforcing Laps, Bends & Splices

MISSION GROUP ARCHITECTS

RE JOHNSON AIA

REVISIONS
03/22/2023

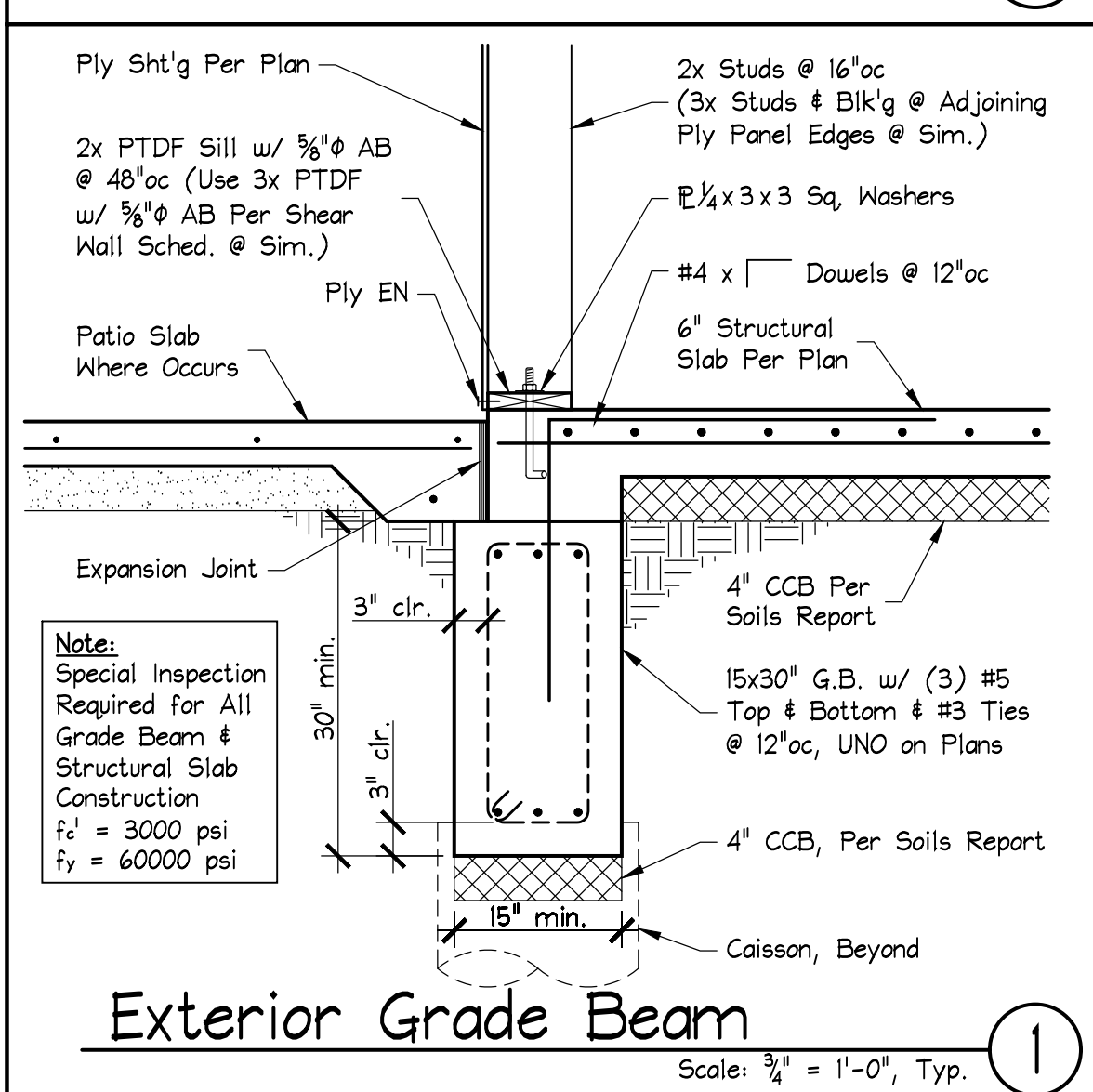
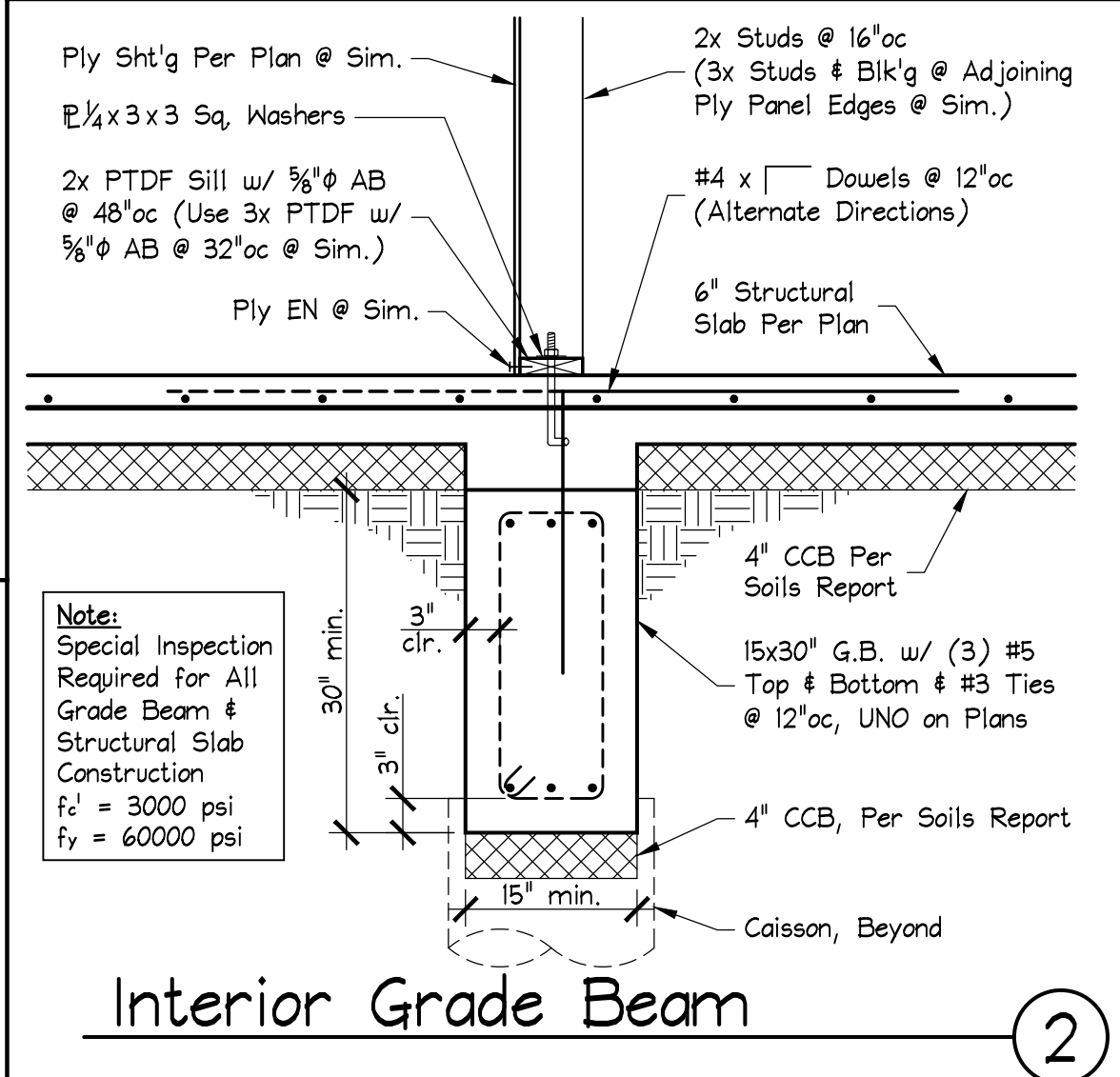
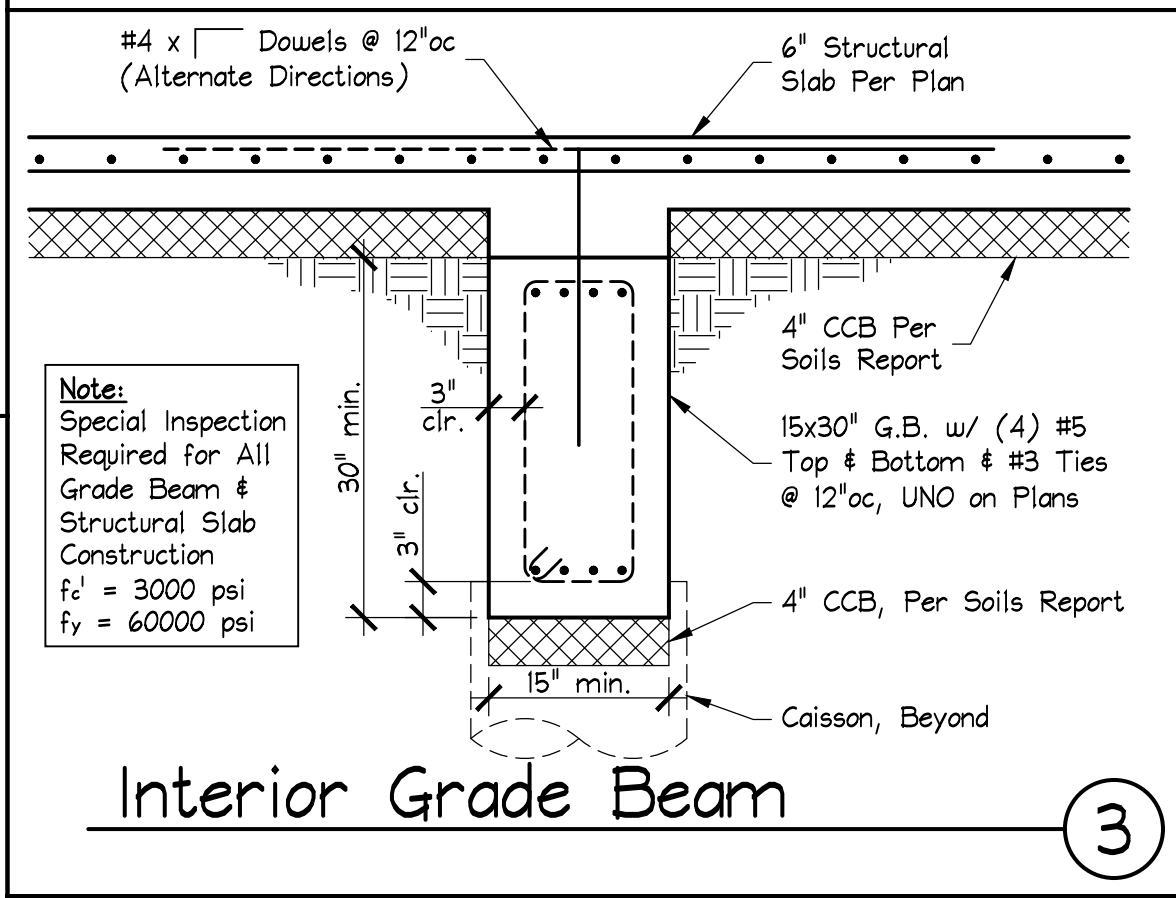
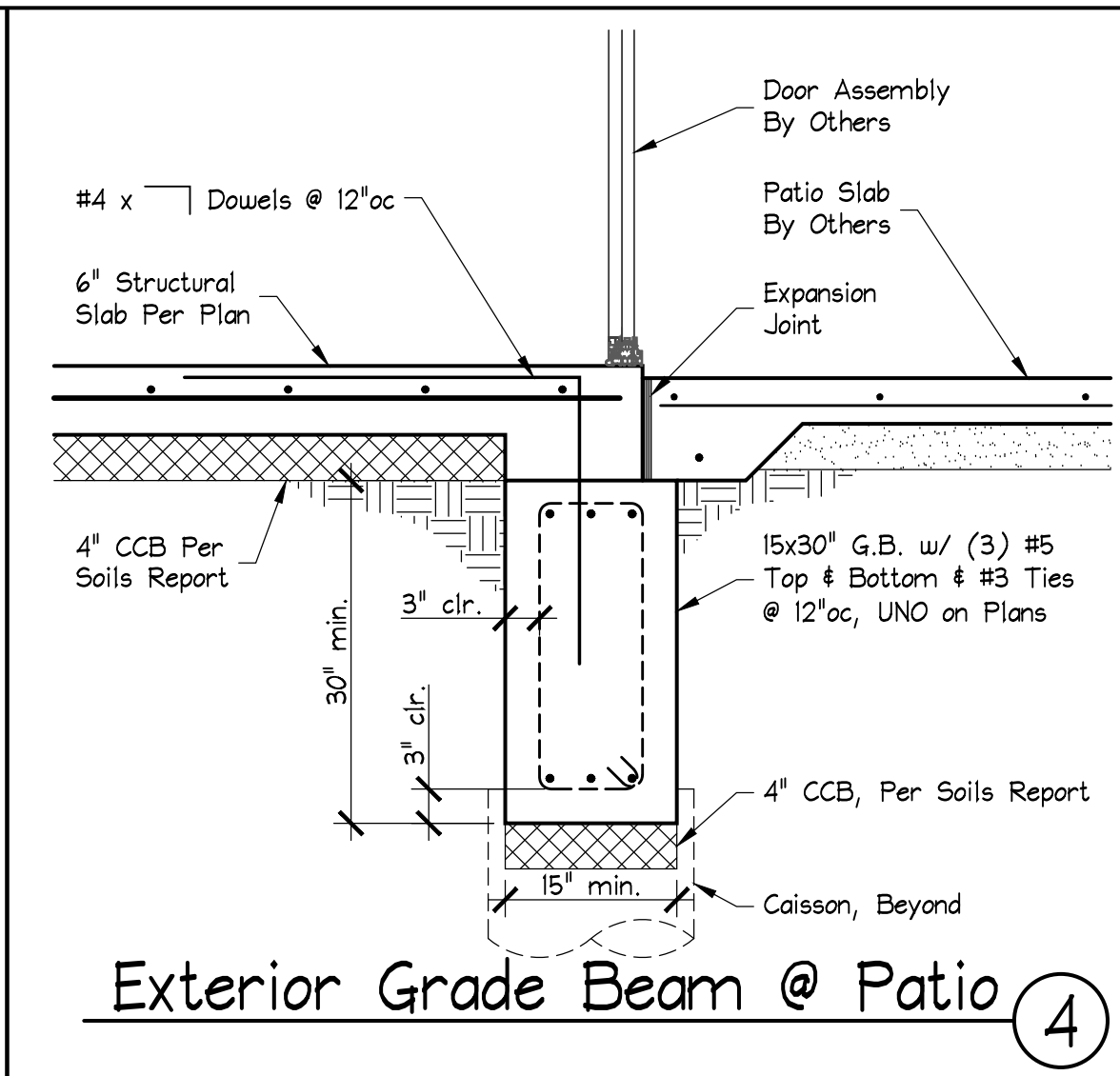
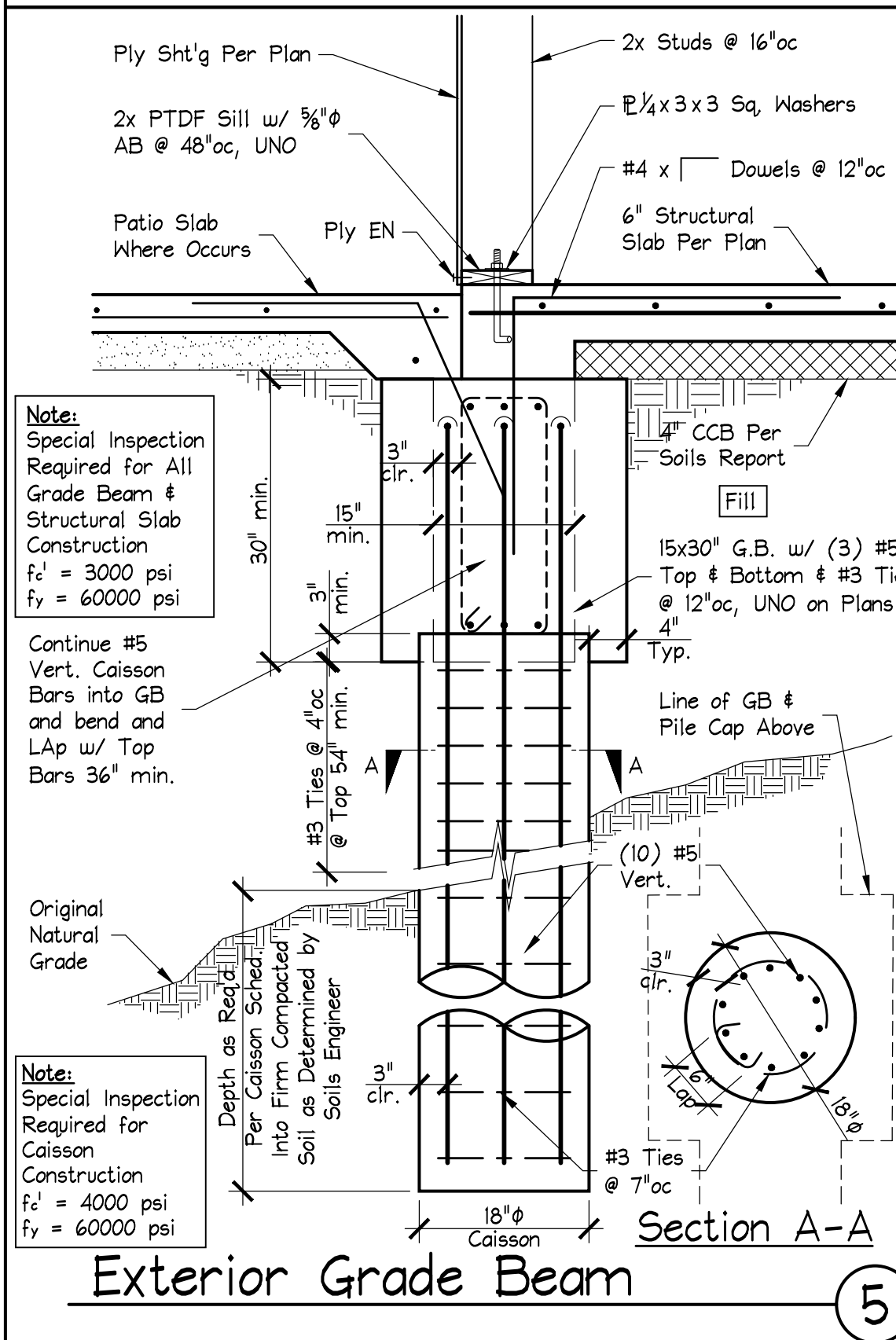
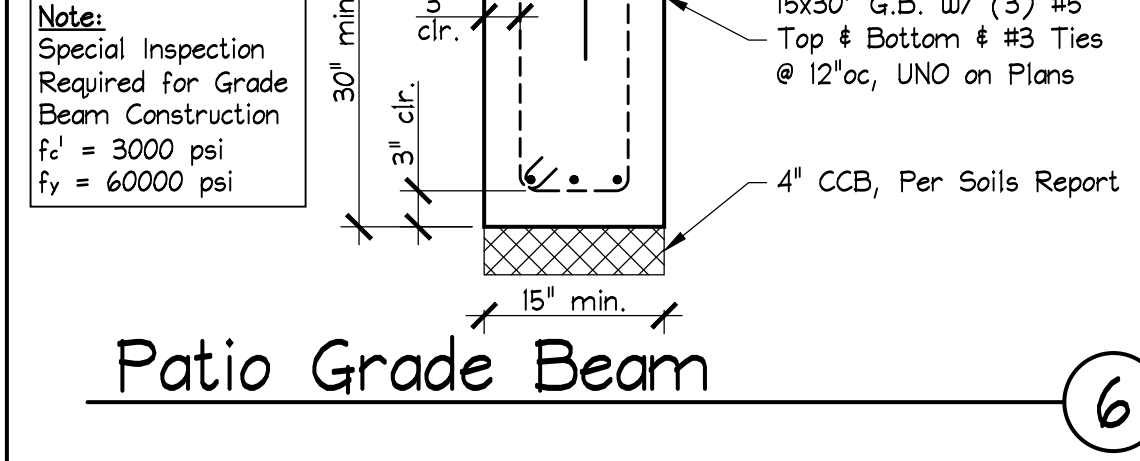
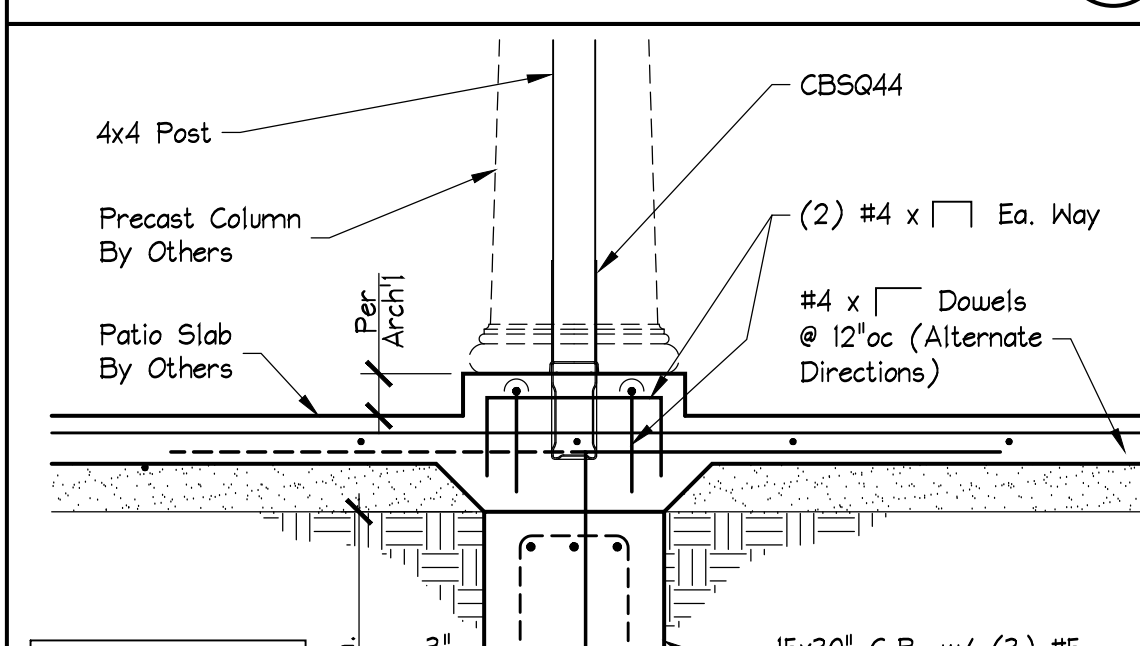
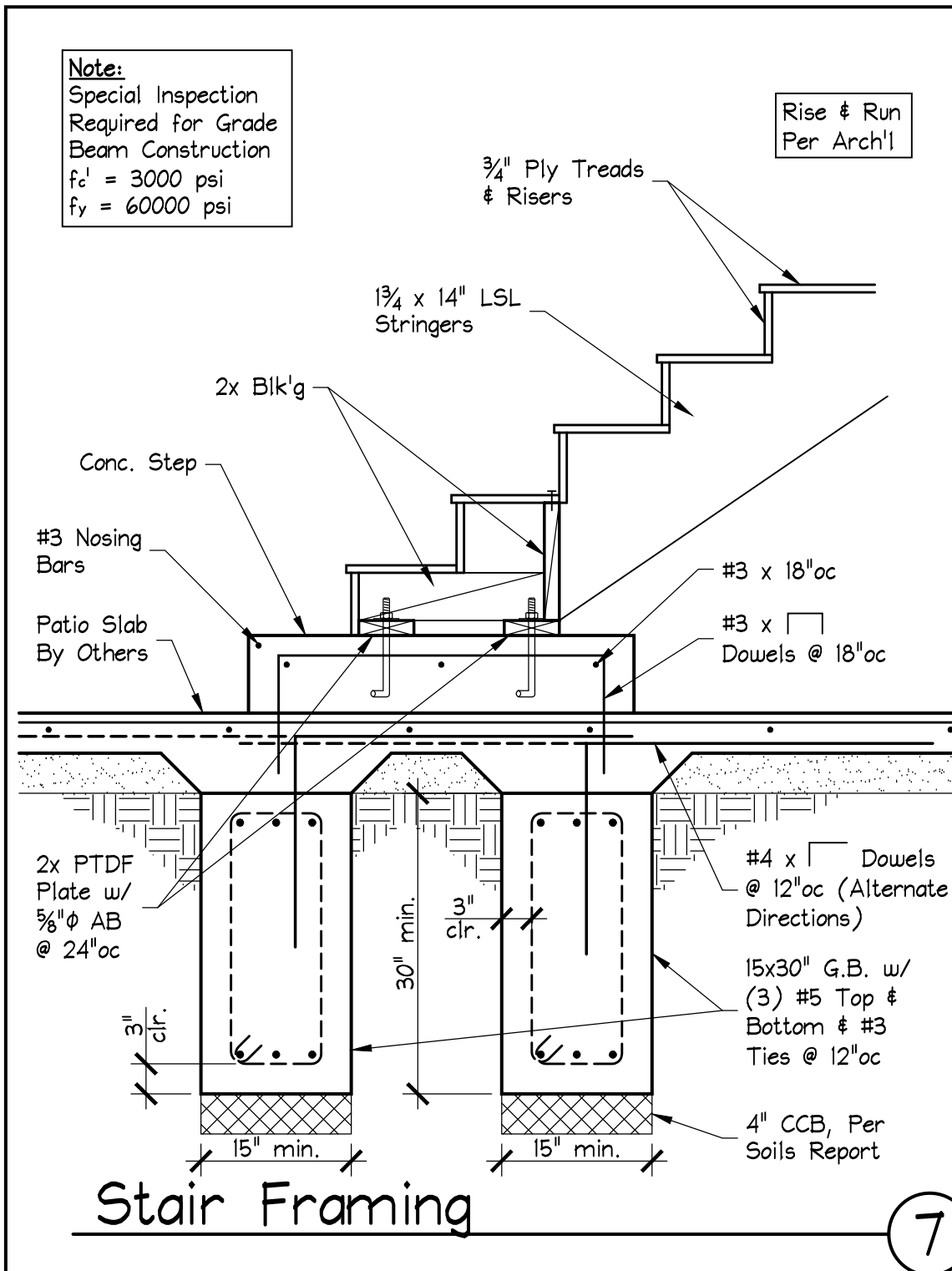
PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109

DATE: 11/21/2022

SHEET: S1.3
K-2253

GENERAL NOTES, SPECIFICATIONS & TYPICAL DETAILS

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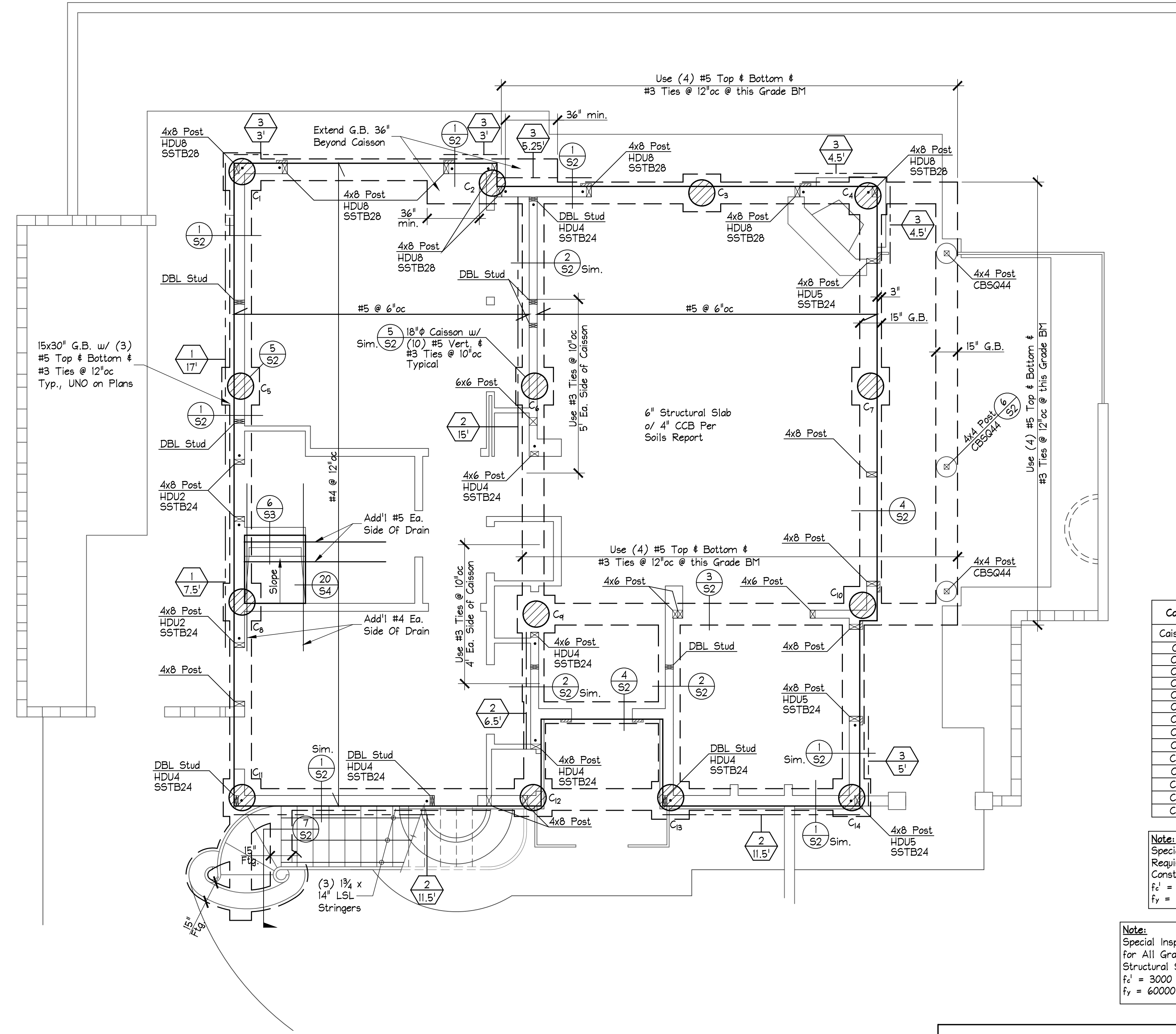


TYPICAL FOUNDATION NOTES

- Prepare Site per Soils Report By: Braun & Associates, Inc. Dated: December 22, 2016, Updated: March 31, 2018 File # 16-11153-2
- Contractor to Verify all Dimensions and Coordinate with Architectural Plans.
- All Anchor Bolts, Slab Dowels, etc. are to be Tied in Place and Holddown Anchors Set in Place by Template Prior to Requesting a Building Department Foundation Inspection.
- For Shear Wall Anchor Bolting, See Shear Wall Schedule.
- Prior to the Contractor Requesting a Building Department Foundation Inspection, the Soils Engineer Shall Advise the Building Official in Writing that:
 - The Building Pad was Prepared in Accordance with the Soils Report
 - The Utility Trenches have been Properly Backfilled and Compacted
 - The Foundation Excavations Comply with the Intent of the Soils Report.
- Holddowns shall be Retightened just Prior to Covering the Wall Framing.
- All Connectors and/or Fasteners in Contact with Pressure Treated Lumber shall be Stainless Steel (304 or 316) or Hot Dipped Galvanized (ASTM A-123 & A-153) G85 minimum.

SYM	Ply and Nailing	Sill Anchorage		Blk'g, Rim or Drag to Top Plate		Framing			Allowable Shear Load	Special Inspection
		Concrete	Wood	Solid Member	TJI Blk'g	Top Plate	Sill # Plate	Studs # Blk'g @ Ply Panel Edges		
1	1/2" Ply C-DX 10d @ 6" oc EN 10d @ 12" oc FN	5/8" x 12" AB @ 48" oc	16d @ 6" oc	A35 @ 16" oc	16d @ 6" oc	2x	2x	2x	310 #/ft	N/A
2	1/2" Ply C-DX 10d @ 4" oc EN 10d @ 12" oc FN	5/8" x 12" AB @ 32" oc	1 1/2" x 6" SDS Screws @ 8" oc	A35 @ 12" oc	16d @ 4" oc	2x	3x	3x	460 #/ft	N/A
3	1/2" Ply Struct. 1 10d @ 3" oc EN 10d @ 12" oc FN	5/8" x 12" AB @ 24" oc	1 1/2" x 6" SDS Screws @ 6" oc	A35 @ 8" oc	N/A	3x	3x	3x	665 #/ft	N/A

- Use Common Nails Only, Do Not Use Sinkers! 10d Min Shank Dia. = .148"
- Use 1/4" x 3/4" Square Washers at All Anchor Bolts
- Where Nails Are Spaced Closer Than 6" oc, the nails Shall be Staggered.
- Site Built Shear Walls to Maintain a Minimum 2:1 Height to Width Ratio, U.N.O.
- Install Simpson Strong Wall Per Manufacturer's Recommendations.
- Simpson Strong Wall Concrete Templates Shall be Used to Correctly Position AB's and HD Anchors for Simpson Strong Wall Products.
- For Shear Wall Construction Details, see Detail (S1) (S2)
- Use (2) 20d Common Nails to Each Stud In Lieu of (2) 16d where 3x Sill Occurs
- Where Panels Are Applied on Both Faces of a Shear Wall and Nail Spacing is Less Than 6" oc on Either Side, Panel Joints Shall be Offset to Fall on Different Framing Members. Alternatively, the Width of the Nailed Face of Framing Member Shall be 3" Nominal or Greater @ Adjoining Ply Panel Edges. Nails at All Adjoining Panel Edges Shall be Staggered.
- WSW Shear Walls: ICC-ES ESR-2652
** SSW Shear Walls: ICC-ES ESR-1679



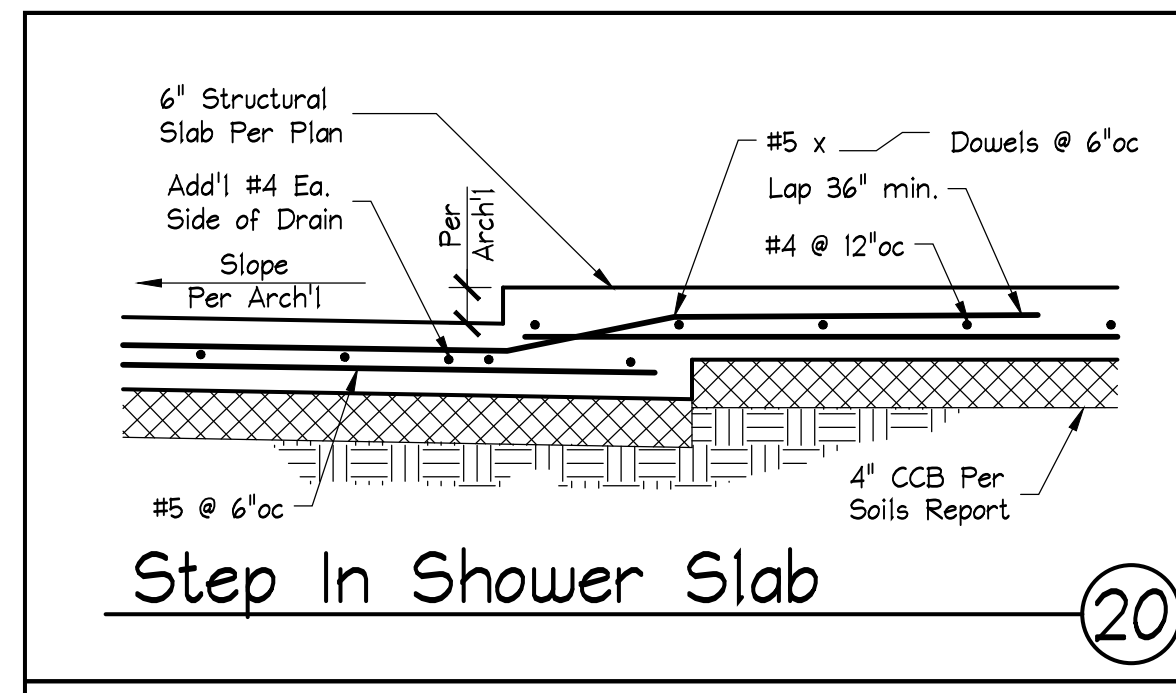
Caisson Schedule

Caisson	Min. Depth
C1	15'
C2	15'
C3	15'
C4	15'
C5	20'
C6	20'
C7	20'
C8	21'
C9	15'
C10	26'
C11	31'
C12	15'
C13	16'
C14	15'

Note:
Special Inspection Required for Caisson Construction
f'c = 4000 psi
fy = 60000 psi

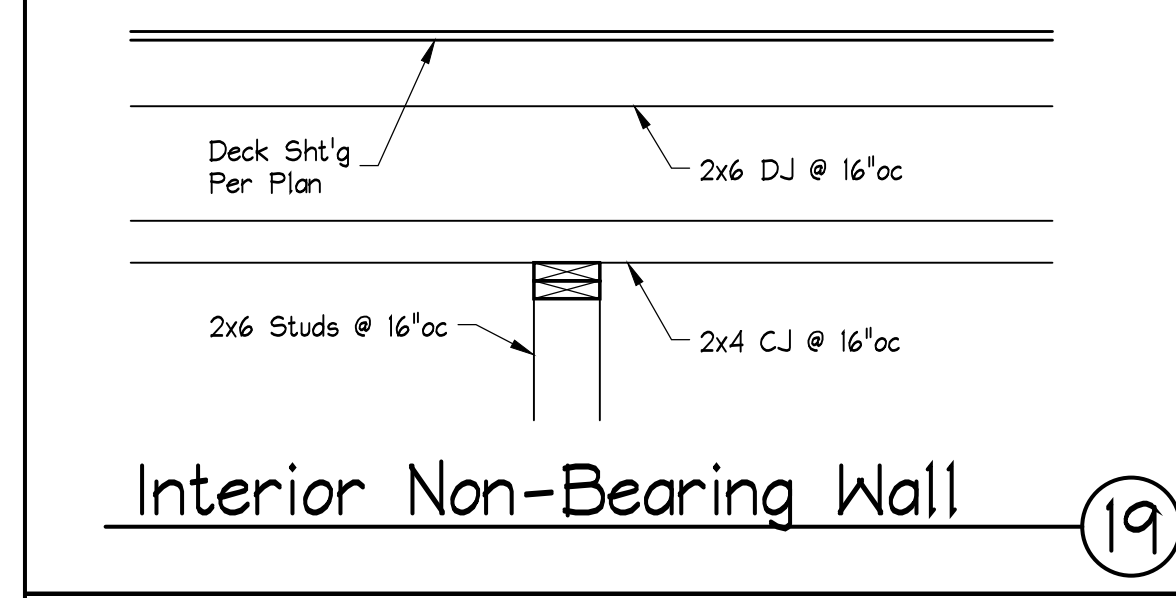
NOTE:
1. EOR to be Notified 48 Hours in Advance to Schedule Structural Observations as Noted in the Structural Observation Program on Sheet S11.
2. Required Special Inspections as Noted on S11 to be Performed by an Approved and Licensed Special Inspector

REVISIONS 03/22/2023
 MISSION GROUP ARCHITECTS
 RE JOHNSON AIA
 FOUNDATION PLAN & DETAILS
 KISLAK RESIDENCE
 3627 CAMPANIL DRIVE
 SANTA BARBARA, CALIFORNIA 93109
 805-969-9910
 1230 COAST VILLAGE CIRCLE STEH SANTA BARBARA, CALIFORNIA 93108
 PROPOSED ADU FOR:
 DATE 11/21/2022
 SHEET S2 K-2255



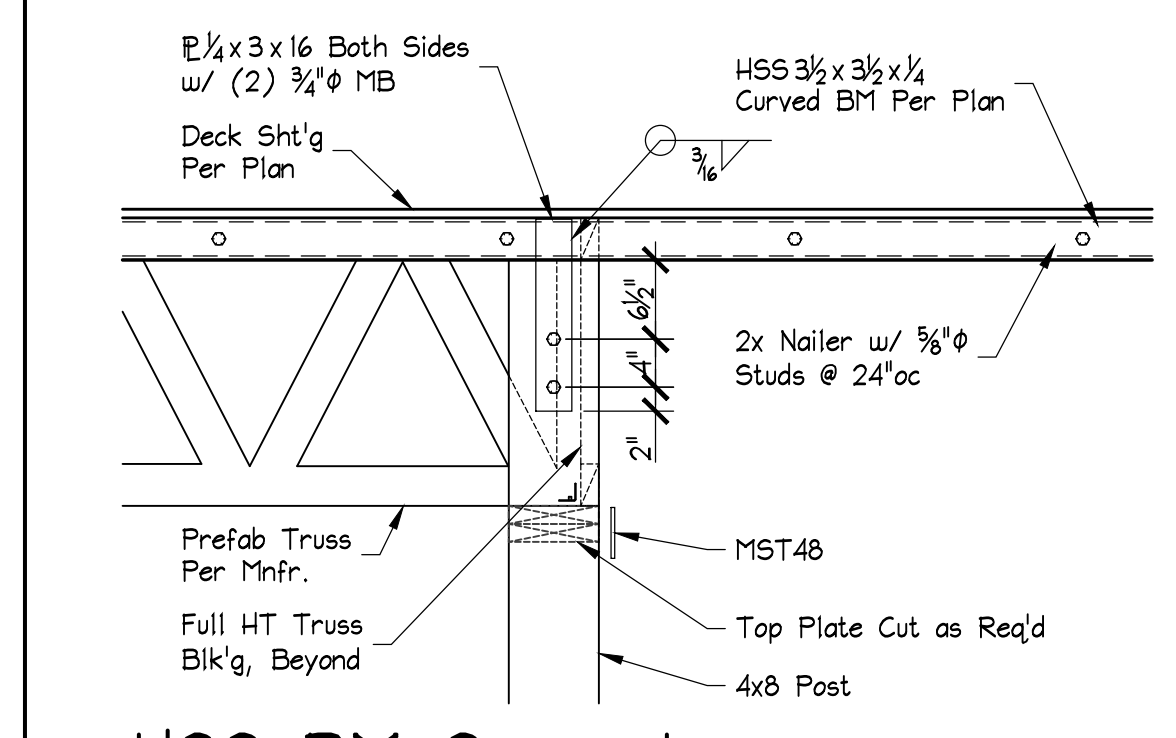
Step In Shower Slab

20



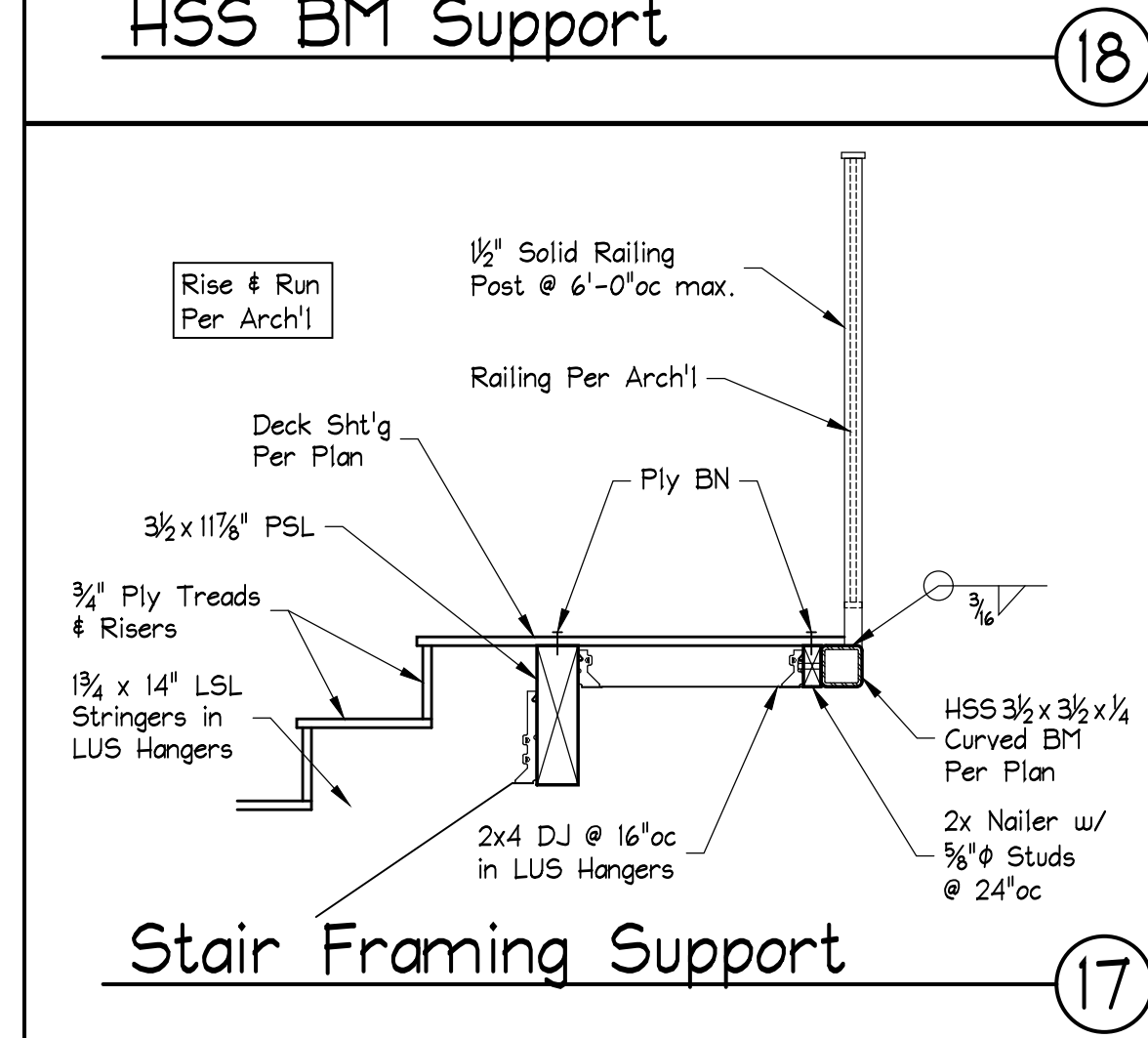
Interior Non-Bearing Wall

19



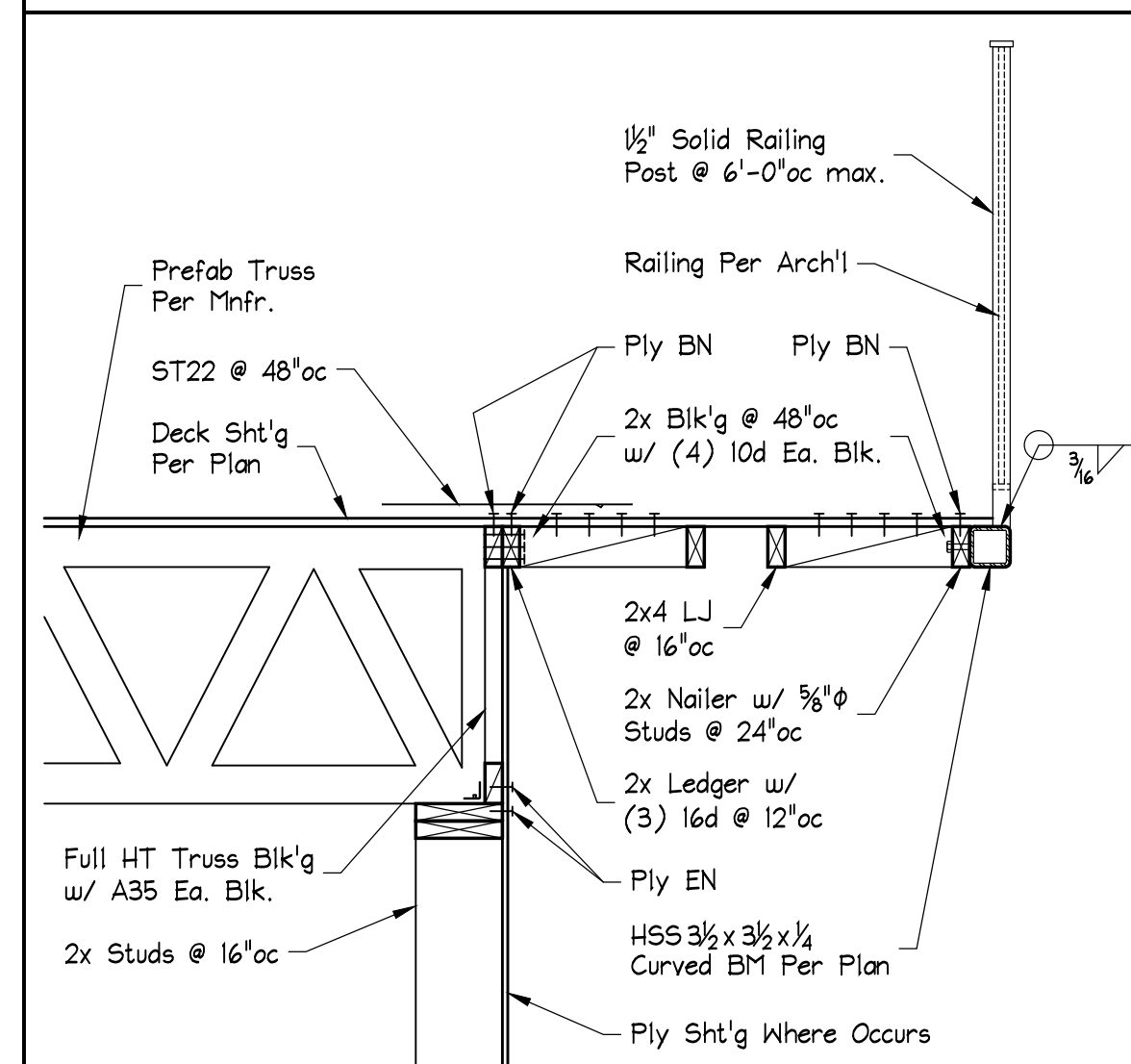
HSS BM Support

18



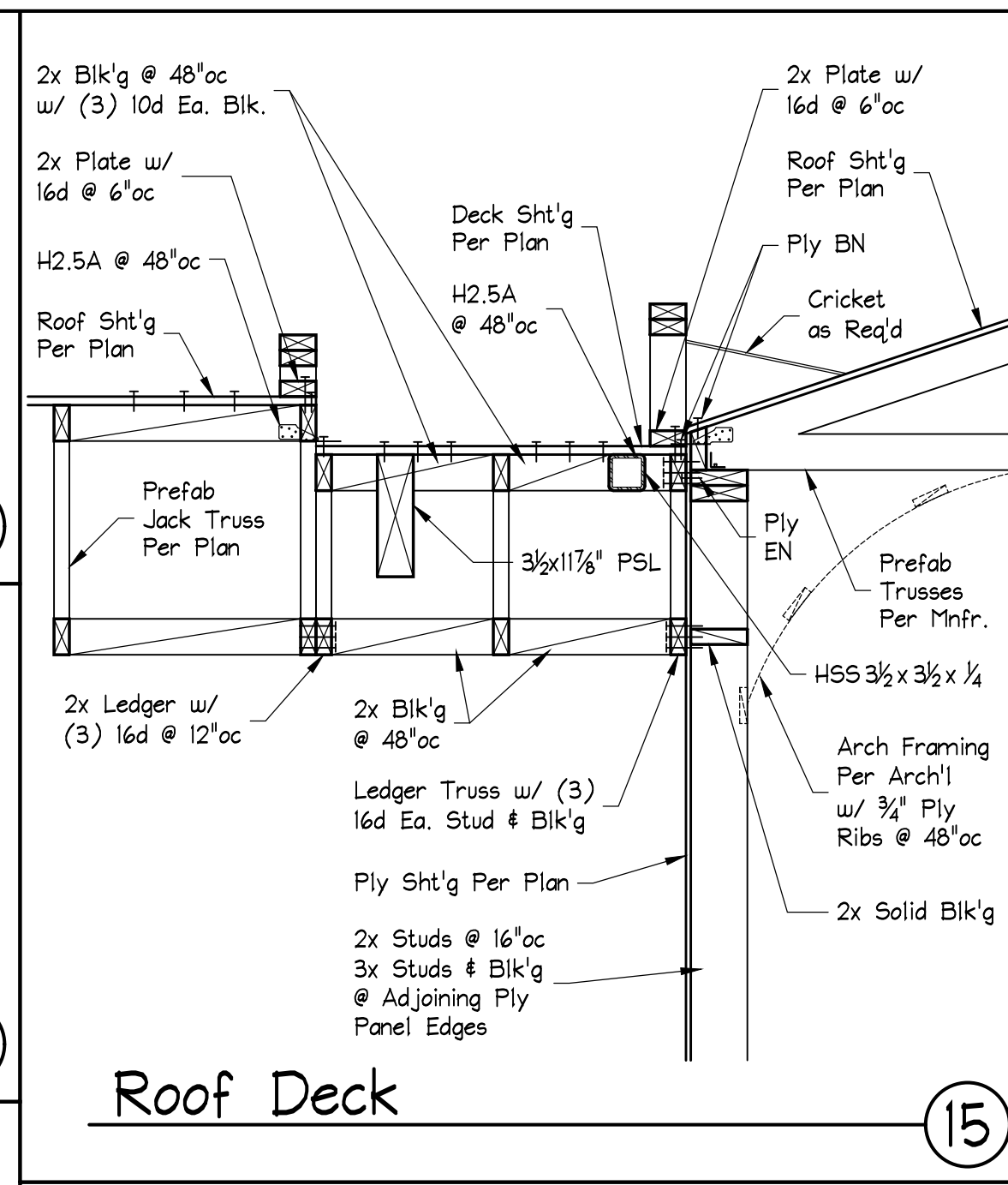
Stair Framing Support

17



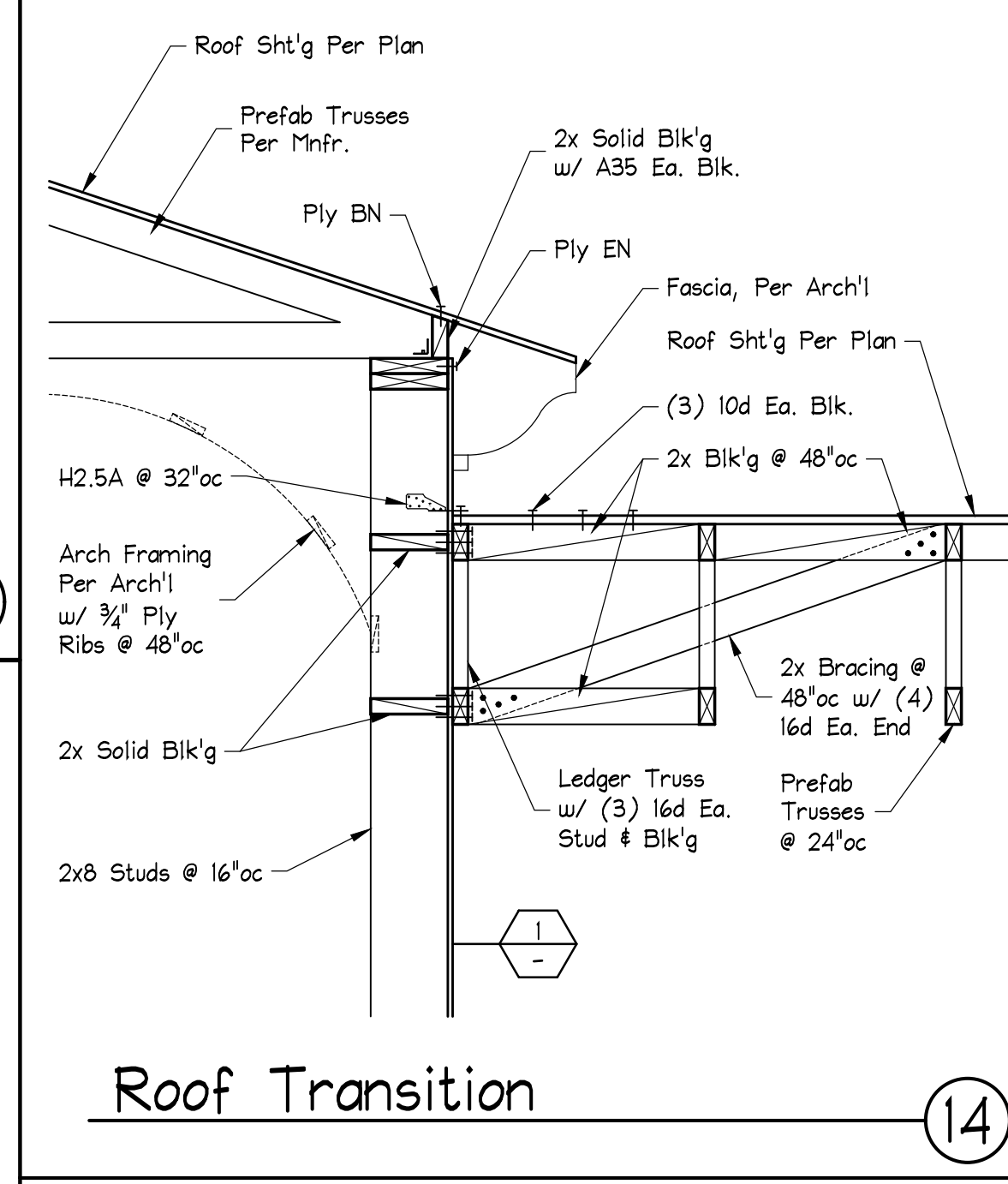
Landing @ Roof Deck

16



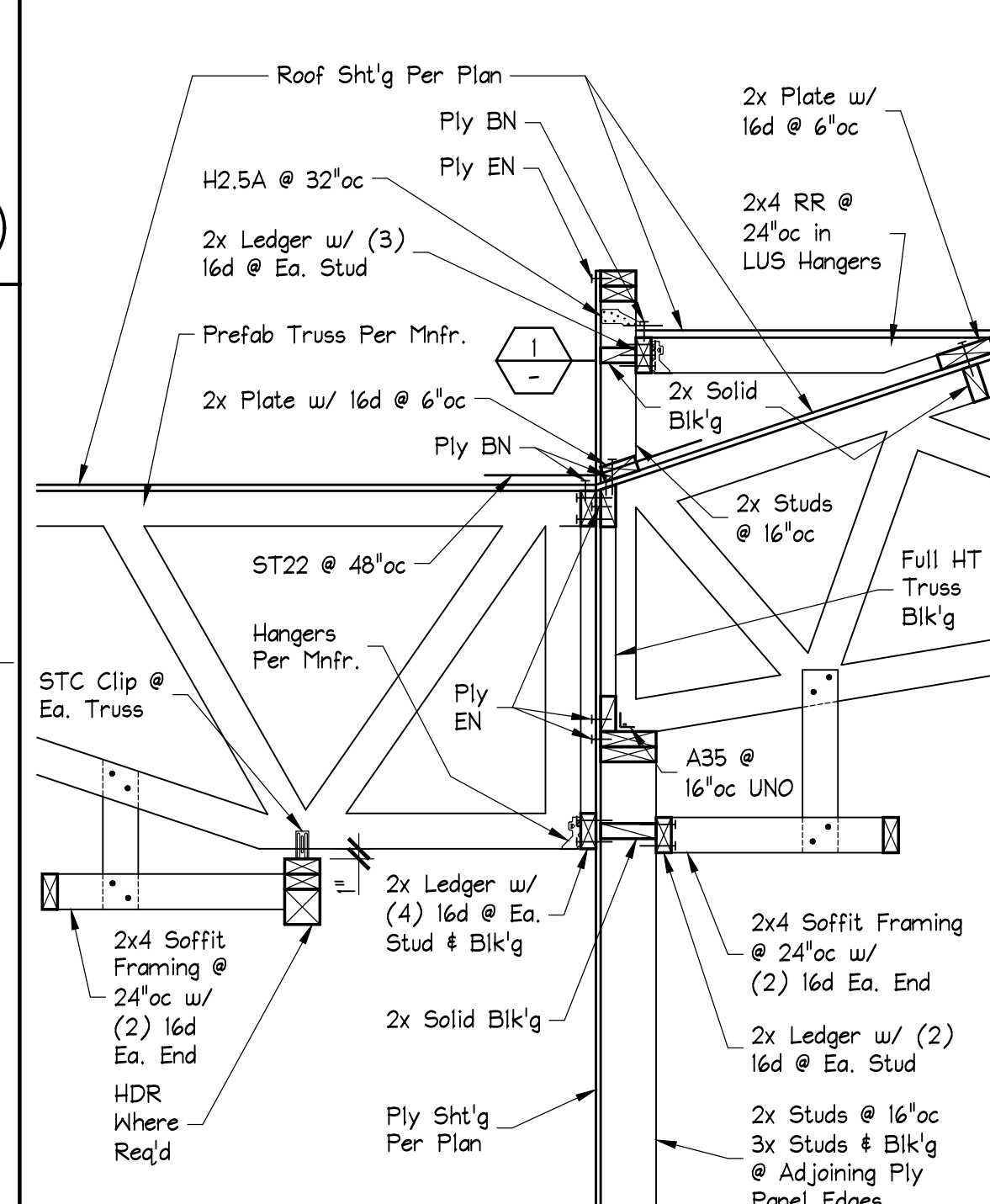
Roof Deck

15



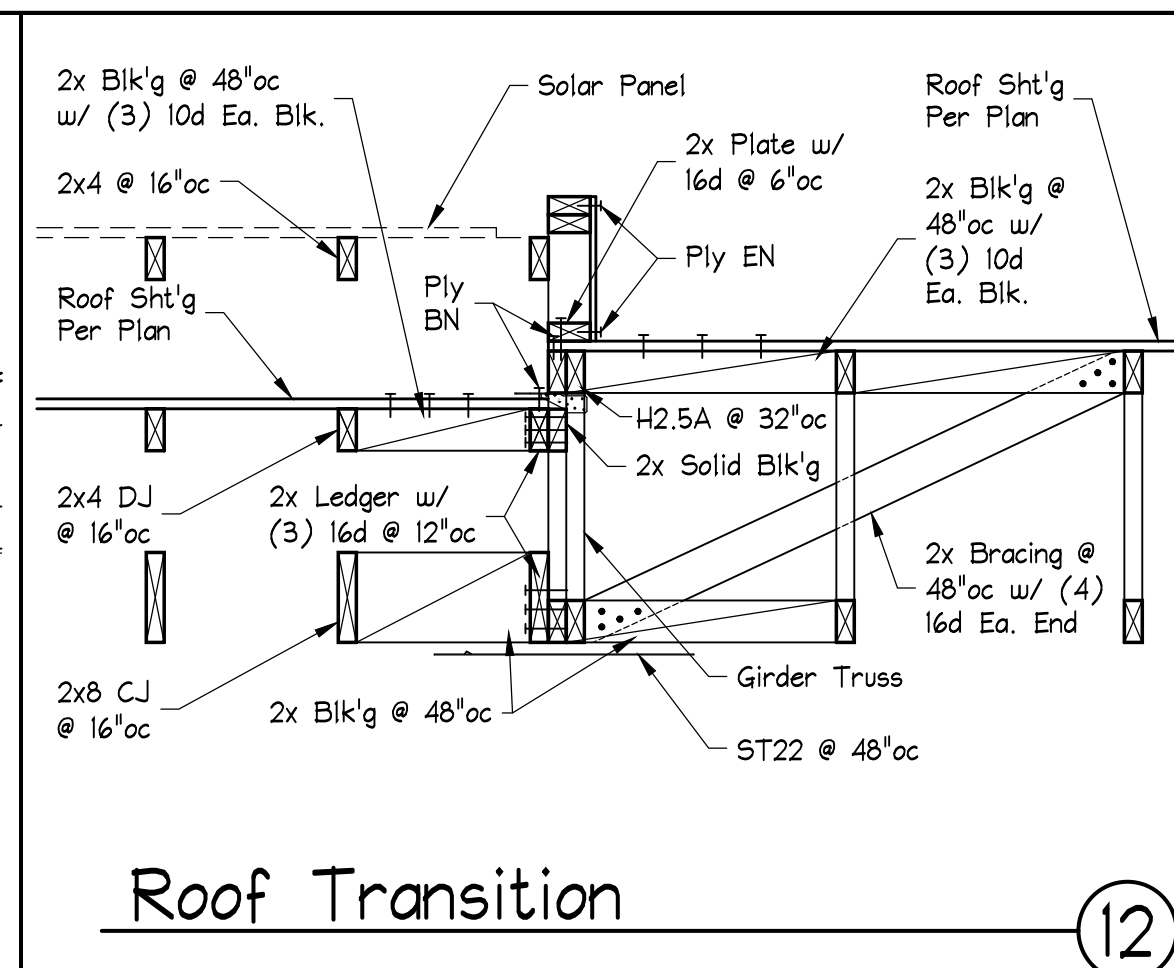
Roof Transition

14



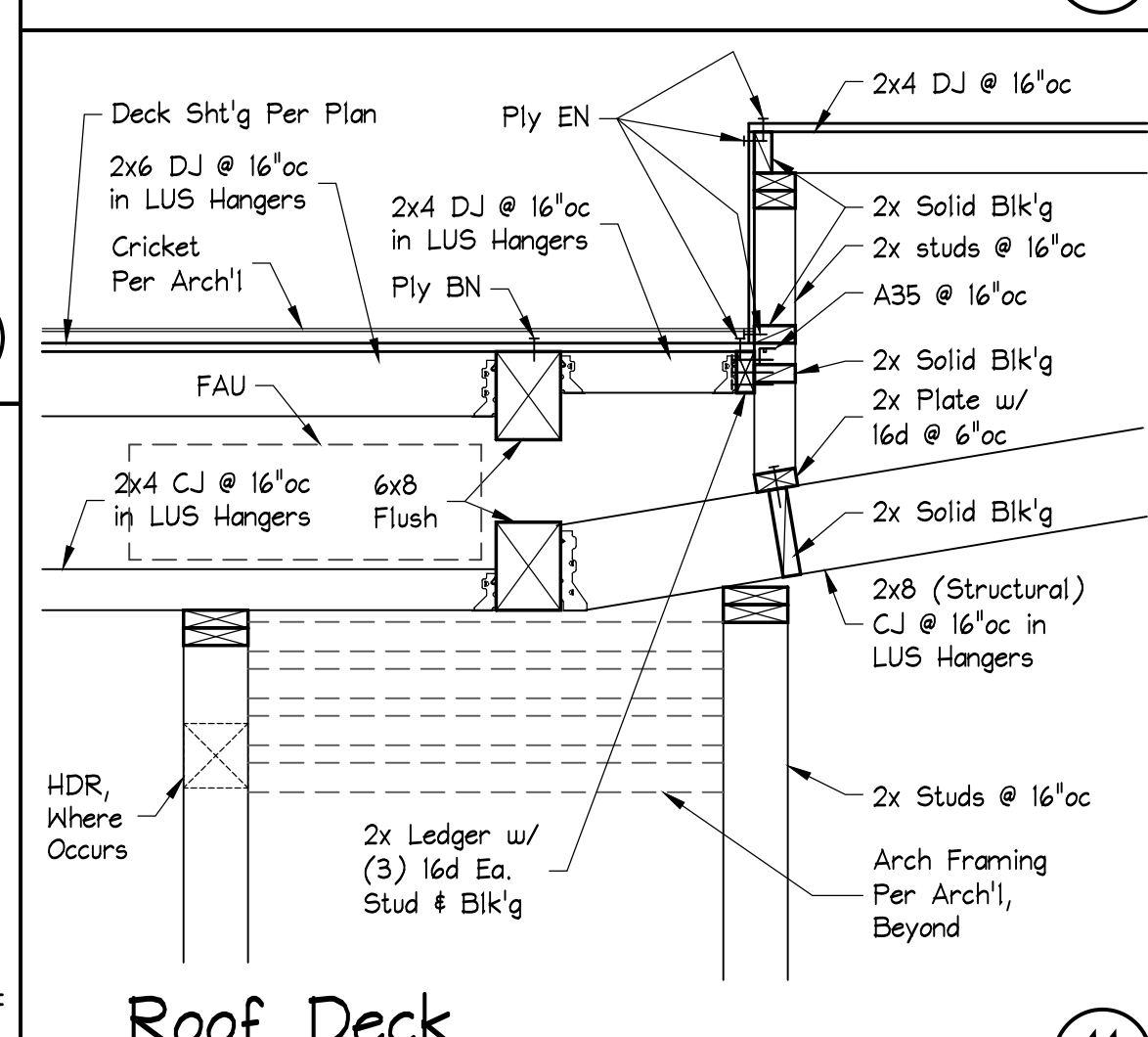
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13



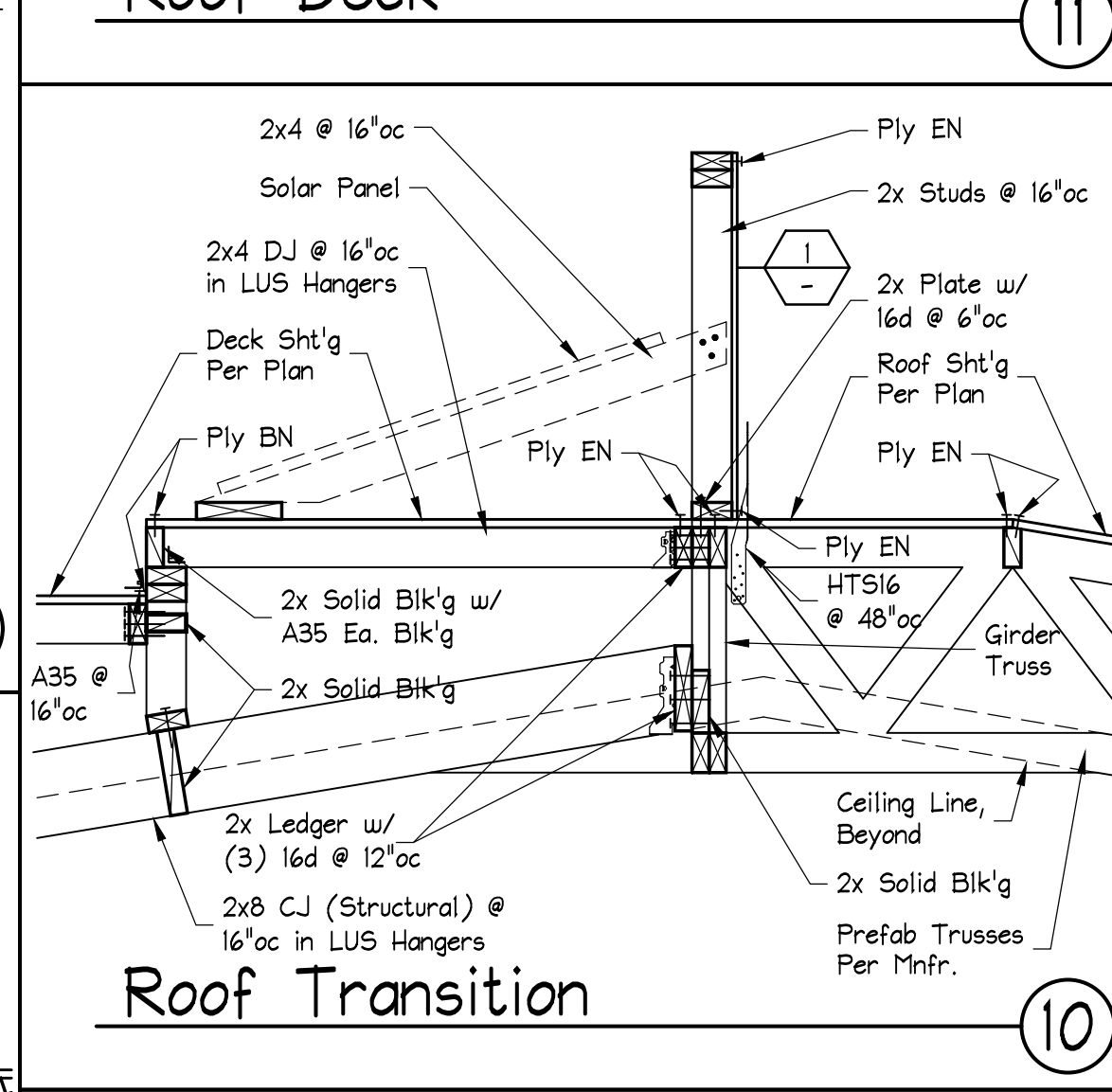
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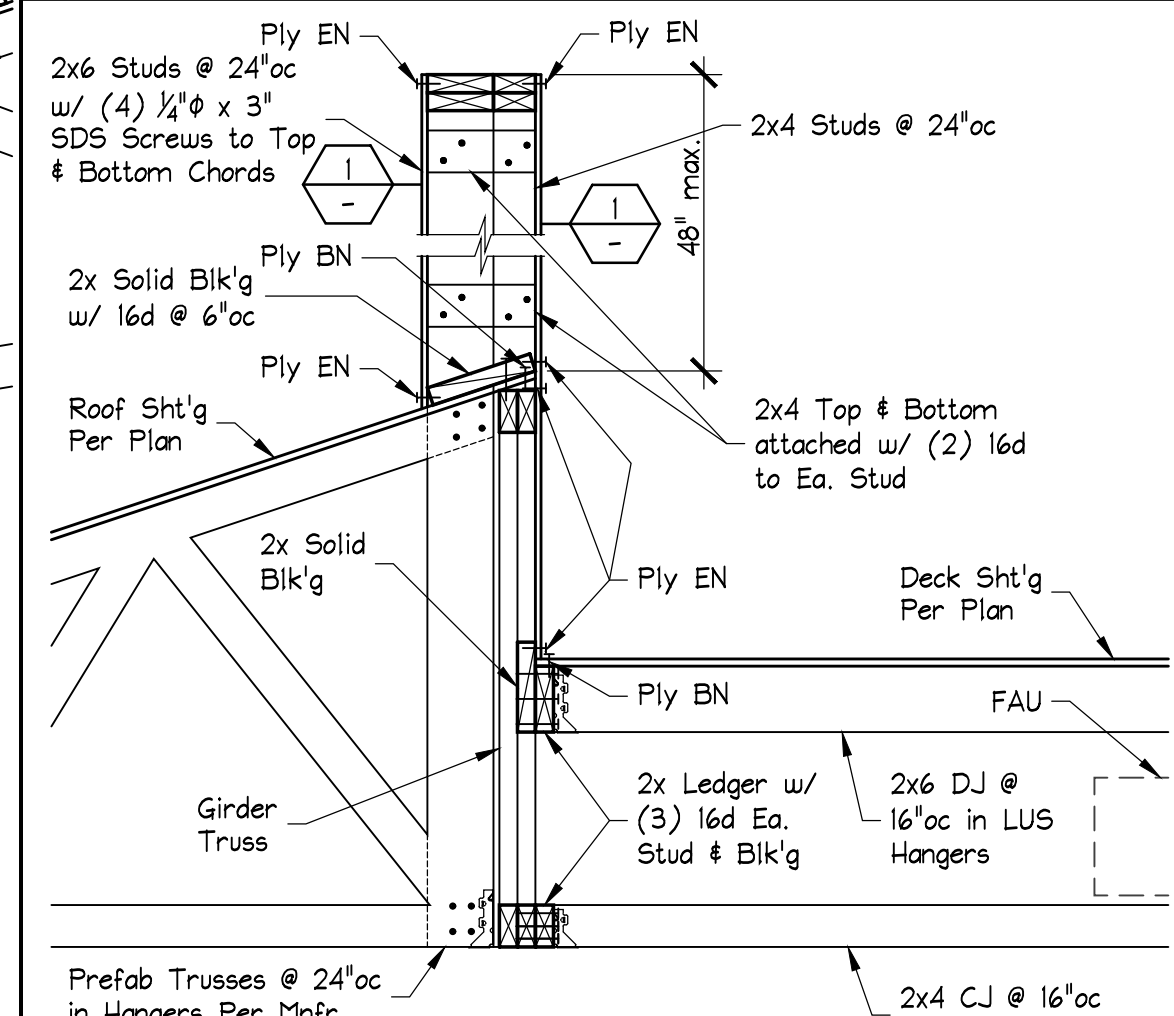
Roof Deck

11



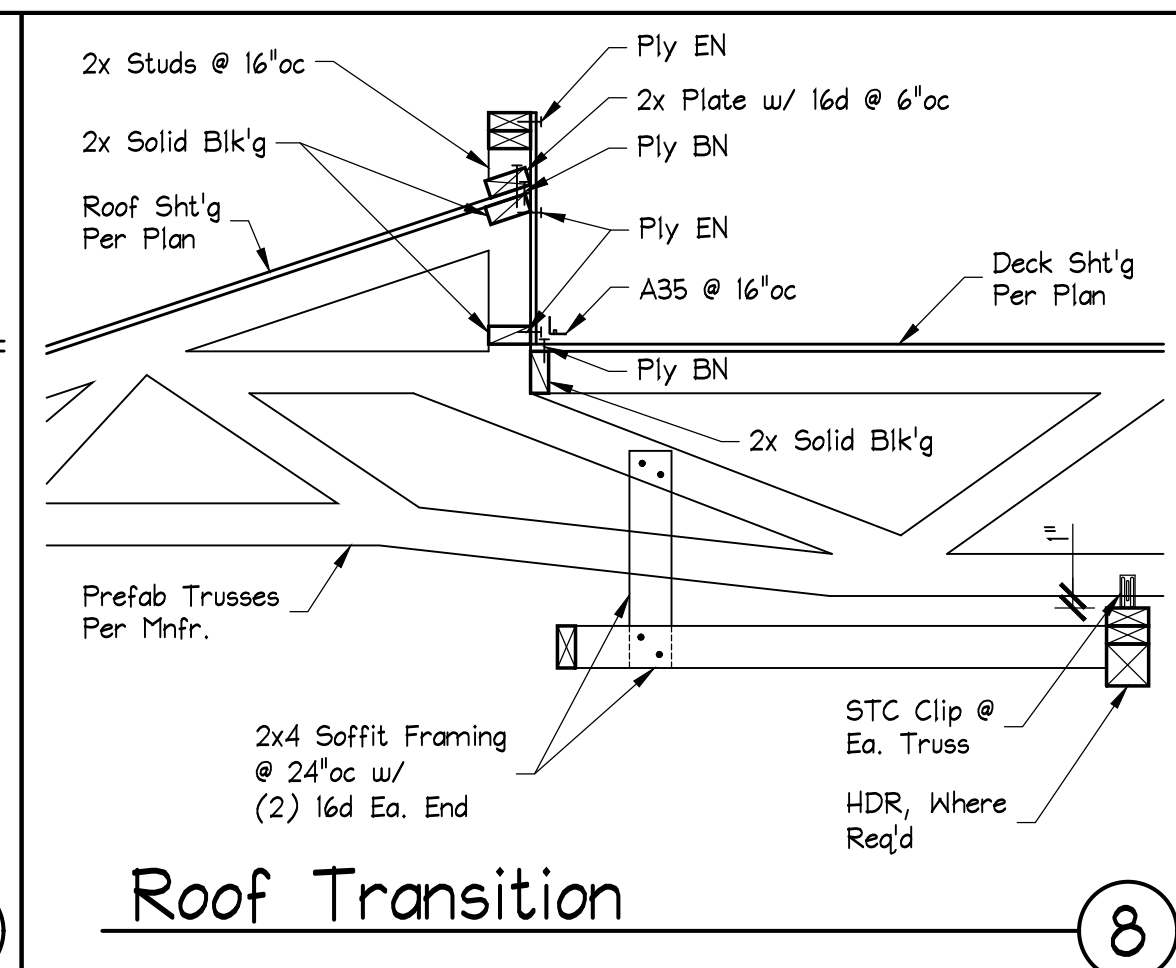
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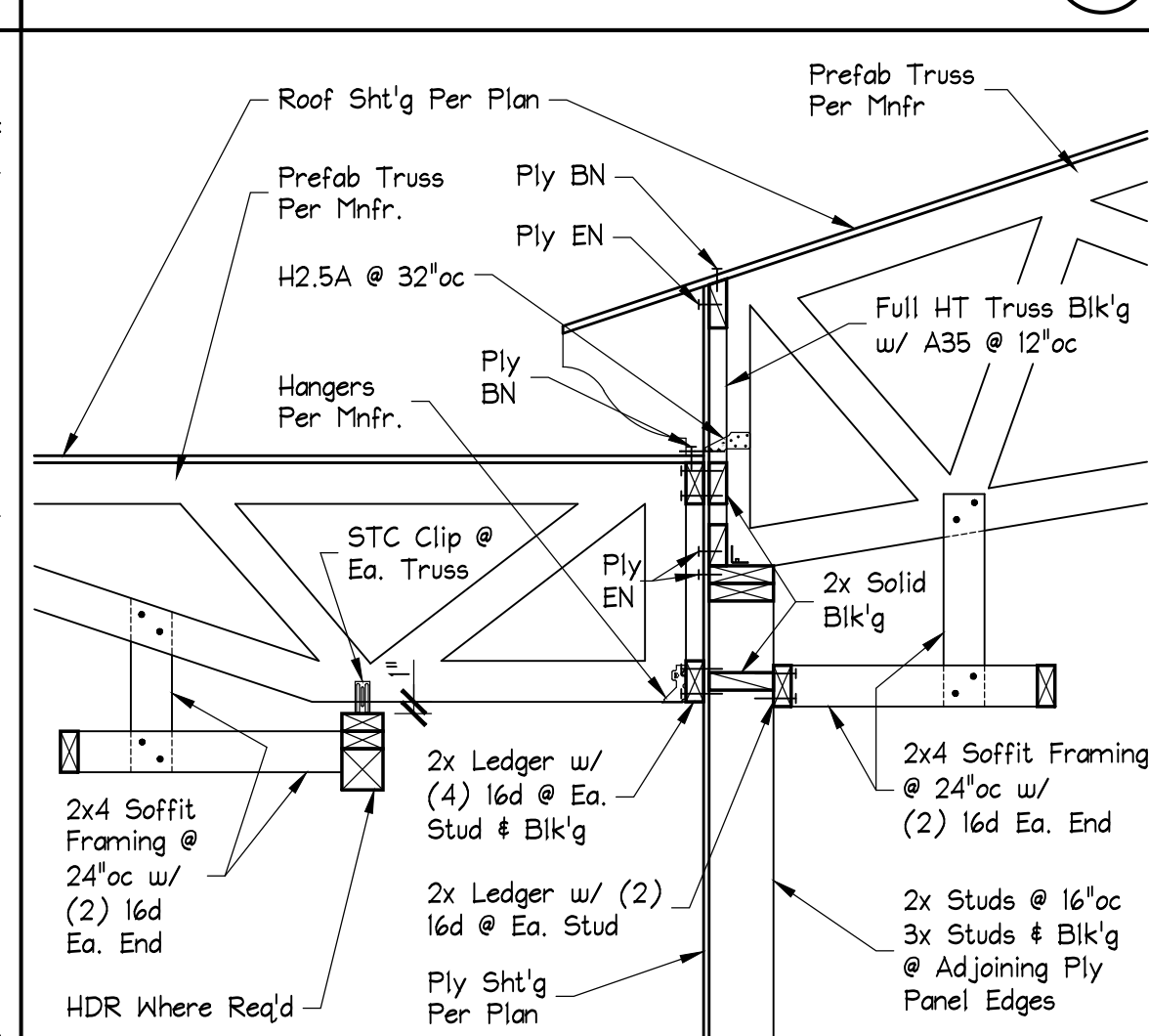
Roof Transition

9



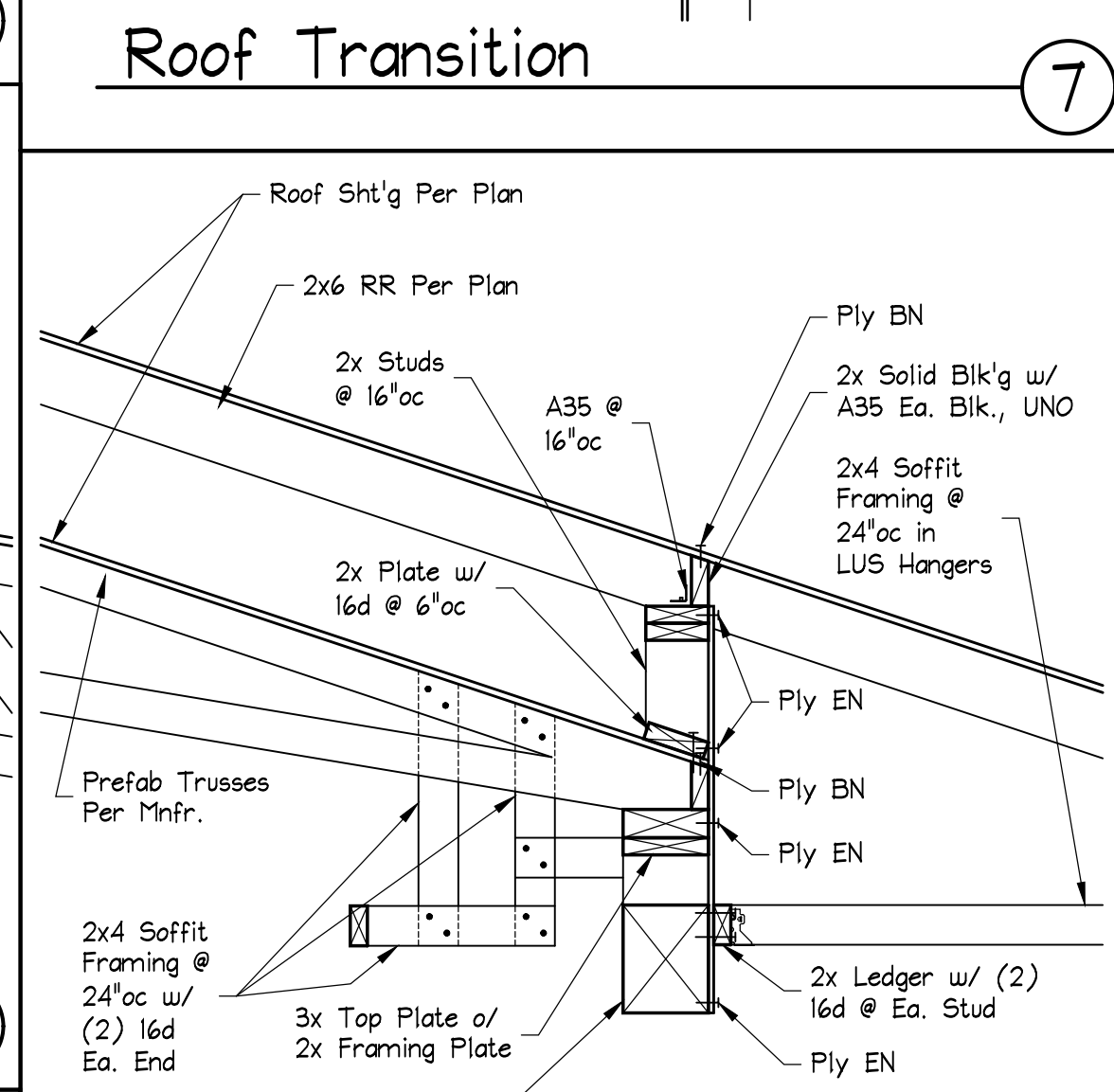
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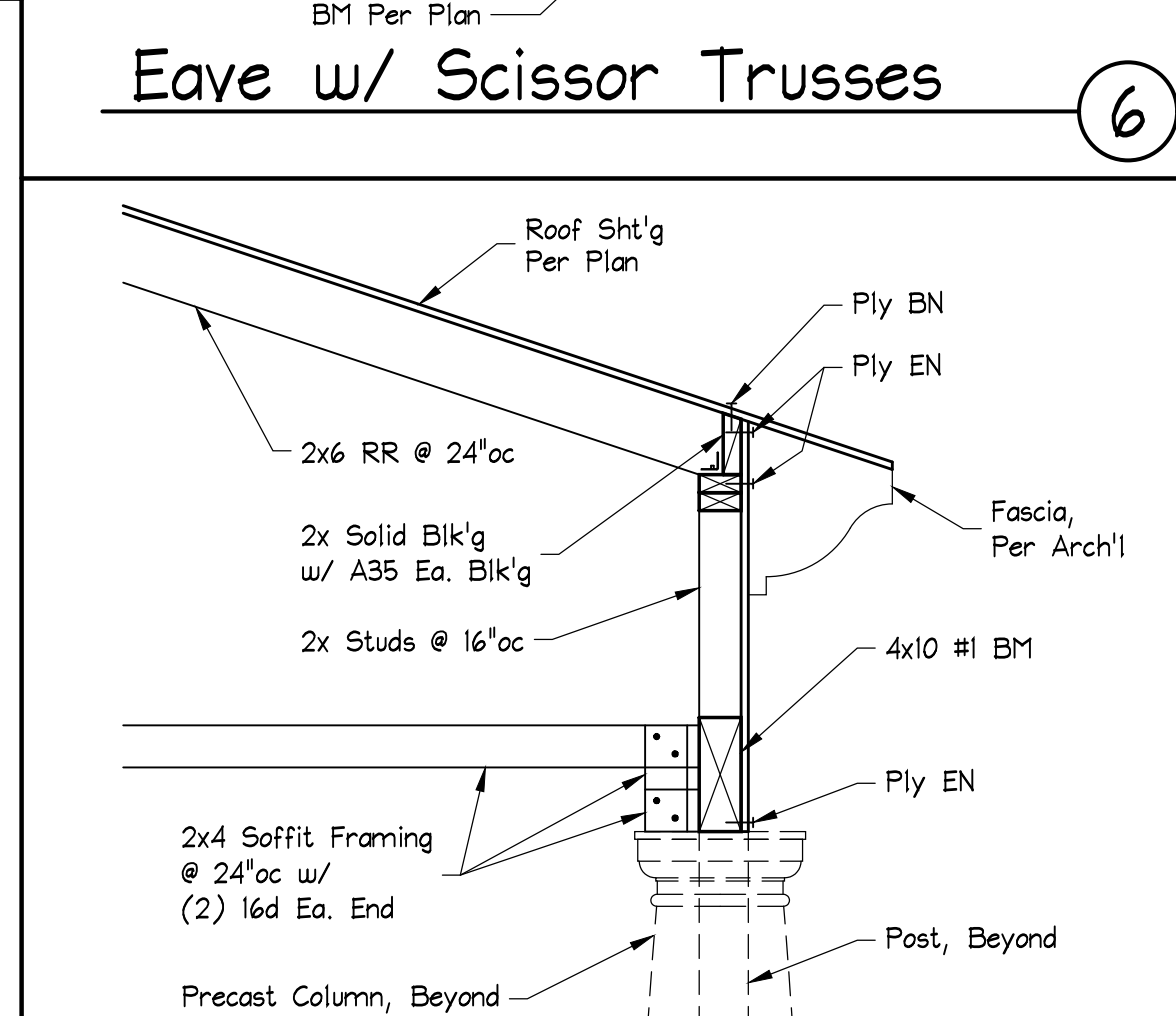
Roof Transition

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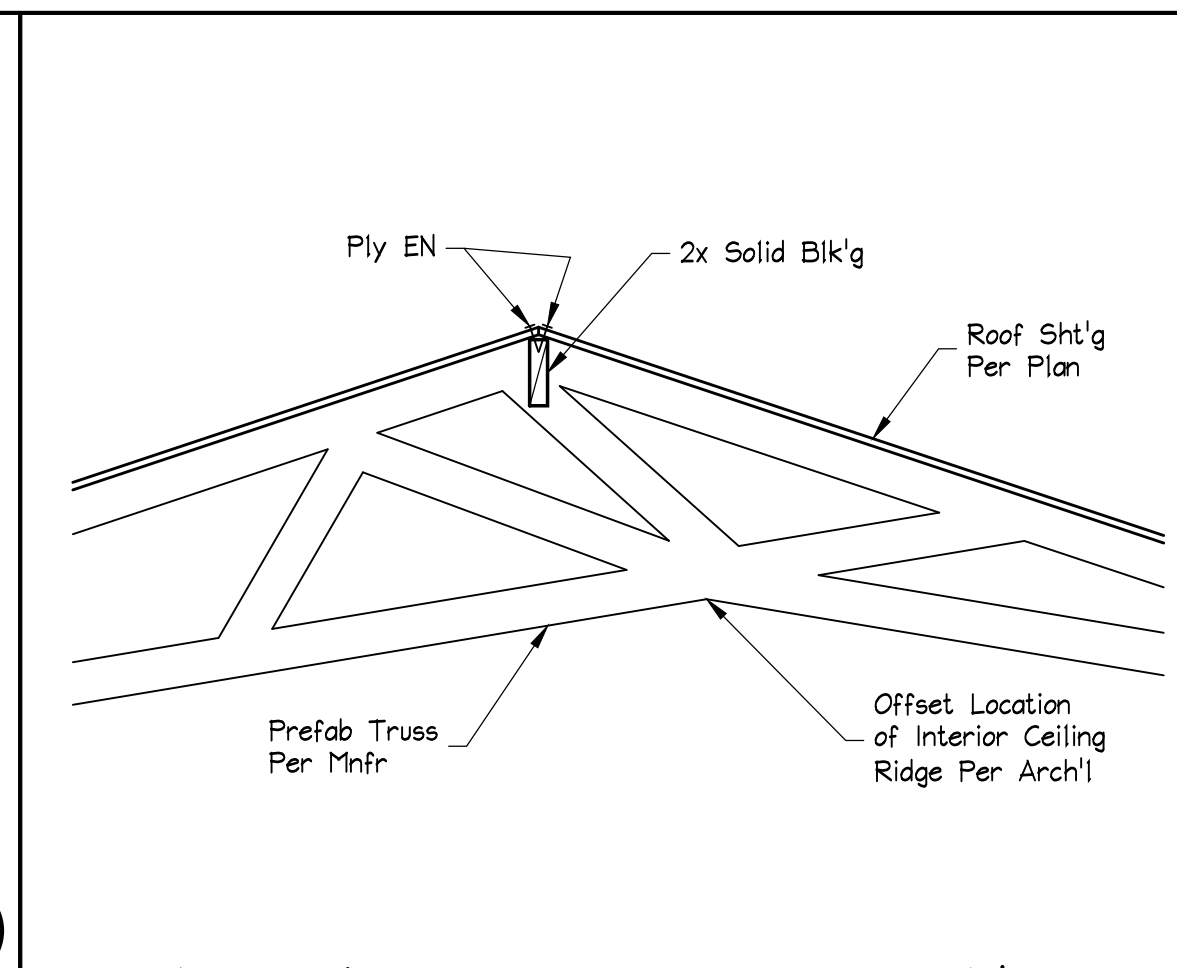
Eave w/ Scissor Trusses

6



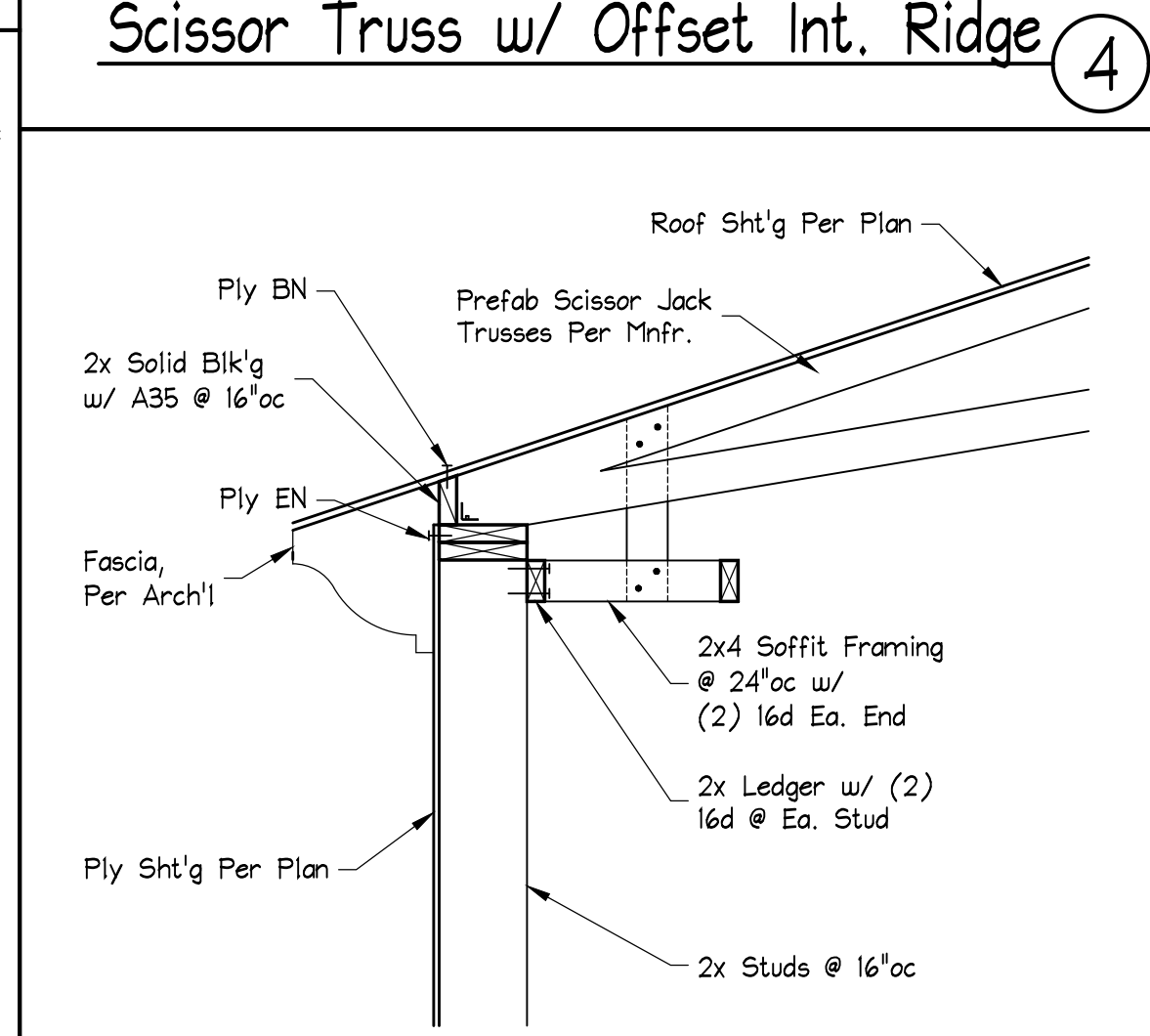
Eave @ Patio

5



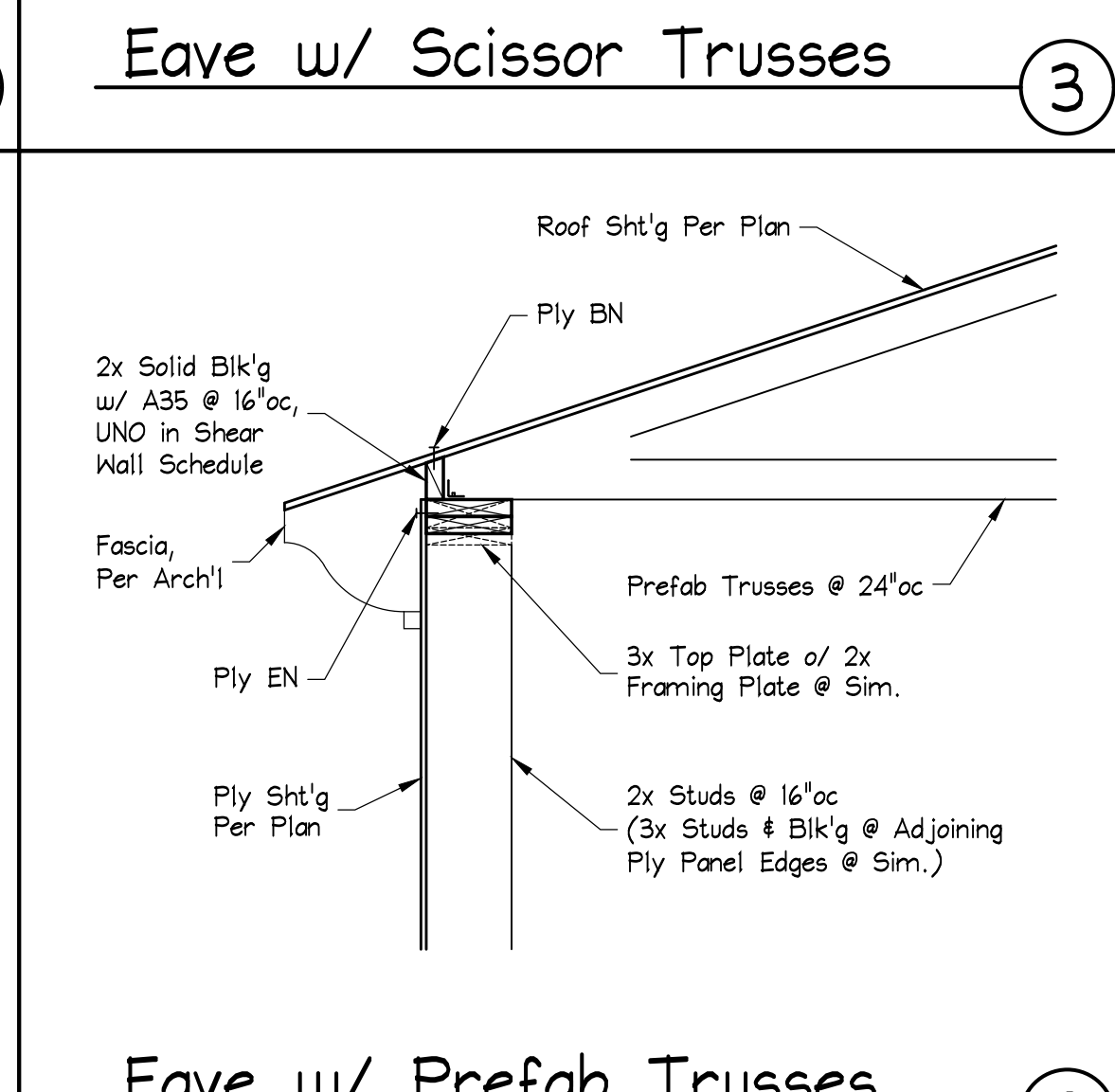
Scissor Truss w/ Offset Int. Ridge

4



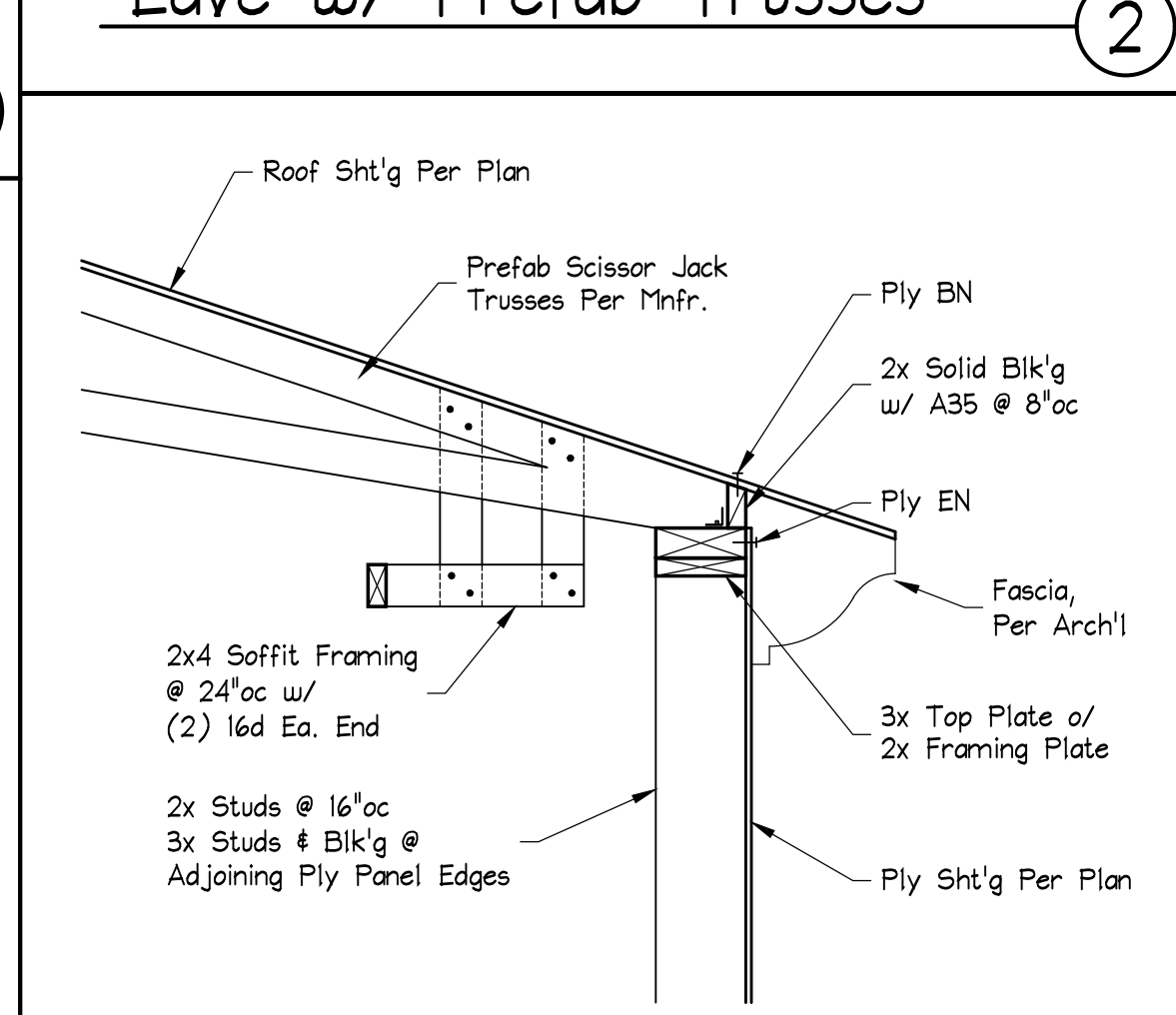
Eave w/ Scissor Trusses

3



Eave w/ Prefab Trusses

2



Eave w/ Scissor Trusses

1

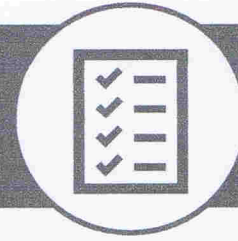
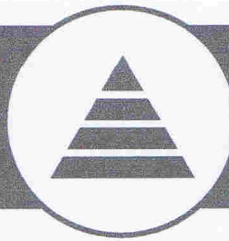
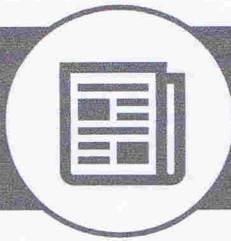
Scale: 3/4" = 1'-0", Typ.

MISSION GROUP ARCHITECTS
RE JOHNSON
AIA
PROFESSIONAL ENGINEER
No. 3734
Exp. 3/31/24
STRUCTURAL DETAILS
PROPOSED ADU FOR:
KISLAK RESIDENCE
3627 CAMPANIL DRIVE
SANTA BARBARA, CALIFORNIA 93109
DATE: 10/27/2022
SHEET: S4
K-2233
REVISIONS
1280 COAST VILLAGE CIRCLE STE 110 SANTA BARBARA CALIFORNIA 93108 805-969-5910 © COPYRIGHT MISSION GROUP ARCHITECTS



FINAL APPROVAL CHECKLIST

SUPPLEMENTAL APPLICATION



GENERAL INFORMATION

WHAT IS FINAL APPROVAL?

Final approval is the last level of design review before applying for a Building Permit (BLD) application. Final approval generally occurs at a separate hearing, after project design approval, and includes a complete set of working drawings with all details, color samples, door hardware, and exterior lighting fixtures for review. Applicants may also request project design approval and final approval on the same hearing date, if sufficient details are provided.

HOW DOES THE PROCESS WORK?

Once a project receives project design approval, it shall constitute the substantive design approval of the project. If substantial changes to the plans are proposed after project design approval, a new project design approval will be required. Design review comments on final approval should only address whether the design substantially conforms to the project design approval, and comments on details and landscaping.

WHEN IS A COMPLETED CHECKLIST REQUIRED?

A completed **Final Approval Submittal Checklist** is required when you submit for final approval. To resubmit an application, upload documents, like plans and letters, into the record in the City's Accela Citizen Access Portal (ACA) system, along with the [Resubmittal Form](#). All forms must be completed, signed, and submitted as a PDF attachment to your electronic submittal.

FINAL APPROVAL CHECKLIST

Provide required details and sheet references with your submittal for final approval. Fill in the blank or indicate N/A if "not applicable". Final approval does not permit the omission of any required information.

PROJECT ADDRESS: 3627 CAMPANIL DR. PLN RECORD ID: 2024-00274

ALL BUILDING ELEVATIONS	Sheet #	Sheet #
<input checked="" type="checkbox"/> Exterior Details	<u>A6.0 - A6.3</u>	<input checked="" type="checkbox"/> Paint or Stain Color (trim, etc.)
<input checked="" type="checkbox"/> Exterior Finishes	<u>A0.4 & A4.0</u>	<input checked="" type="checkbox"/> Materials (roofing, plaster, etc.)
<input checked="" type="checkbox"/> Parapet Heights	<u>A4.0 & A5.0</u>	<input type="checkbox"/> Exterior Lighting (incl. cut sheets)
<input type="checkbox"/> Roof/Attic/Understory Vents	<u>N/A</u>	<input type="checkbox"/> Specification Sheets, as applicable

CONSTRUCTION DETAILS	Sheet #	Sheet #
<input checked="" type="checkbox"/> Retaining Wall	<u>C4.0</u>	<input checked="" type="checkbox"/> Ironwork
<input checked="" type="checkbox"/> Window/Door detail	<u>A6.1</u>	<input checked="" type="checkbox"/> Stairs
<input checked="" type="checkbox"/> Roof Details (eaves)	<u>A6.0</u>	<input checked="" type="checkbox"/> Handrails
<input checked="" type="checkbox"/> Decks	<u>A3.0, A5 & 6.2</u>	<input type="checkbox"/> Skylights
<input type="checkbox"/> Fences/Arbors/Trellis	<u>N/A</u>	<input type="checkbox"/> Awnings
<input type="checkbox"/> Trash/Recycling Enclosures	<u>N/A</u>	<input checked="" type="checkbox"/> Gutters and Down Spouts

ELECTRICAL/MECHANICAL/PLUMBING EQUIPMENT	Sheet #
<input type="checkbox"/> Transformer Vault	<u>N/A</u>
<input checked="" type="checkbox"/> Utility Service Meter	<u>E2</u>
<input checked="" type="checkbox"/> Screening Elements	<u>A1.1</u>
<input checked="" type="checkbox"/> Generators/Electrical/Mechanical/HVAC (including cut sheets & dBA at property lines)	<u>A1.1</u>
<input type="checkbox"/> Fire Valves (Verify Fire Sprinkler Ordinance per SBMC §8.04 requirements)	<u>N/A</u>
<input type="checkbox"/> Cross Connection Control Devices (backflow device)	<u>N/A</u>

CONSULTANT/ENGINEER SHEETS	Sheet #	Sheet #
<input checked="" type="checkbox"/> Electrical	<u>E1 - E3</u>	<input checked="" type="checkbox"/> Structural
<input checked="" type="checkbox"/> Mechanical	<u>M1 - M2.1</u>	<input checked="" type="checkbox"/> Plumbing

ROOFTOP ARCHITECTURAL DETAILS

Sheet #

- | | |
|---|-------------|
| <input checked="" type="checkbox"/> HVAC Equipment (exhaust fans, condensing units, air conditioning units, etc.) | <u>MZ.1</u> |
| <input checked="" type="checkbox"/> Dimensions of equipment and screening | <u>A1.1</u> |
| <input type="checkbox"/> Mission tile roofing installation specifications | <u>N/A</u> |
| <input type="checkbox"/> Specification Sheets, if applicable | <u>N/A</u> |
| <input checked="" type="checkbox"/> Parapet Height | <u>A9.0</u> |
| <input type="checkbox"/> Screens | <u>N/A</u> |
| <input checked="" type="checkbox"/> Chimney Caps | <u>A6.0</u> |
| <input checked="" type="checkbox"/> Flashing | <u>A6.0</u> |
| <input checked="" type="checkbox"/> Gutters/ Scuppers | <u>A6.0</u> |
| <input checked="" type="checkbox"/> Solar panel location or potential future solar panel installation (if applicable) | <u>A3.0</u> |
| <input checked="" type="checkbox"/> High fire roof coverings, valleys, gutters | <u>A6.0</u> |

COLOR AND MATERIAL BOARDS

Sheet #

- | | |
|---|-------------------------|
| <input checked="" type="checkbox"/> Paint and Stain Color Names and Numbers | <u>A04.0 & A4.0</u> |
| <input checked="" type="checkbox"/> Material Type, Brand and Inventory Number | <u>11 11</u> |

LANDSCAPE PLAN

Sheet #

Sheet #

- | | | | |
|--|-------------|--|------------------------|
| <input type="checkbox"/> Irrigation Plan | <u>N/A</u> | <input type="checkbox"/> High Fire/Defensible Space | <u>N/A</u> |
| <input type="checkbox"/> Plant Species/Number/Sizes | <u>N/A</u> | <input type="checkbox"/> Water Conservation Standards | <u>N/A</u> |
| <input type="checkbox"/> Planters, Pots, Furniture | <u>N/A</u> | <input checked="" type="checkbox"/> Site Walls (materials and color) | <u>C4.0 & A4.0</u> |
| <input checked="" type="checkbox"/> Paving Materials | <u>C2.1</u> | <input type="checkbox"/> Backflow Device | <u>N/A</u> |
| <input checked="" type="checkbox"/> Erosion Control Measures | <u>C3.0</u> | <input type="checkbox"/> Rooftop Garden/Landscaped Roof | <u>N/A</u> |

Storm Water Management Program (SWMP)

Sheet #

- | | |
|---|-------------|
| <input checked="" type="checkbox"/> Location of filtration devices | <u>C2.0</u> |
| <input checked="" type="checkbox"/> Cross-section details | <u>C4.0</u> |
| <input checked="" type="checkbox"/> Drainage flow from all impervious areas | <u>C2.1</u> |
| <input checked="" type="checkbox"/> Amounts of new, replaced, or removed impervious areas | <u>C2.1</u> |
| <input checked="" type="checkbox"/> Hydrology/Storm Water Report | <u>C2.1</u> |